



LAKE COUNTRY

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1577.0063.01

WARNING NOTICE - The District of Lake Country has been made aware of potential fire safety hazards due to old water distribution infrastructure that does not meet current fire standards in certain older neighbourhoods that were amalgamated with the District. The District is undergoing a review of the required repairs to bring all water distribution systems up to standard. The District has posted more information regarding the location of potential risk areas, as well as the District's efforts to address these hazards at lakecountry.bc.ca.

Small Diameter Watermains

URBAN
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March 2016

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Appendices

Appendix A – Areas Reviewed for the Small Diameter Watermain Analysis

Appendix B – Available Fire Flow Mapping

Appendix C – Kalamalka Distribution System Upper Pressure Zone Review (Urban Systems)

Appendix D – Seaton Road Watermain Extension (Urban System)

Appendix E – Moberly Road Watermain Extension (Urban Systems)

Appendix F – Pre-Design Cost Estimates

Appendix G – List of Miscellaneous Projects

1 Introduction

The purpose of this report is to identify existing watermains and pressure reducing valve (PRV) stations that do not meet fire protection requirements as set out by the District of Lake Country (District) and to identify and prioritize upgrades to address these deficiencies.

Urban Systems Ltd. (Urban) met with the District to confirm design constraints, review pipe failure history and inspect proposed alignments to determine preferred upgrade concepts. The criteria and methodology used to identify deficient areas is outlined in Sections 2. The deficient areas are identified in Appendix B. Proposed upgrades and estimated costs are identified in Section 4. The proposed works are summarized and prioritized in Section 5 of the report.

2 Design Criteria and Methodology

The District identified six areas within the water system as identified in **Appendix A** that were to be reviewed for this project. The key design criteria used for this project are listed below:

- ▶ Minimum available fire flow of 5,000 L/min must be provided in all areas with the exception of two areas listed below where a minimum fire flow of 10,000 L/min is required;
 - Oyama Road (between Pelmewash Parkway and Sawmill Road Booster Station)
 - OK Centre Area, downstream of existing 6th Street PRV Station
- ▶ Fire flow demands are to be added to Maximum Day Demands (MDD);
- ▶ Minimum residual pressure under fire flow conditions is 140 kPa; and,
- ▶ Maximum velocity in pipes under MDD plus fire flow conditions shall not exceed 4 m/s.

In addition to the above criteria, the District provided mark-ups of other pipes and fire hydrants that are to be replaced because of age, capacity, material or likelihood of failure. These pipes are included in the proposed upgrades identified in subsequent sections of this report.

The District's Subdivision and Development Bylaw requires new subdivisions provide a minimum of 3,600 L/min unless additional flow is required per Fire Underwriter's Survey (FUS) Guidelines.

It should be noted that a detailed assessment was not completed to confirm that the 5,000 L/min and 10,000 L/min fire flows meet the Fire Underwriter's Survey recommendations for all existing land uses. Rather this value was arrived at through direction from the District based on balancing risk with costs.

Existing fire hydrant spacing was reviewed to ensure that the maximum distance between hydrants did not exceed 180 meters.

Urban used the above design criteria and the future conditions (2032) hydraulic water model to identify deficiencies in the existing available fire flow and required upgrades to address these deficiencies.

3 Results

Appendix B identifies areas where existing available fire flow is deficient. The next section of the report identifies upgrades to address these deficient areas.

4 Proposed Upgrades

Eleven (11) upgrades are proposed to address existing fire flow deficiencies. A summary of each project is identified in this section along with the estimated cost. Detailed cost estimates are provided in **Appendix F**. Miscellaneous upgrades to address concerns the District has with the water system are also identified in this section of the report.

4.1 Okanagan Centre Area

The Okanagan Centre Area is currently supplied by the Beaver Lake source. The distribution mains within the review area vary in diameter between 50 mm and 250 mm. Fire flow to the area is limited by the fact that the entire area is fed via a single supply main along Camp and Hare Roads, meaning most of the hydrants in the area are unable to supply a fire flow of more than 2,000 L/min.

The area was previously reviewed by Mould Engineering in their August 2009 report entitled “Okanagan Centre Area Water Supply Review”. The report used a maximum required fire flow rate of 5,400 L/min but did not address any offsite impacts east of Hare Road that could restrict flow.

There are four PRV stations within the study area and their capacities and demand are listed in **Table 1** below. The existing capacity of each station is sufficient to provide maximum day demands plus design fire flows.

Table 1: Okanagan Centre PRV Stations Demands vs. Capacities

Station Name	Valve Sizes (mm)	Estimated Demands (L/min)			Required FF (L/min)	Maximum Required Flow (L/min)	Maximum Capacity (L/min)
		2032 WDD	2032 MDD	2032 PHD			
Davidson (#18)	150 and 75	262	2,181	2,990	10,000	12,181	12,500
Camp (#17)	150 and 75	224	1,721	2,416	10,000	11,721	12,400
Hare (#14)	100 and 50	9	89	118	5,000 ⁽¹⁾	5,089	5,800
6 th Street (#15)	100 and 50	130	841	1,244	10,000	10,841	6,000 ⁽²⁾

⁽¹⁾ There are no fire hydrants currently installed downstream of PRV. At least one hydrant should be installed

⁽²⁾ Existing station capacity does not meet required fire flow plus maximum day demand

Two upgrade options were reviewed for improving fire flow to the Okanagan Centre Area. Optional pricing for replacing the existing PRVs with above ground structures and replacing asbestos cement (A/C) watermain were also reviewed at the District’s request.

Option 1

A new distribution main along Okanagan Centre Road and upsizing of the existing supply main from 6th Street to Davidson Road is recommended in order to supply the minimum required fire flow to the area.

The estimated cost allows for the existing rear-yard watermain between 5th Street and Camp Road to be abandoned per the District's request. The estimated cost to construct these upgrades is **\$4,300,000** and is shown on **Figure 1-1**. The figure also lists the fire flow required for each roadway within the service area. The cost estimate includes the replacement cost for the 6th Street PRV Station.

Option 2

The upgrades under Option 2 west of Hare Road are identical to Option 1. This option improves system looping and redundancy with a second supply main into the area and a new PRV station. The new supply main reduces the diameter of the Hare Road watermain upgrade required north of Withers Road and eliminates Camp Road watermain upgrades east of Cheesman Road. The estimated cost to construct the twin supply option is **\$4,020,000** (inclusive of the costs in Option 1 for upgrades west of Hare Road) and is shown on **Figure 1-2**. The figure also lists the fire flow required for each roadway within the service area. The cost estimate includes the replacement cost for the 6th Street PRV Station.

Preferred Option

The District will select a preferred option at the predesign stage. Option 2 provides more system redundancy than Option 1 but has higher operational costs as it requires a new PRV facility. Option 2 will require a utility easement from Hare Road to Camp Road across private property.

Optional Items

Optional A/C watermain replacement costs for two sections of watermain as shown on both Figures 1-1 and 1-2 has been provided in Table 2 per the District's request. Table 2 also includes optional pricing for PRV replacements.

Table 2: Okanagan Centre Area Upgrade Costs

Project	See Figure	See Cost Estimate (Appendix F)	Project Cost
Optional AC Watermain Replacement – Davidson PRV to Camp PRV	1-1 or 1-2	1.4 ⁽¹⁾	\$670,000
Optional AC Watermain Replacement – Tyndall Road to Davidson PRV	1-1 or 1-2	1.5	\$550,000
Optional PRV Replacement – Davidson Road	1-1 or 1-2	1.6	\$320,000
Optional PRV Replacement – Camp Road	1-1 or 1-2	1.7	\$320,000
Optional PRV Replacement – Hare Road	1-1 or 1-2	1.8	\$320,000

⁽¹⁾ This watermain segment is replaced under Option 1 and is only considered an optional cost should Option 2 be chosen

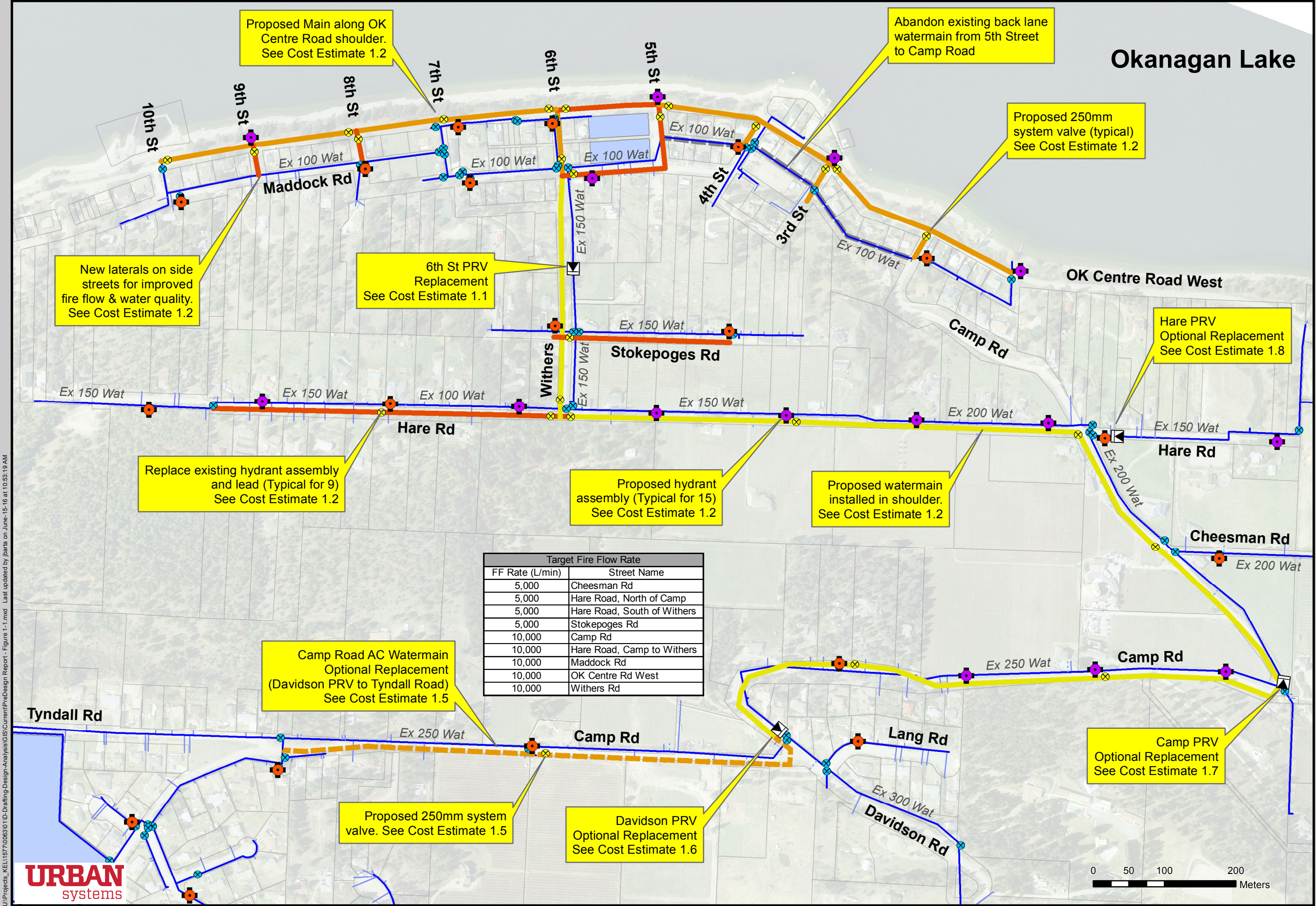


Okanagan Lake

Key Plan

Legend

- Reservoir
- PS Pump Station
- Power Generation
- Chlorinator
- Intake
- Pressure Tank
- Cistern
- PRV Station
- Existing Hydrants
- Proposed Hydrants
- System Valves
- Proposed System Valves
- District Lots
- Existing Watermains**
 - Watermain
 - Hydrant Lead
 - Drain Lines
 - Domestic Service
 - Irrigation Service
- Watermain Upgrades**
 - 200mm
 - 250mm
 - 250mm (Optional)
 - 300mm
 - Mains to be Abandoned



Proposed Main along OK Centre Road shoulder. See Cost Estimate 1.2

Abandon existing back lane watermain from 5th Street to Camp Road

Proposed 250mm system valve (typical) See Cost Estimate 1.2

New laterals on side streets for improved fire flow & water quality. See Cost Estimate 1.2

6th St PRV Replacement See Cost Estimate 1.1

Replace existing hydrant assembly and lead (Typical for 9) See Cost Estimate 1.2

Proposed hydrant assembly (Typical for 15) See Cost Estimate 1.2

Proposed watermain installed in shoulder. See Cost Estimate 1.2

Hare PRV Optional Replacement See Cost Estimate 1.8

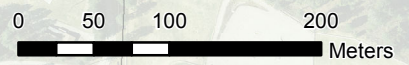
Camp Road AC Watermain Optional Replacement (Davidson PRV to Tyndall Road) See Cost Estimate 1.5

Proposed 250mm system valve. See Cost Estimate 1.5

Davidson PRV Optional Replacement See Cost Estimate 1.6

Camp PRV Optional Replacement See Cost Estimate 1.7

Target Fire Flow Rate	
FF Rate (L/min)	Street Name
5,000	Cheesman Rd
5,000	Hare Road, North of Camp
5,000	Hare Road, South of Withers
5,000	Stokepoges Rd
10,000	Camp Rd
10,000	Hare Road, Camp to Withers
10,000	Maddock Rd
10,000	OK Centre Rd West
10,000	Withers Rd



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Okanagan Centre Upgrades Option #1

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Jun 15, 2016

Figure 1-1



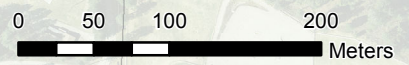
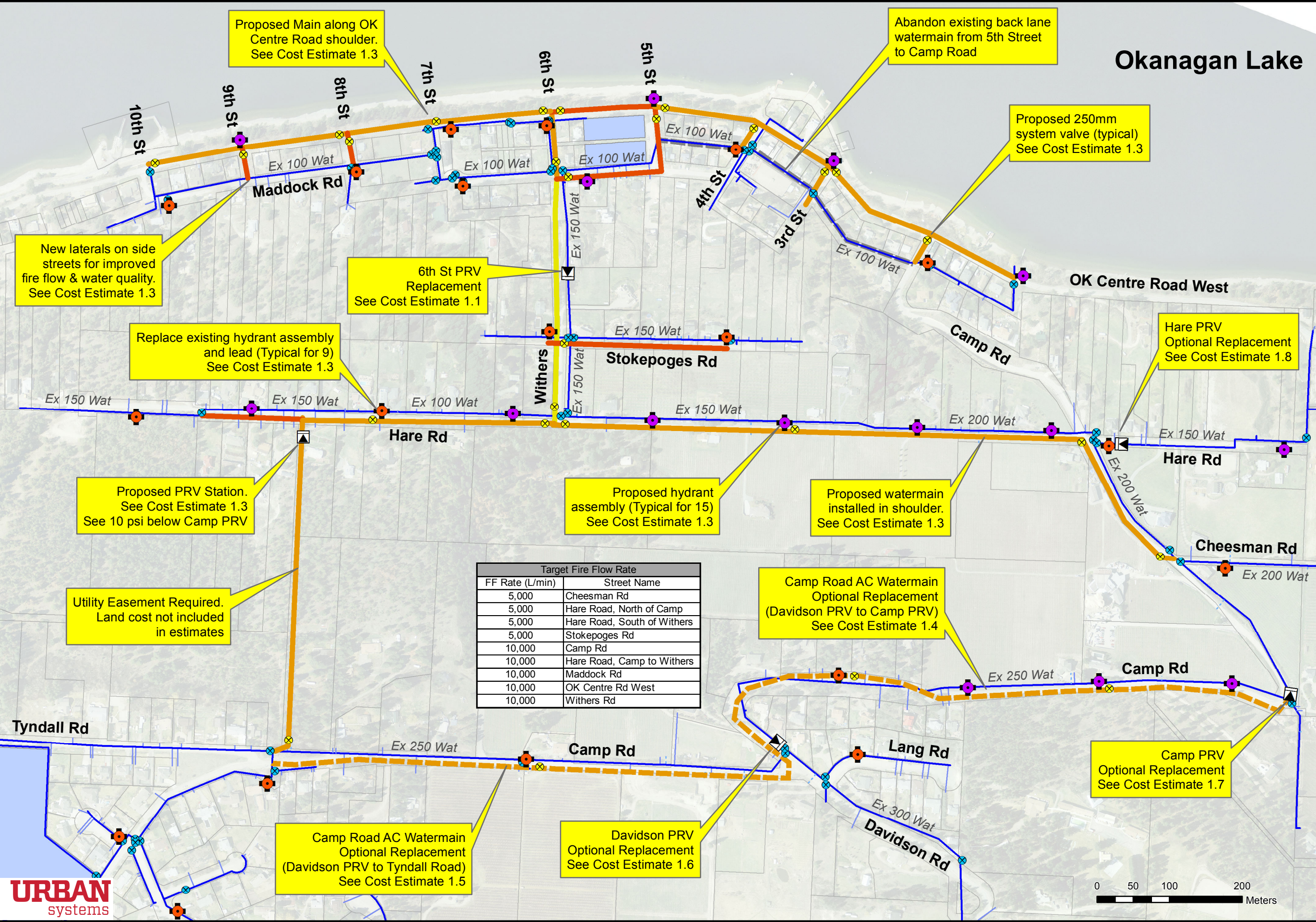
Okanagan Lake

Key Plan

Legend

- Reservoir
- PS Pump Station
- Power Generation
- Chlorinator
- Intake
- Pressure Tank
- Cistern
- PRV Station
- Existing Hydrants
- Proposed Hydrants
- System Valves
- Proposed System Valves
- Proposed PRV
- District Lots
- Existing Watermains**
 - Watermain
 - Hydrant Lead
 - Drain Lines
 - Domestic Service
 - Irrigation Service
- Watermain Upgrades**
 - 200mm
 - 250mm
 - 250mm (Optional)
 - 300mm
 - Mains to be Abandoned

Target Fire Flow Rate	
FF Rate (L/min)	Street Name
5,000	Cheesman Rd
5,000	Hare Road, North of Camp
5,000	Hare Road, South of Withers
5,000	Stokepoges Rd
10,000	Camp Rd
10,000	Hare Road, Camp to Withers
10,000	Maddock Rd
10,000	OK Centre Rd West
10,000	Withers Rd



Okanagan Centre Upgrades Option #2

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Figure 1-2

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4.2 Oyama Isthmus Area

The Oyama Isthmus Areas is supplied by Kalamalka Lake source. The proposed Oyama Isthmus Upgrade is broken down into two components based on fire flow requirements:

- 1) Oyama Road Upgrade (supply to the isthmus), requiring a minimum fire flow of 10,000 L/min; and
- 2) Isthmus Upgrades, requiring a minimum fire flow of 5,000 L/min.

Refer to **Figure 2** for all proposed upgrades noted below.

The existing distribution main along Oyama Road, between Trask and Greenhow Roads, currently restricts the available fire flow to the isthmus and Sawmill Road areas, when drawing solely from the Irvine Reservoir. The proposed watermain must be upgraded to 300 mm diameter to meet fire flow requirements of the isthmus. The upgrades allow for a maximum fire flow of 8,700 L/min along Oyama Road when drawing solely from the Irvine Reservoir. When drawing from both the Irvine Reservoir and the Oyama Lake water source (via the Sawmill Booster and PRV station), the available fire flow along Oyama Road increases to 20,000 L/min. The estimated cost to construct the Oyama Road upgrade is **\$360,000**.

The existing water system within the isthmus is well looped with the exception of the dead-end mains on Trask and Greenhow Roads. It is proposed to upgrade the existing watermains for portions of Trask, Ackerman, Roberts and Greenhow Roads. In addition to upsizing the undersized mains shown in the figure, it is suggested to replace the existing galvanized iron watermain in Harris Lane. The estimated cost to construct the isthmus upgrades is **\$870,000**.

A future subdivision between Greenhow and Trask Roads will also improve fire flow availability to the outlying parts of the isthmus. A watermain connection between Trask and Greenhow Roads must ultimately be constructed to ensure minimum fire flow requirements are met. The future watermain, shown on **Figure 2**, was assumed to be 200 mm in diameter, but was not included in the above cost estimate as it is assumed to be developer funded.



Key Plan

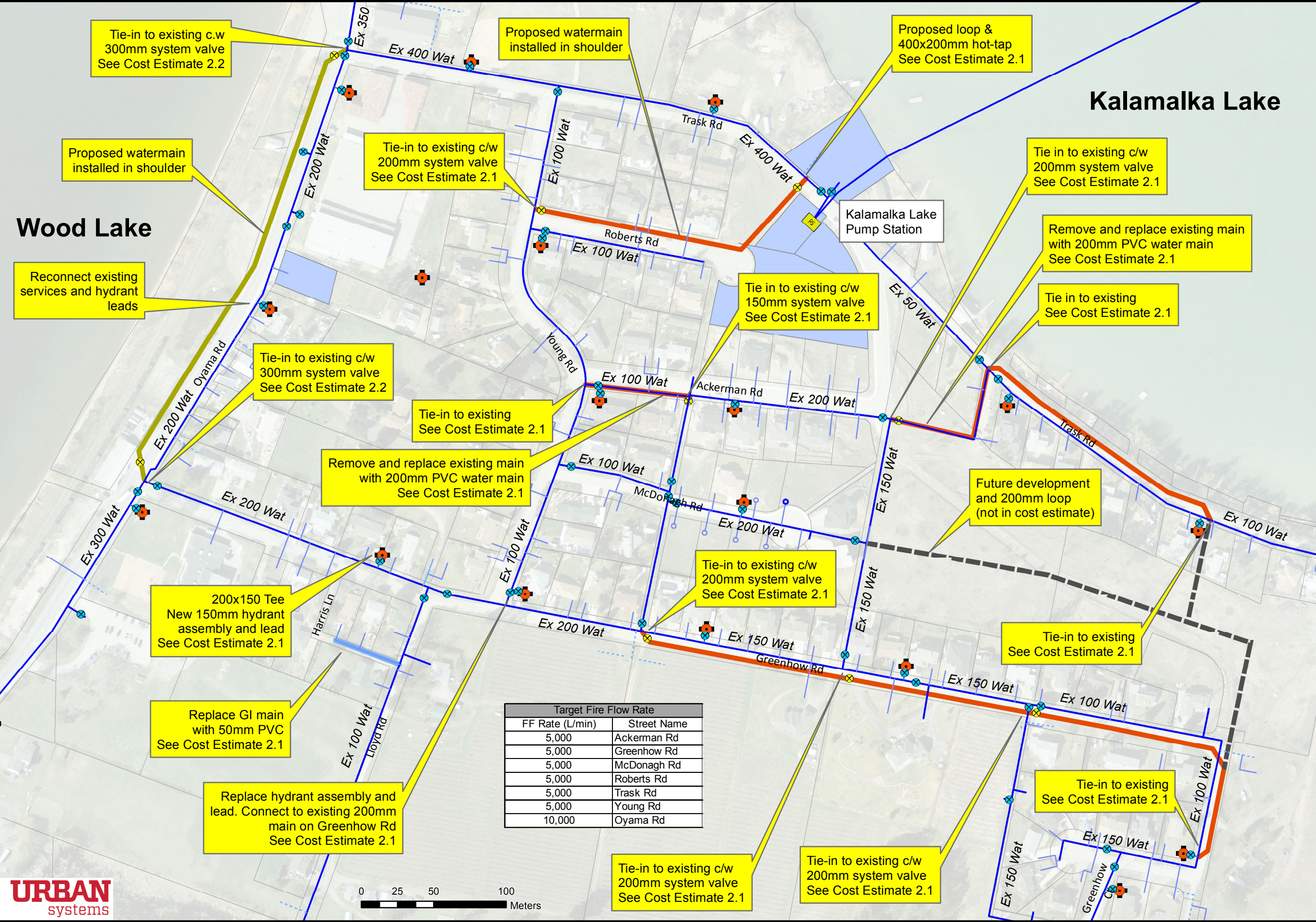
Legend

- Water Structures**
- Reservoir
 - PS Pump Station
 - Intake
 - System Valves
 - Existing Hydrants
 - Proposed System Valve
 - District Lots
- Existing Watermains**
- Watermain
 - Hydrant Lead
 - Drain Lines
 - Domestic Service
 - Irrigation Service
- Watermain Upgrades**
- 50mm
 - 200mm
 - 300mm
 - Future Watermains

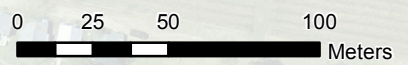
Oyama Isthmus Area Upgrade

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Mar 10, 2016

Figure 2



Target Fire Flow Rate	
FF Rate (L/min)	Street Name
5,000	Ackerman Rd
5,000	Greenhow Rd
5,000	McDonagh Rd
5,000	Roberts Rd
5,000	Trask Rd
5,000	Young Rd
10,000	Oyama Rd



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4.3 Irvine Boosted Zone

The Irvine Boosted Zone is fed by booster pumps which draw water from the Irvine Reservoir (Kalamalka Lake source). Many of the pressure and fire flow issues in this area were addressed in the Urban Systems report “Kalamalka Lake Water Distribution System – Upper Pressure Zone Review” (Irvine report), dated January 2013 and is included as **Appendix C**. The Irvine report used a minimum fire flow of 3,600 L/min as identified in the *MMCD Design Guidelines Manual 2005*, based on low density residential land use, which differs from the criteria in this document. The cost estimate for the booster station was updated to reflect the higher required fire flow rate of 5,000 L/min of this report.

The proposed Irvine Booster Zone Upgrade allows for:

- ▶ Replacing the existing watermain from the Irvine Booster site to the south end of Old Mission Road with a 200 mm watermain to improve fire protection (per the Irvine report); and,
- ▶ Replacing the existing 100 mm AC watermain between Old Mission and Highland Roads with a 150 mm PVC watermain.

The proposed upgrades are shown in **Figure 3**. The estimated cost for the Irvine Boosted Zone Watermain Upgrade is **\$1,140,000**. In addition to the proposed watermain upgrades, it is recommended that the Irvine Booster Station be upgraded per the 2013 Irvine report. The estimated cost for upgrading the booster station is **\$1,070,000**.

The watermains to the north of the Irvine Reservoir and Booster station serve only agricultural parcels and are not intended to provide fire protection. No upgrades have been identified for these mains.

The District enquired as to the possibility of removing the existing rear-yard watermain between Old Mission and Highland Roads. This idea was dismissed for the following reasons:

- pressure for Highland Road customers serviced from the Irvine boosted zone would drop;
- the restoration costs to re-align the water service was deemed prohibitive given the extent of retaining walls and landscaping present; and
- water quality is improved by creating a new loop at the south end of Old Mission Road



Key Plan

Legend

Water Structures

- Reservoir
- PS Pump Station
- Intake

PRV Stations

- PRV Station
- Existing Hydrants
- Proposed Hydrants
- System Valves
- Proposed System Valves

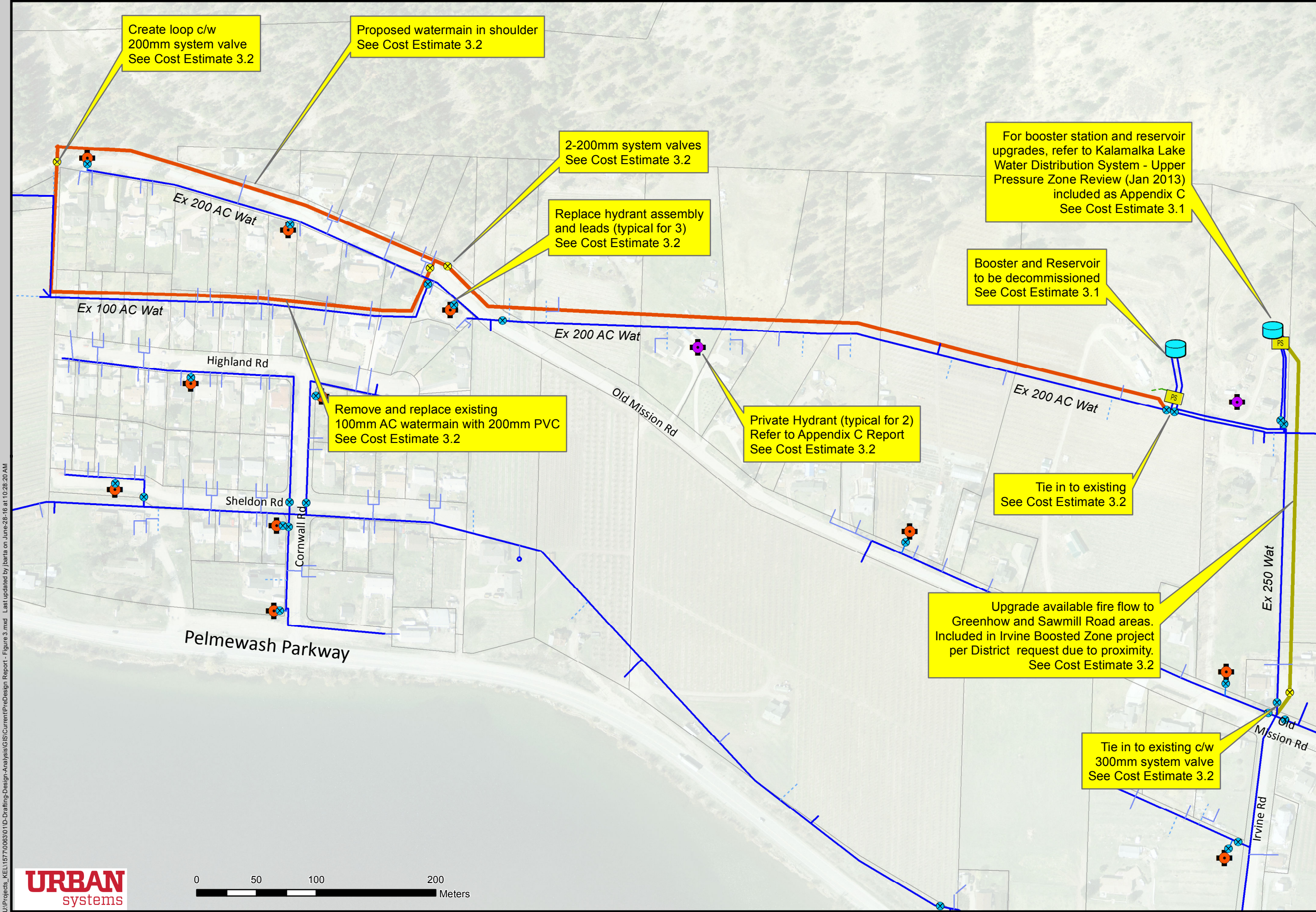
Existing Watermains

- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service

150mm
200mm
300mm

Irvine Boosted Zone Upgrades
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Mar 10, 2016

Figure 3



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4.4 Cornwall Road Area

The Cornwall Road area is gravity-fed by the Irvine Reservoir (Kalamalka Lake source), through a single supply main that varies in diameter between 150 mm and 200 mm. There is no looping in the area and the existing hydrant leads were assumed to be 100 mm in diameter.

The existing 200 mm and 150 mm supply mains must be upsized to 250 mm and 200 mm respectively to ensure adequate fire flow can be supplied to the existing The District suggested adding a loop between Highland and Sheldon Roads as shown on **Figure 4-1**. This proposed loop will improve water quality. Two upgrade options were reviewed for the Cornwall Road area as noted below.

Option 1

The proposed upgrade, which incorporates the items described above, for the area is shown on **Figure 4-1**. The estimated cost to construct the upgrade is **\$1,430,000**.

Option 2

An optional loop from Old Mission Road to Pelmewash Parkway can be constructed to avoid upsizing the existing 200 mm main to 250 mm along Pelmewash Parkway as shown on **Figure 4-2**. The estimated cost to construct this option is **\$1,290,000**.

Preferred Option

The District will select a preferred option at the predesign stage. Option 2 provides better system redundancy and has a lower capital cost than Option 1.



Key Plan

Legend

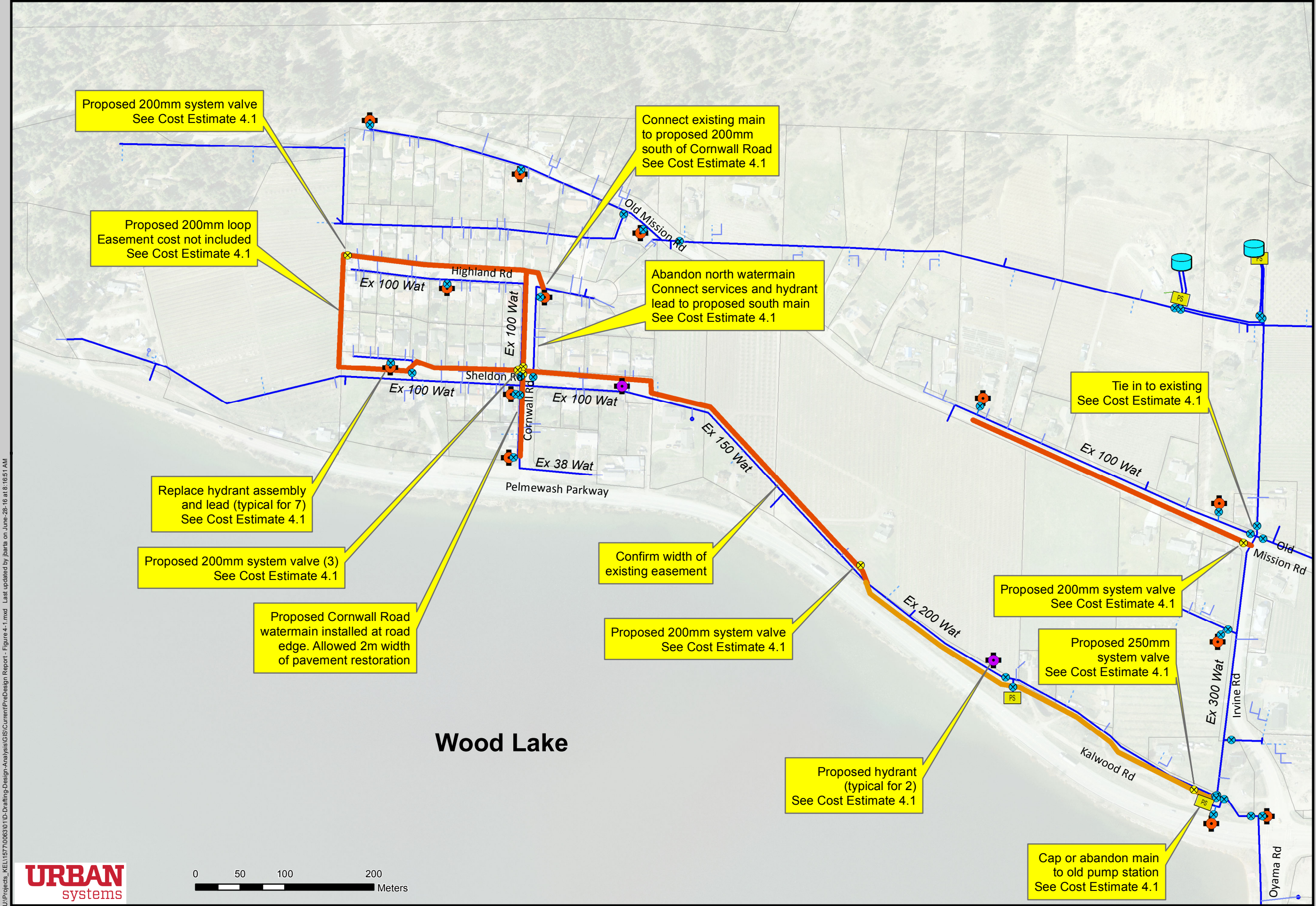
Water Structures

- Reservoir
 - PS Pump Station
 - Intake
 - PRV Station
 - System Valves
 - Proposed System Valves
 - Existing Hydrants
 - Proposed Hydrants
 - 200mm
 - 250mm
- Existing Watermains**
- Watermain
 - Hydrant Lead
 - Drain Lines
 - Domestic Service
 - Irrigation Service

Cornwall Road Upgrades Option #1

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Mar 10, 2016

Figure 4-1



Proposed 200mm system valve
See Cost Estimate 4.1

Proposed 200mm loop
Easement cost not included
See Cost Estimate 4.1

Connect existing main
to proposed 200mm
south of Cornwall Road
See Cost Estimate 4.1

Abandon north watermain
Connect services and hydrant
lead to proposed south main
See Cost Estimate 4.1

Tie in to existing
See Cost Estimate 4.1

Replace hydrant assembly
and lead (typical for 7)
See Cost Estimate 4.1

Proposed 200mm system valve (3)
See Cost Estimate 4.1

Proposed Cornwall Road
watermain installed at road
edge. Allowed 2m width of
pavement restoration

Confirm width of
existing easement

Proposed 200mm system valve
See Cost Estimate 4.1

Proposed 200mm system valve
See Cost Estimate 4.1

Proposed 250mm
system valve
See Cost Estimate 4.1

Proposed hydrant
(typical for 2)
See Cost Estimate 4.1

Cap or abandon main
to old pump station
See Cost Estimate 4.1

Wood Lake



Key Plan

Legend

Water Structures

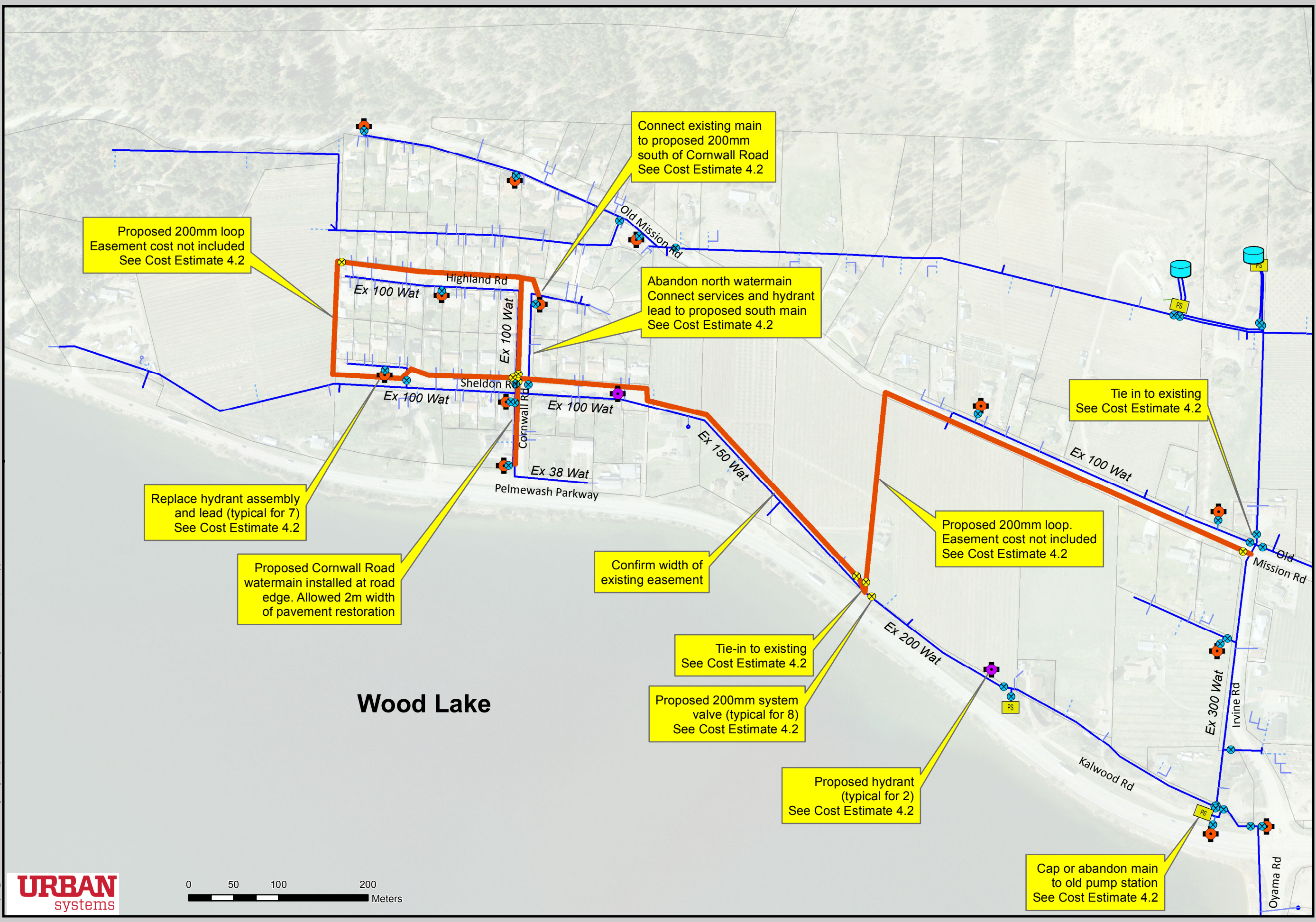
- Reservoir
- PS Pump Station
- Intake
- PRV Station
- System Valves
- Proposed System Valves
- Existing Hydrants
- Proposed Hydrants

Existing Watermains

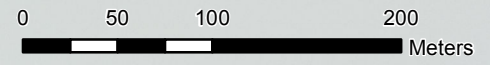
- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service

Watermain Upgrades

- 200mm



Wood Lake



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Cornwall Road Upgrades Option #2

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Mar 10, 2016

Figure 4-2

4.5 Carrs Landing Area

The Carrs Landing area is primarily supplied by a single watermain from the Beaver Lake water source. The majority of the existing watermains downstream of the Carrs Landing PRV station are 100 mm and 150 mm in diameter and there is minimal looping in this area.

There is also a private water system which supplies water to users along Moberly Road and the Raven Ridge water system (also known as the Lakepine system). Refer to **Figure 5-3**. These areas could be supplied by the Beaver Lake source in the future. As such, the demands for this area were considered in the sizing of the proposed PRV and watermain upgrades for Carrs Landing.

Table 3 below highlights the capacity of the existing PRV stations in the Carrs Landing Area and reflects the ultimate system demands that include the private water system customers being supplied via the Beaver Lake source. Per the table below, the Carrs Landing Road PRV station must be upgraded. Refer to **Table 4** for the Carrs Landing replacement PRV station cost estimate. It has been assumed that existing buried PRV chamber can be replaced with an above ground station. Land acquisitions costs are not included in the estimate. An estimate has also been included for the Goldie PRV Station replacement as per the District's request.

Table 3: Carrs Landing PRV Station Demands vs. Capacities

Station Name	Valve Sizes (mm)	Estimated Demands (L/min)			Required FF (L/min)	Maximum Required Flow (L/min)	Maximum Capacity (L/min)
		2032 WDD	2032 MDD	2032 PHD			
Goldie (#22)	150 and 50	190	4,130	6,195	5,000	9,130	16,650
Carrs Landing (#23)	100 and 50	177	3,750	5,625	5,000 ⁽¹⁾	8,750	5,700

⁽¹⁾ PRV capacity of 5,700 L/min is not sufficient to convey MDD+FF of 8,750 L/min

Additional fire hydrants were added along Carrs Landing Road and Pixton Road to ensure the maximum spacing between hydrants did not exceed 180 meters.

Two upgrade options were reviewed for the Carrs Landing Area as noted below.

Option 1 (not including demands from private water system)

The existing Carrs Landing Road watermain must be upsized to 250 mm in diameter (from Broadbent Road to Moberly Road) in order to meet the minimum fire flow requirements. In addition, the existing watermains on Pixton Road, McCreight Road and Carrs Landing north of Moberly Road must be upgraded to 200 mm diameter.

The proposed upgrade, which incorporates the items described above, for the area is shown on **Figure 5-1**. The estimated cost to construct the upgrade is **\$2,580,000** (including the Carrs Landing PRV replacement).

Option 2 (not including demands from the private water system)

An optional 200 mm watermain loop can be installed along Pixton Road as shown on **Figure 5-2**. The proposed loop would preclude the need to upgrade some of the existing 150 mm diameter piping at the north end of Pixton Road. The estimated cost to construct these upgrades is **\$2,520,000** (including the Carrs Landing PRV replacement). Land acquisition costs for an easement for the proposed watermain is not included in the estimate.

Preferred Option

The District will select a preferred option at the predesign stage. While Option 2 provides better system redundancy and has a lower capital cost than Option 1 (not including land acquisition costs), it will require land acquisition for utility easements.

Required Upgrades to Supply Private Water System from Beaver Lake

If the users that are currently supplied by the private water system switch over to the Beaver Lake source, an additional upgrade beyond those identified in Option 1 and 2 will be required to ensure adequate fire flow is available. The additional upgrade will involve upsizing a section of watermain along Carrs Landing Road and Goldie Road to 300 mm as shown on **Figure 5-3**. Additional discussion regarding the Moberly Road watermain extension is provided through a technical memorandum included under **Appendix E**.

Table 4 summarizes the various upgrade options for the Carrs Landing area water system.

Table 4: Carrs Landing Area Upgrade Costs

Project	See Figure	See Cost Estimate (Appendix F)	Project Cost
Replace Carrs Landing PRV Station (req'd both Options)	5-1 or 5-2	5.1	\$320,000
Option #1 Watermain Upgrades	5-1	5.2	\$2,260,000
Option #2 Watermain Upgrades	5-2	5.3	\$2,200,000
Additional Upgrades to Supply Private Water System	5-3	5.4	\$270,000
Optional Goldie PRV Replacement	5-1 or 5-2	5.5	\$320,000

Okanagan Lake



Key Plan

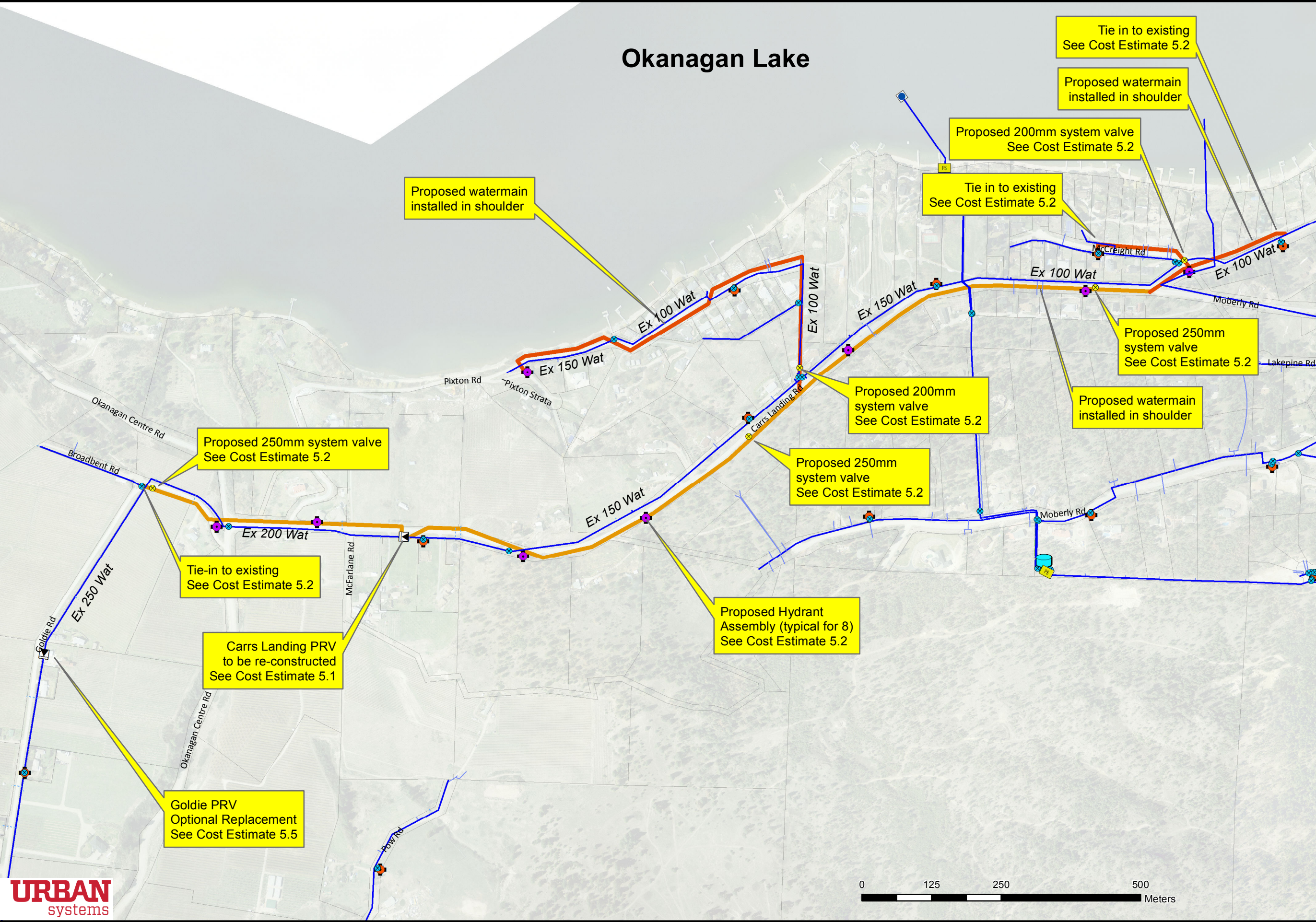
Legend

Water Structures

- Reservoir
- PS Pump Station
- Intake
- PRV Station
- System Valves
- Proposed System Valves
- Existing Hydrants
- Proposed Hydrants

Existing Watermains

- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service
- 200mm
- 250mm



Proposed 250mm system valve
See Cost Estimate 5.2

Tie-in to existing
See Cost Estimate 5.2

Carrs Landing PRV
to be re-constructed
See Cost Estimate 5.1

Goldie PRV
Optional Replacement
See Cost Estimate 5.5

Proposed watermain
installed in shoulder

Proposed 250mm
system valve
See Cost Estimate 5.2

Proposed Hydrant
Assembly (typical for 8)
See Cost Estimate 5.2

Proposed 200mm
system valve
See Cost Estimate 5.2

Proposed 200mm system valve
See Cost Estimate 5.2

Tie in to existing
See Cost Estimate 5.2

Proposed watermain
installed in shoulder

Tie in to existing
See Cost Estimate 5.2

Proposed 250mm
system valve
See Cost Estimate 5.2

Proposed watermain
installed in shoulder

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**Carrs
Landing Area
Upgrades
Option #1**
1:6,500
Mar 10, 2016

Figure 5-1

Okanagan Lake



Key Plan

Legend

Water Structures

- Reservoir
- PS Pump Station
- Intake
- PRV Station
- System Valves
- Proposed System Valve
- Existing Hydrants
- Proposed Hydrants

Existing Watermains

- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service

Watermain Upgrades

- 200mm
- 250mm

Proposed watermain installed in shoulder

Tie in to existing
See Cost Estimate 5.3

Proposed watermain installed in shoulder

Proposed 200mm system valve
See Cost Estimate 5.3

Tie in to existing
See Cost Estimate 5.3

Proposed 250mm system valve
See Cost Estimate 5.3

Proposed watermain installed in shoulder

Proposed 200mm loop.
Easement cost not included.
See Cost Estimate 5.3

Proposed 250mm system valve
See Cost Estimate 5.3

Proposed 200mm system valve
See Cost Estimate 5.3

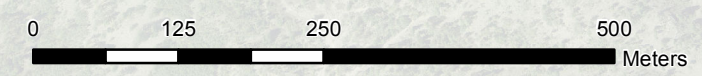
Proposed 250mm system valve
See Cost Estimate 5.3

Proposed Hydrant Assembly (typical for 8)
See Cost Estimate 5.3

Tie-in to existing
See Cost Estimate 5.3

Carrs Landing PRV to be re-constructed
See Cost Estimate 5.1

Goldie PRV
Optional Replacement
See Cost Estimate 5.5



Carrs Landing Area Upgrades Option #2
1:6,500
Mar 10, 2016

Figure 5-2

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Key Plan

Legend

Water Structures

- Reservoir
- PS Pump Station
- Intake
- PRV Station
- System Valves
- Existing Hydrants

Existing Watermains

- District Lots
- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service
- 300mm
- Other Water Systems

Okanagan Lake

RavenRidge (Lakepine) Booster Station & Intake

private water system boundary (Moberly Road)

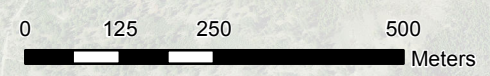
Carrs Landing PRV

RavenRidge water system boundary

Extent of Carrs Landing watermain to be oversized from 250mm to 300mm to accommodate future connection of Moberly Road and/or Ravenridge customers to Beaver Lake source. See Cost Estimate 5.4 See Appendix E for additional details See Figures 5-1 and 5-2 for all other Carrs Landing upgrades.

Goldie PRV

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Carrs Landing Area Moberly Oversizing
1:10,000
Mar 10, 2016

Figure 5-3

4.6 Ponderosa Road Area

The Ponderosa Road area is fed from the Okanagan Lake source water via a distribution main from the Lakes subdivision and has two pressure zones. Both zones are controlled by a PRV station, installed two hundred meters from the north end of Old Mission Road during the 2014 realignment of Highway 97. As shown in **Table 5**, the PRV station was adequately designed to meet the fire flow requirements of the area.

Table 5: Maximum Flow through PRV Stations

Station Name	Valve Sizes (mm)	Estimated Demands (L/min)			Required FF (L/min)	Maximum Required Flow (L/min)	Maximum Capacity (L/min)
		2032 WDD	2032 MDD	2032 PHD			
Old Mission Road Upper (North) (#30)	150 and 50	3	16	24	5,000	5,016	17,800
Old Mission Road Lower (South) (#30)	150 and 37.5	60	199	302	5,000	5,199	12,100

The upper pressure zone is located at the north end of Old Mission Road is fed by a 200 mm watermain and has two hydrants. This portion of the water system is sufficient to meet the fire flow and pressure requirements set out in this report.

The lower pressure zone is comprised of the existing watermains along Old Mission Road, south of the PRV station, and the watermain along Ponderosa Road. The majority of the lower pressure zone watermains are 100 mm in diameter. The lower pressure zone also has three standpipes that can be used by the fire department.

In order to provide the required fire flow to the lower pressure zone, all watermains must be upgraded to 200 mm pipe size and the existing hydrant assemblies (standpipes) and leads should be replaced with new units meeting the current District standards. The existing spacing between the standpipes is 260 meters and new hydrant assemblies will be installed at the same locations.

The proposed upgrades to the Ponderosa Road area water system are shown in **Figure 6**. The estimated cost to construct these upgrades is **\$670,000**.



Key Plan

Legend

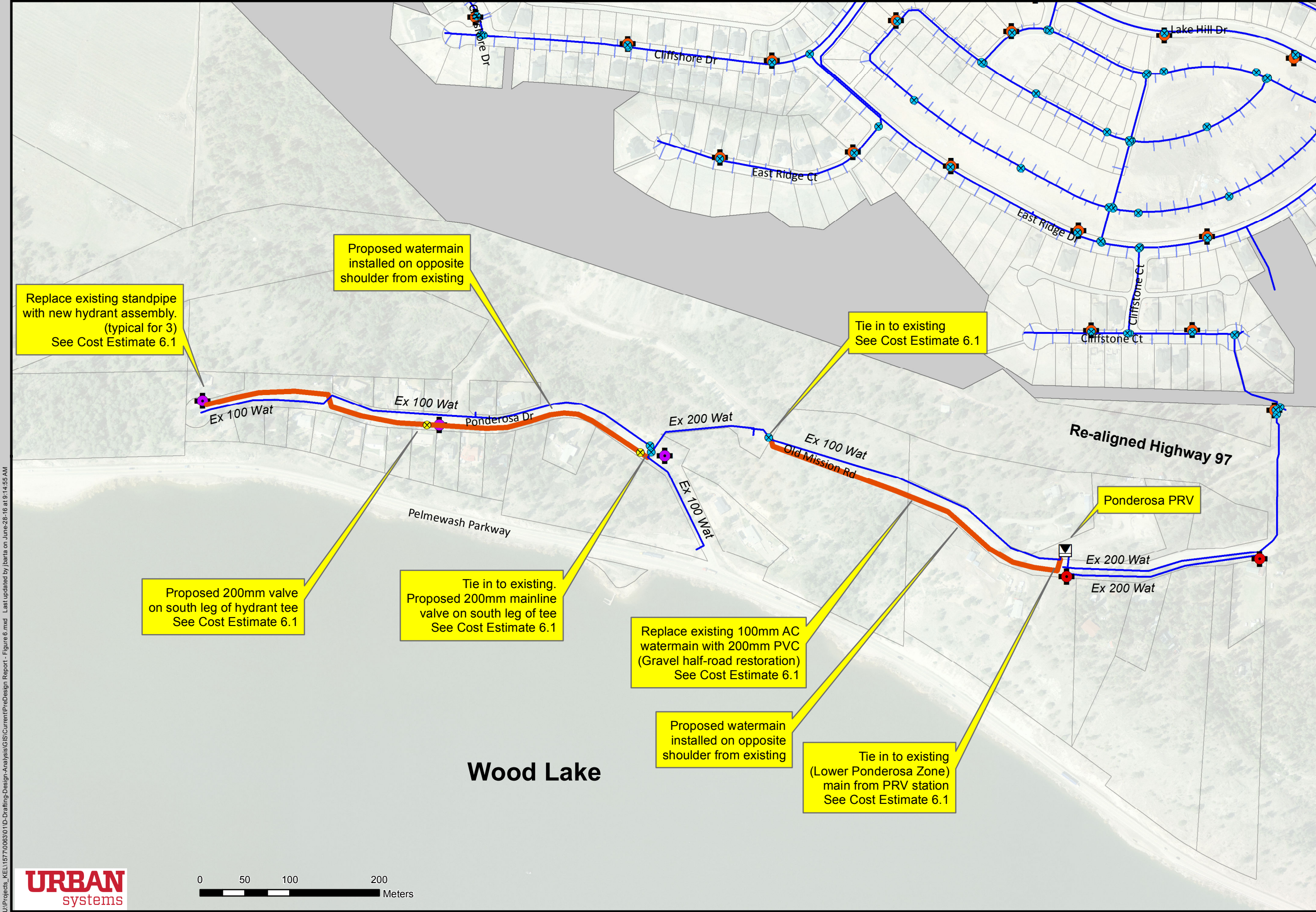
Water Structures

- Reservoir
 - PS Pump Station
 - Intake
 - PRV Station
 - Ponderosa PRV (Hwy97 Project)
 - System Valves
 - Existing Hydrants
 - Existing Hydrants (Hwy97 Project)
 - Proposed Hydrants
 - Proposed System Valves
 - District Lots
- Existing Watermains**
- Watermain, AC
 - Watermain, GI
 - Watermain, All Other
 - Irrigation Service
 - Domestic Service
 - Hydrant Lead
 - Drain Lines
 - Ponderosa Mains (Hwy97 Project)
- Diameter**
- 200mm

Ponderosa Road Area Upgrades

1:4,000
Mar 10, 2016

Figure 6



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4.7 Coral Beach Area

The Coral Beach Area is supplied by a small water system that draws water from Okanagan Lake. The Coral Beach water system provides service to approximately 65 lots and consists of a lake intake, booster station, chlorinator, a ninety (90) cubic meter reservoir, three (3) fire hydrants and distribution piping that varies in diameter between 50 mm and 250 mm.

The existing fire hydrants are not adequately spaced to provide fire flow coverage to the existing customers on the system. The hydrant assemblies are connected to the distribution system with 100 mm diameter leads and each assembly and hydrant lead should be replaced with a hydrant meeting the current District standards. Three additional hydrant assemblies will be added to provide suitable coverage.

The existing 75 mm and 100 mm distribution mains along Coral Beach Road that supply the hydrant assemblies should be abandoned and replaced with 200 mm diameter watermains. The supply main to the reservoir should also be abandoned and replaced with a 200 mm watermain. The District could consider converting the existing reservoir common inlet and outlet pipe to a dedicated fill line after the upgrades are completed to improve reservoir turnover times.

The existing reservoir is not sufficient to meet the balancing, fire flow and emergency storage requirements of the system. The District provided the 2015 maximum daily consumption of 300 L/min for the Coral Beach system. The typical reservoir sizing methodology is $S = A + B + C$, where:

A = equalization storage (25% of maximum day demands);

B = fire flow storage (per FUS requirements); and

C = emergency storage (25% of A plus B).

$$A = 0.25 \times 300 \text{ L/min} \times 1,440 \text{ min/day} \times 1 \text{ cubic meter}/1000 \text{ L} = 108 \text{ m}^3$$

$$B = 5000 \text{ L/min} \times 1.75 \text{ hours} \times 60 \text{ min/hr} \times 1 \text{ cubic meter}/1000 \text{ L} = 525 \text{ m}^3$$

$$C = 0.25 \times (108 + 525) = 158 \text{ m}^3$$

$$S = 791 \text{ m}^3$$

This report assumes that the existing valve chamber and reservoir inlet/outlet piping will be replaced with 200 mm piping. An expansion of 700 cubic meters will be required to provide adequate storage.

The proposed upgrades to the Coral Beach Water System are shown on **Figure 7**. The estimated cost to construct these upgrades is **\$1,370,000**. This estimate does not include any treatment upgrades that the District may want to consider (e.g., UV disinfection).

Okanagan Lake



Key Plan

Legend

Water Structures

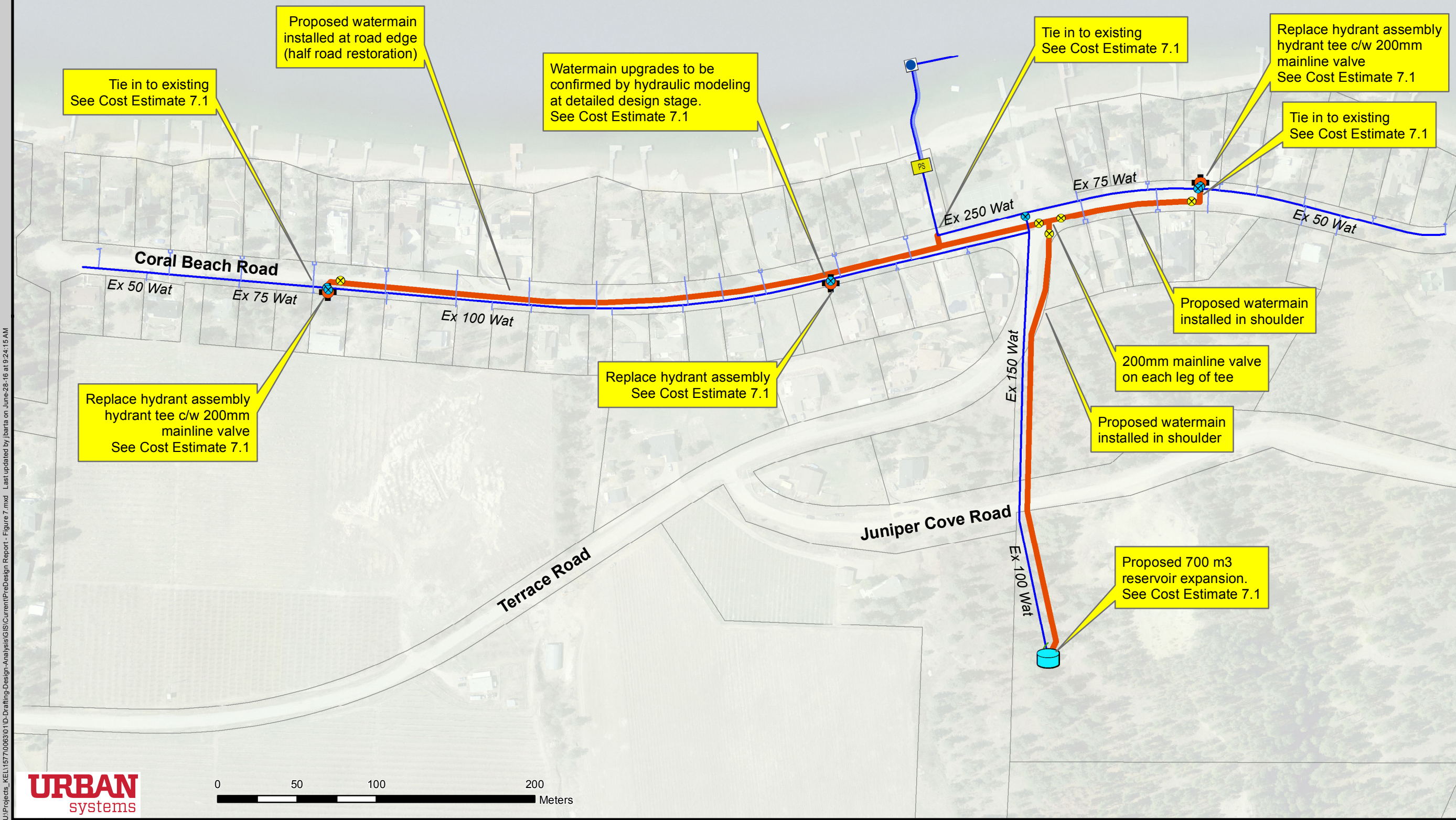
- Reservoir
- PS Pump Station
- Intake
- PRV Station
- System Valves
- Existing Hydrants
- Proposed System Valve

Existing Watermains

- Watermain
- Hydrant Lead
- Domestic Service
- Irrigation Service

Watermain Upgrades

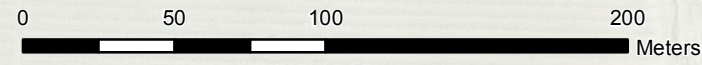
- 200mm



Coral Beach Area Upgrades

1:2,500
Mar 10, 2016

Figure 7



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4.8 Owls Nest Road Area

The Owls Nest Road area is currently supplied by a single distribution main, varying in size from 150 mm to 200 mm from the Irvine Reservoir (Kalamalka Lake source). The existing main crosses through an orchard between Owls Nest Road and Pelmewash Parkway which presents maintenance and access issues. The District has also requested the replacement of several watermain segments along Old Mission Road due to the pipe material being series 160 PVC.

Two options were reviewed for supplying the Owls Nest Road area with the minimum required fire flow as noted below.

Option 1

The existing watermain from Irvine Road to Evans Road can be upgraded 250 mm pipe and the remainder of Owls Nest Road can be upgraded to 200 mm pipe as is shown on **Figure 8-1**. The estimated cost to construct these upgrades is **\$1,590,000**.

Option 2

The second option is to create a new 200 mm loop from Old Mission Road to Owls Nest Road via Pelmewash Parkway and Evans Road. The loop removes the need to upgrade any piping through the existing orchard between Pelmewash Parkway and Owls Nest Road. This option still requires the Old Mission Road watermain to be upgraded to 250 mm from Irvine Road to Pelmewash Parkway. This second option is shown on **Figure 8-2**. The estimated cost to construct these upgrades is **\$1,730,000**.



Key Plan

Legend

Water Structures

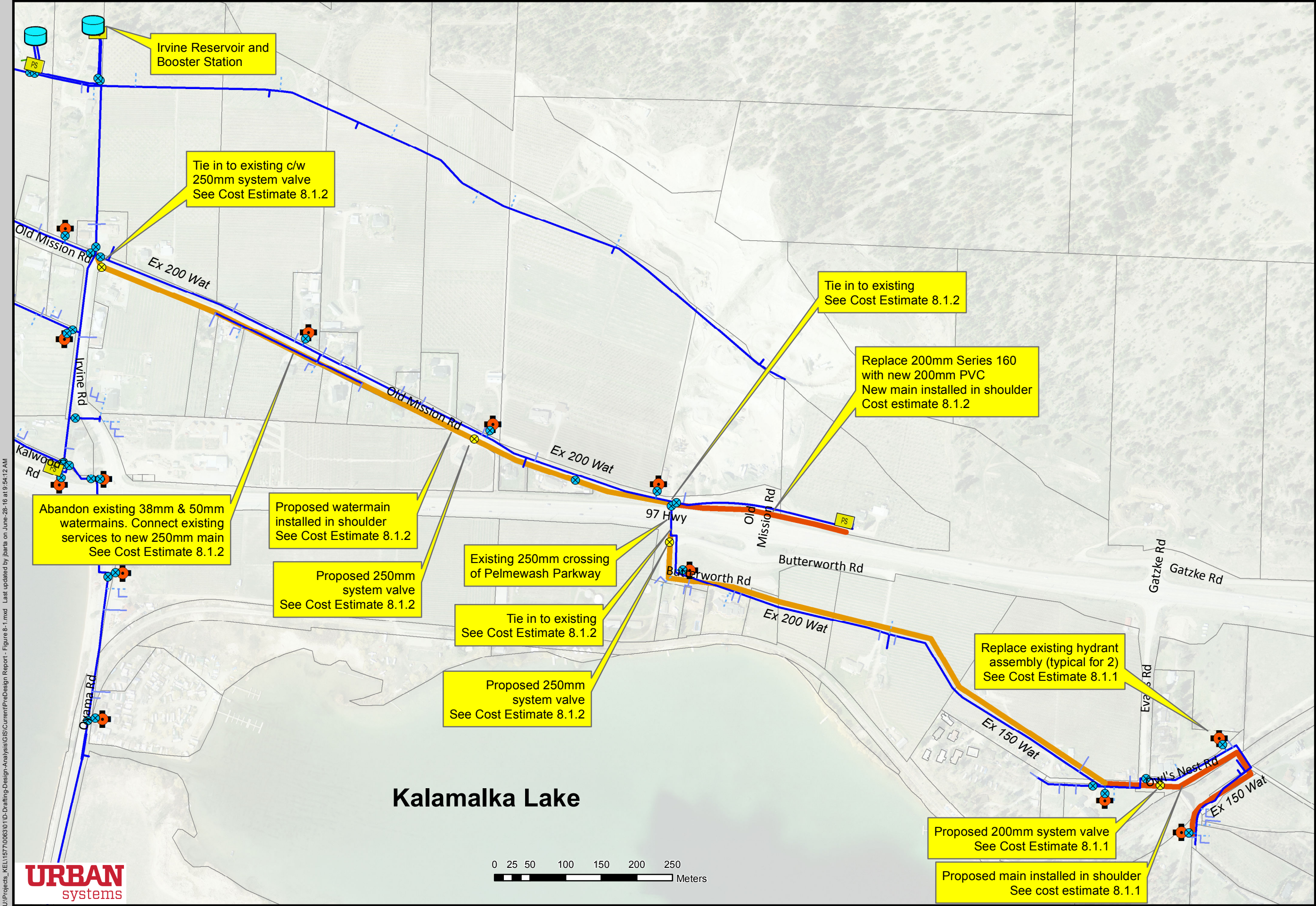
- Reservoir
- PS Pump Station
- Intake
- PRV Station
- System Valves
- Proposed System Valves
- Existing Hydrants

Existing Watermains

- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service

Watermain Upgrades

- 200mm
- 250mm



Kalamalka Lake



Owls Nest Area Upgrades Option #1

1:5,000
Mar 10, 2016

Figure 8-1

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Key Plan

Legend

Water Structures

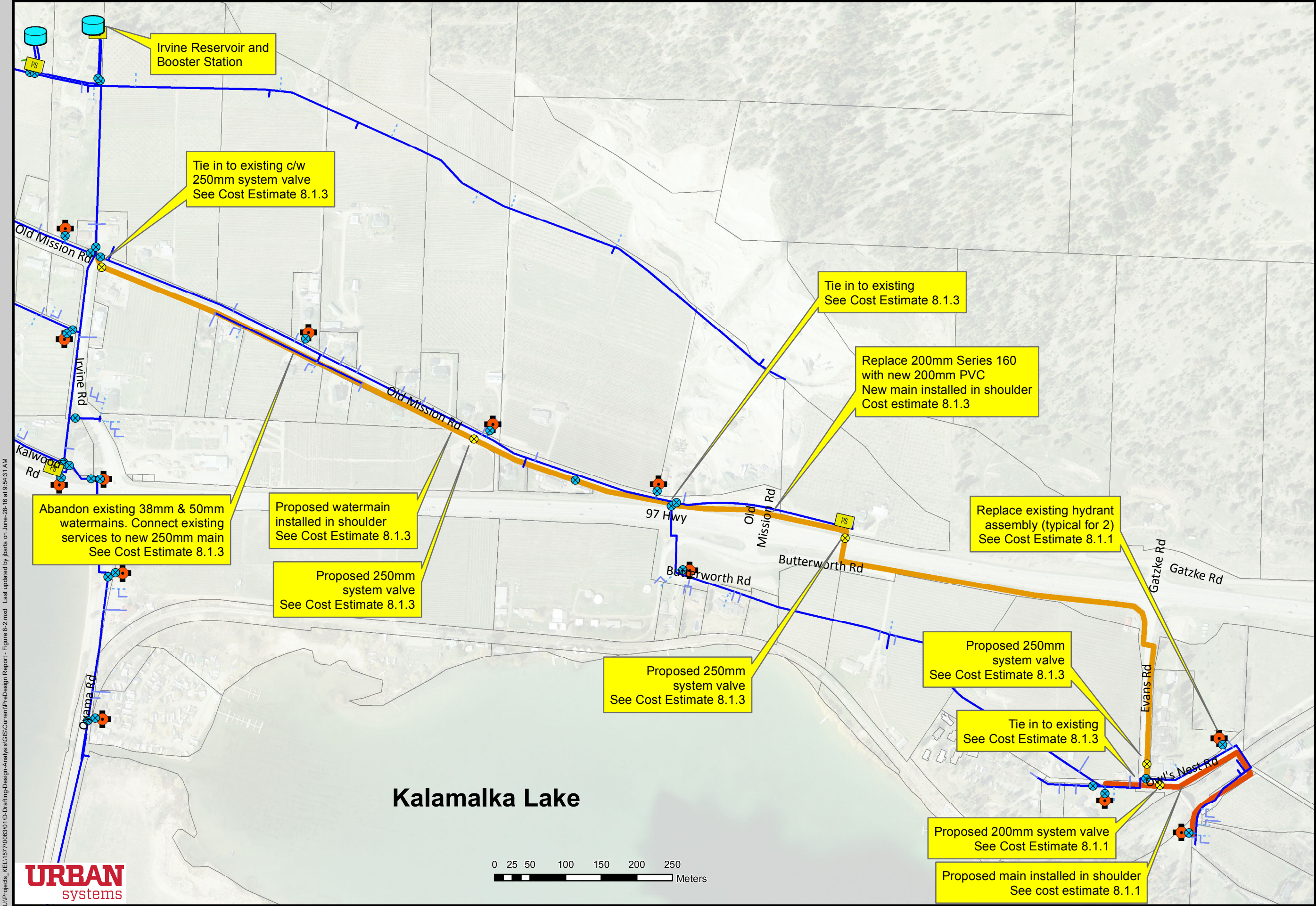
- Reservoir
- PS Pump Station
- Intake
- PRV Station
- System Valves
- Existing Hydrants
- Proposed System Valve

Existing Watermains

- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service

Watermain Upgrades

- 200mm
- 250mm



Owls Nest Area Upgrades Option #2
1:5,000
Mar 10, 2016

Figure 8-2

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4.9 Seaton Road Area

Urban Systems prepared a technical memorandum in September of 2014 to address servicing to 10011 Seaton Road as well as the existing undersized mains in the roadway. The memorandum has been included in **Appendix D** of the pre-design report. The estimated cost to construct these upgrades is **\$390,000** and is illustrated in **Figure 8-3**.

4.10 Woodsdale-Oceola Road Interconnect

The existing Oceola Road watermain south of Pretty Road is currently undersized being 100 mm in diameter. Supply to the area is currently being improved via a connection to the Lakes subdivision watermain at Okanagan Centre Road.

System redundancy, fire flow protection and water quality can be significantly improved by interconnecting the Oceola Road watermain to the Woodsdale Area. The proposed watermain should be 250 mm in diameter and will require a trenchless crossing of Highway 97. The existing fire hydrant near Turtle Bay pub should be replaced and a new hydrant assembly should be constructed at Woodsdale Court. A new 250 mm diameter mainline valve at each hydrant location should be added to assist the District with flushing operations.

The estimated cost to construct these upgrades is **\$1,170,000** and is illustrated in **Figure 8-4**.

4.11 Oyama Road Area

The Oyama Road Area is fed from the Oyama Lake source. Much of the Oyama Road water system is not looped and many adjacent roadways are fed via different PRV stations, making looping a difficulty.

One area that could be easily looped is Oyama Road, between Broadwater and Towgood Roads. This area is on the outskirts of the existing water system and the upgrade would still provide value to the District in the future when the area is converted over to the Beaver Lake source.

The minimum required size for the proposed watermain loop is 200 mm. The estimated cost to construct these upgrades is **\$950,000** and is illustrated in **Figure 8-5**.

Note that the Oyama Lake source distribution area review did not include assessing changes that will be required when the area is switched over to the Beaver Lake source in the future. It is our understanding that the District will complete a detailed assessment of the distribution system prior to doing this.



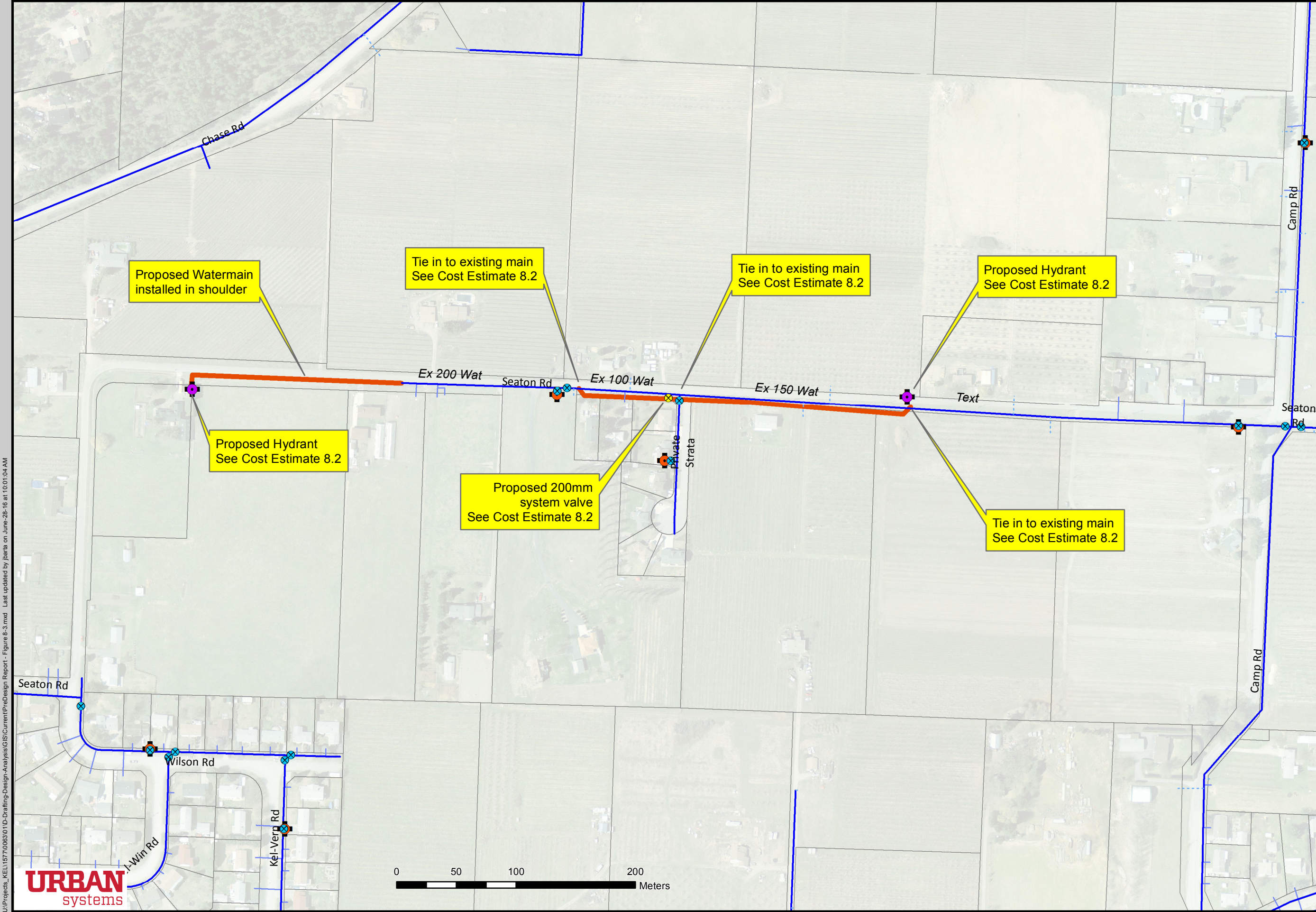
Key Plan

- Legend**
- Water Structures**
- Reservoir
 - PS Pump Station
 - Intake
 - PRV Station
 - System Valves
 - Proposed System Valves
 - Existing Hydrants
 - Proposed Hydrants
- Existing Watermains**
- Watermain
 - Hydrant Lead
 - Domestic Service
 - Irrigation Service
- Watermain Upgrades**
- 200mm

Seaton Road Area Upgrades

1:3,000
Mar 10, 2016

Figure 8-3



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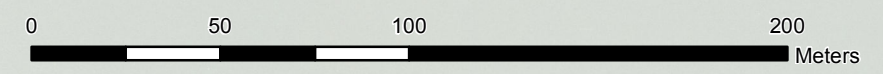
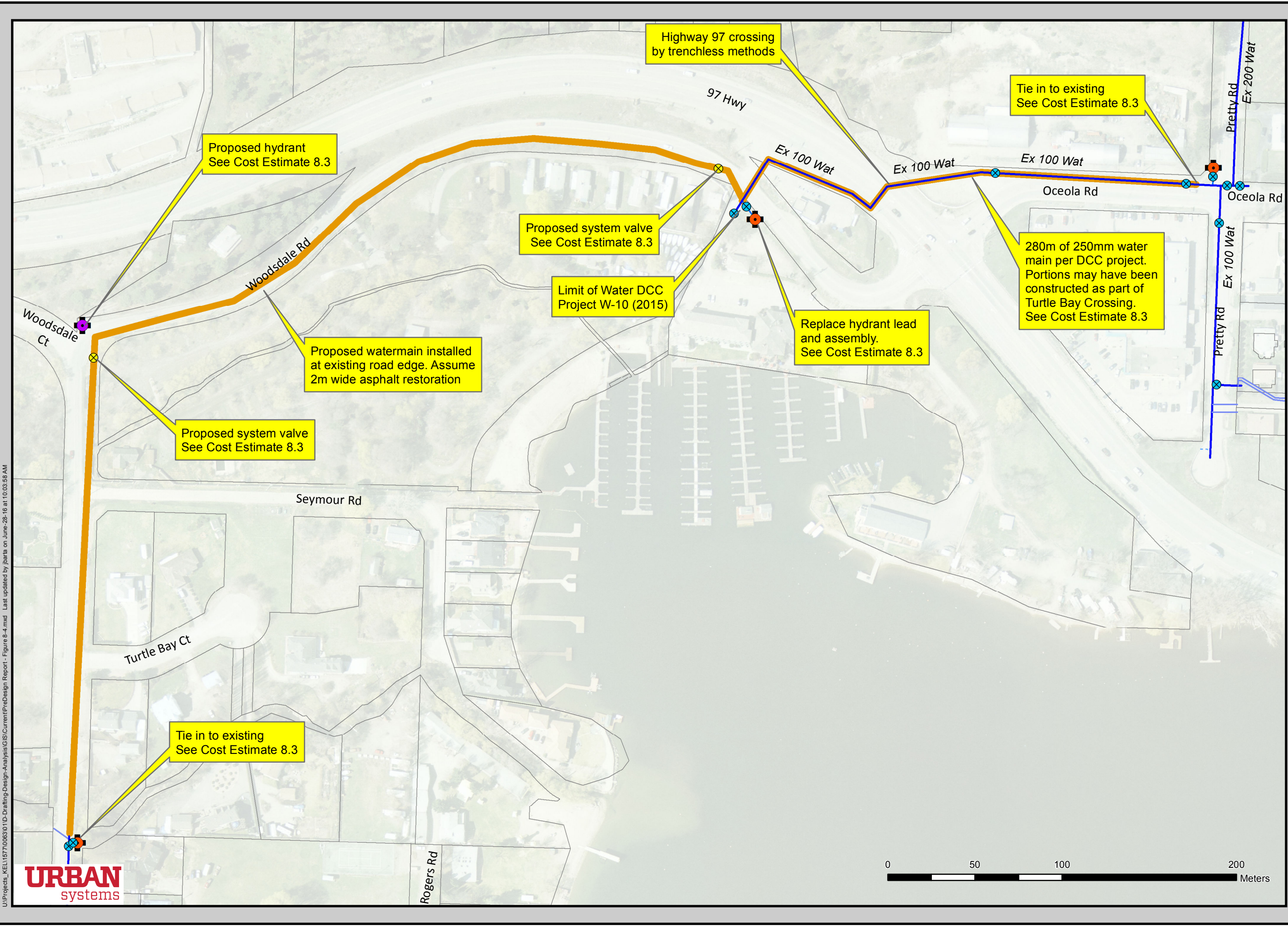
Key Plan

- Legend**
- Water Structures**
- Reservoir
 - PS Pump Station
 - Intake
 - PRV Station
 - System Valves
 - Existing Hydrants
 - Proposed Hydrants
- Existing Watermains**
- Watermain
 - Hydrant Lead
 - Drain Lines
 - Domestic Service
 - Irrigation Service
 - Proposed Valves
- Watermain Upgrades**
- 250mm

Oceola Road Woodsale Road Interconnect

1:2,000
Mar 10, 2016

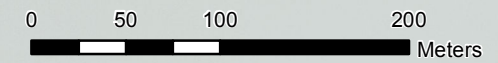
Figure 8-4



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Wood Lake



Key Plan

Legend

Water Structures

- Reservoir
- PS Pump Station
- Intake
- System Valves
- Proposed System Valves
- Existing Hydrants

PRV Stations

- PRV Station
- System Valves
- Proposed System Valves
- Existing Hydrants

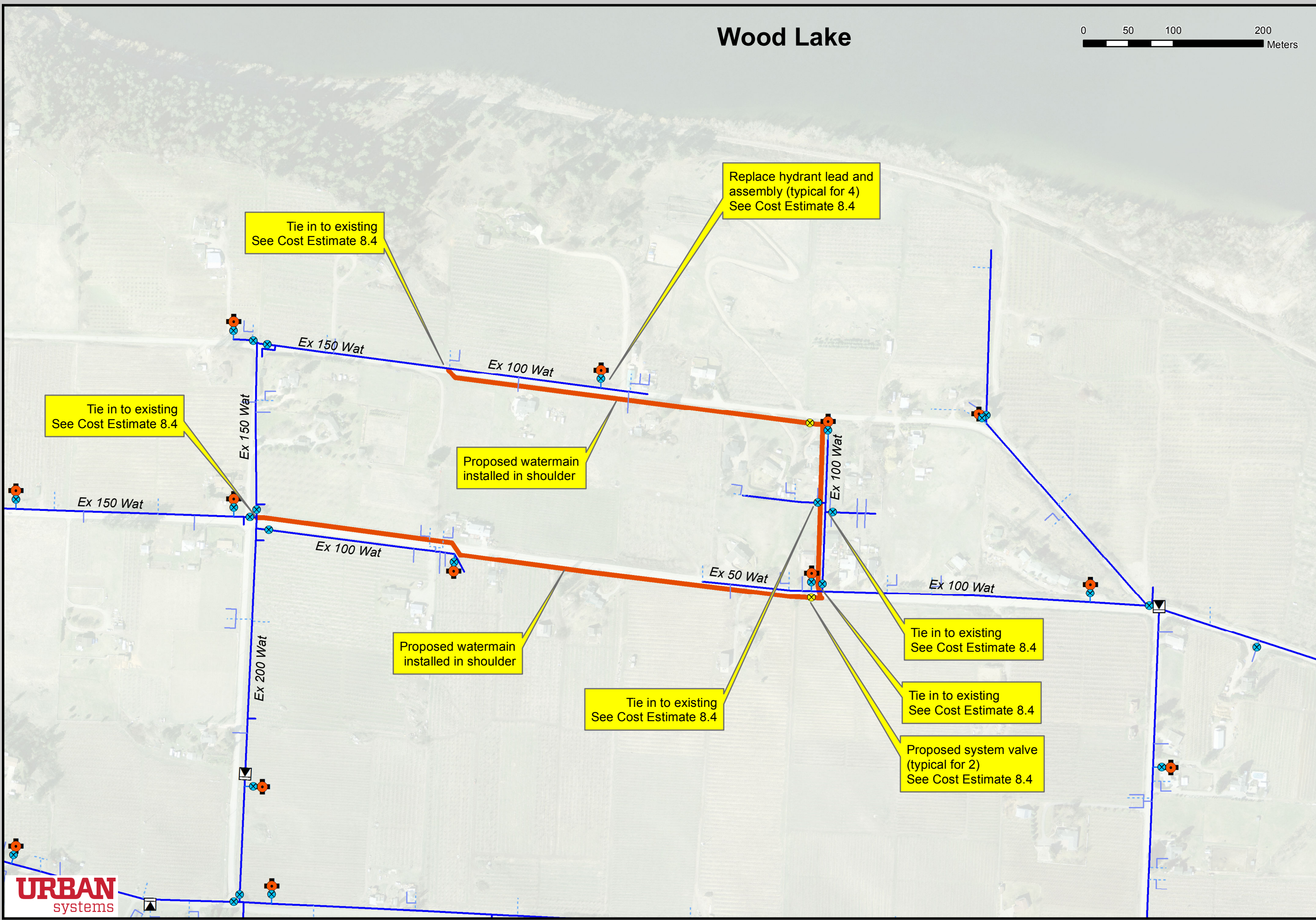
Existing Watermains

- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service

Watermain Upgrades

- 200mm

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Tie in to existing
See Cost Estimate 8.4

Tie in to existing
See Cost Estimate 8.4

Replace hydrant lead and
assembly (typical for 4)
See Cost Estimate 8.4

Proposed watermain
installed in shoulder

Proposed watermain
installed in shoulder

Tie in to existing
See Cost Estimate 8.4

Tie in to existing
See Cost Estimate 8.4

Tie in to existing
See Cost Estimate 8.4

Proposed system valve
(typical for 2)
See Cost Estimate 8.4



**Broadwater/
Oyama Road
Looping
Upgrades**
1:4,000
Mar 10, 2016

Figure 8-5

4.12 Miscellaneous Upgrades

The District provided additional mark-ups of locations within the water distribution system where there are undersized hydrant leads, watermains in poor condition and potential looping benefits. The following figures provide general locations for the District requests:

- Figure 8-6 General Upgrades – Oyama Area
- Figure 8-7 General Upgrades – Winfield Area
- Figure 8-8 General Upgrades – Winfield Area
- Figure 8-9 General Upgrades – Winfield Area
- Figure 8-10 General Upgrades – Winfield Area

Wood Lake



Key Plan

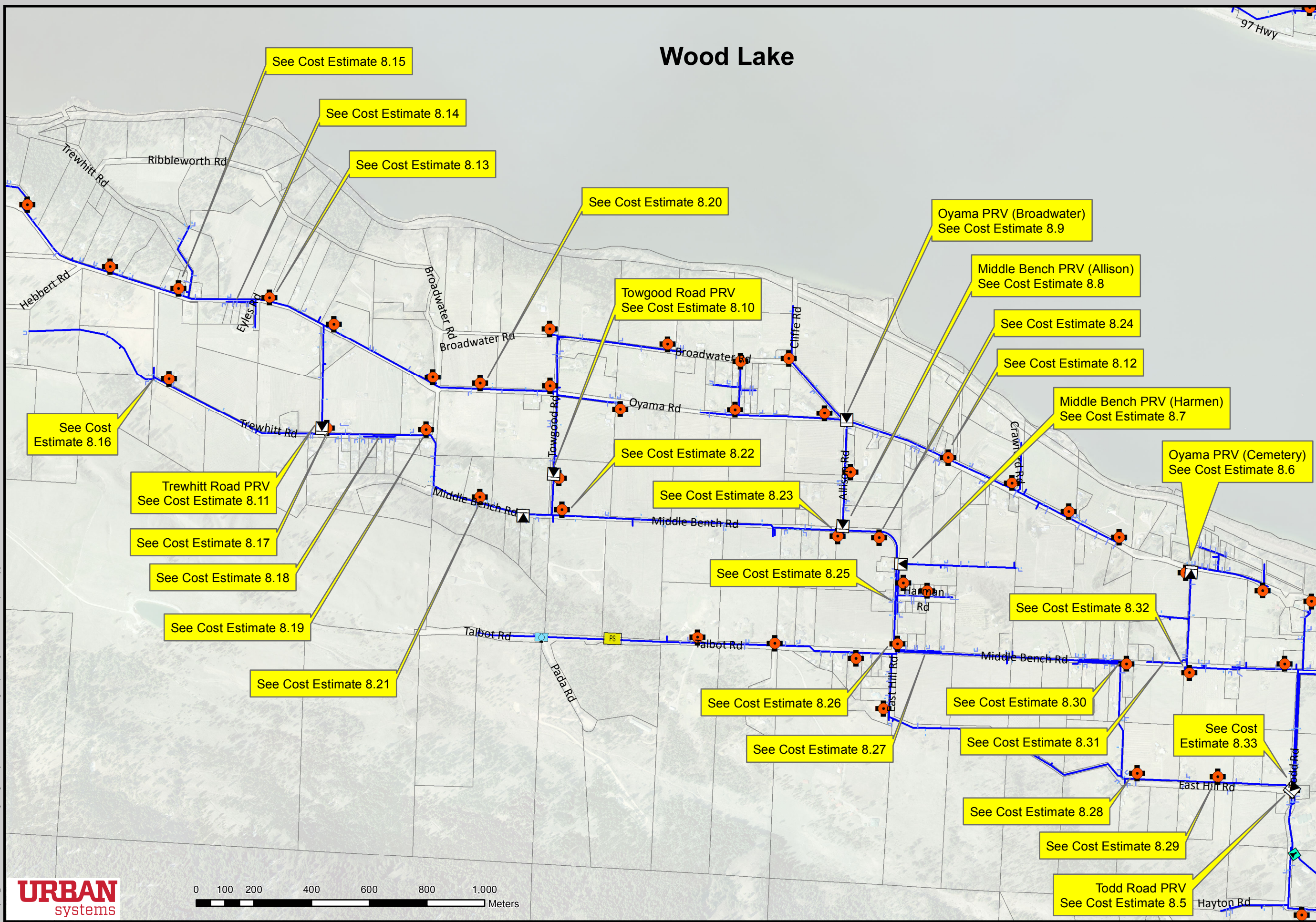
Legend

Water Structures

- Reservoir
- PS Pump Station
- Intake
- PRV Station
- Existing Hydrants

Existing Watermains

- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service



Oyama Water System Misc. Upgrades

1:12,500
Mar 10, 2016

Figure 8-6

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Key Plan

Legend

Water Structures

- Reservoir
- PS Pump Station
- Intake
- PRV Station
- Existing Hydrants

Existing Watermains

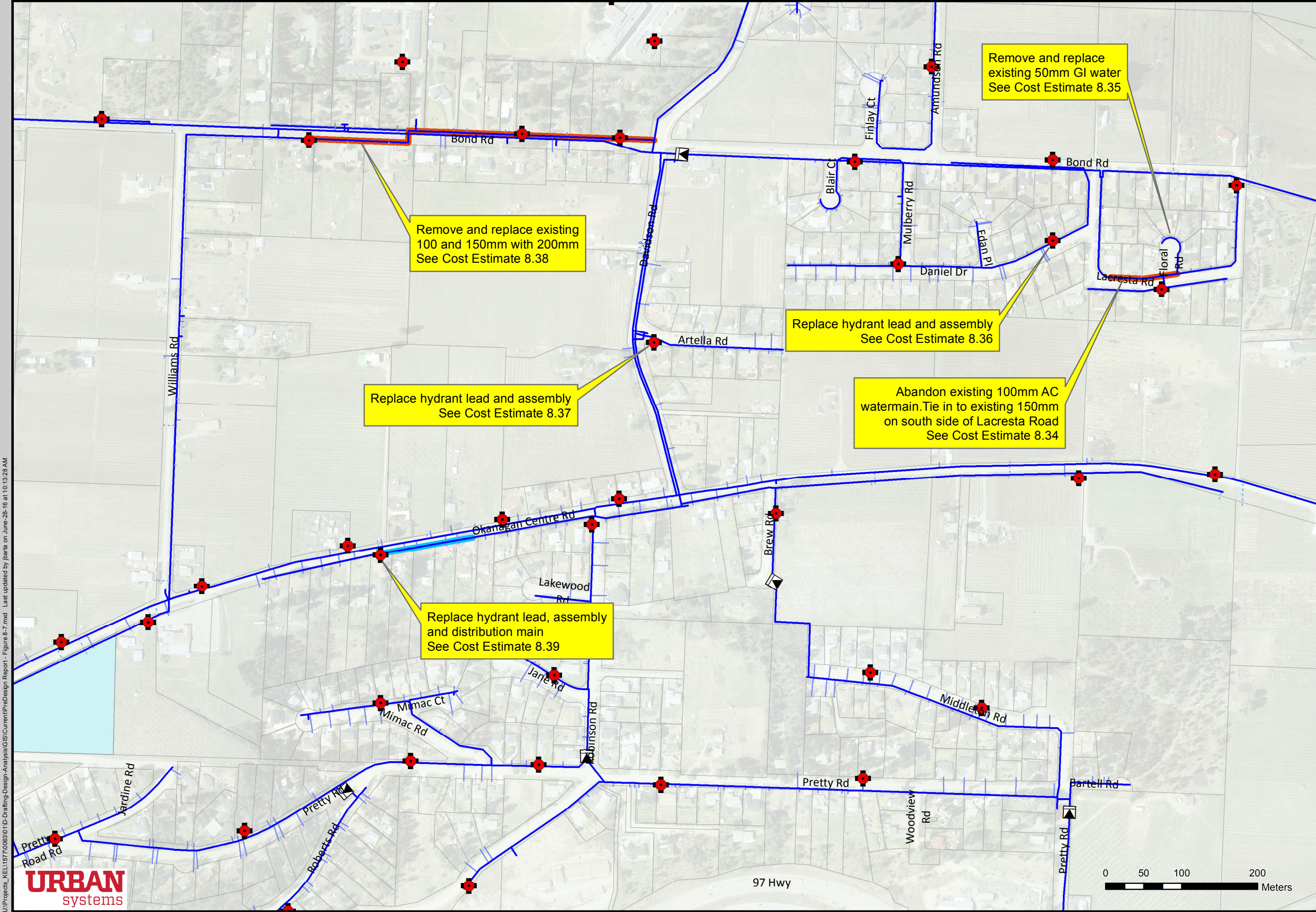
- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service

Watermain Upgrades

- 150mm
- 200mm

Beaver Lake System Misc. Upgrades
1:4,715
Mar 10, 2016

Figure 8-7



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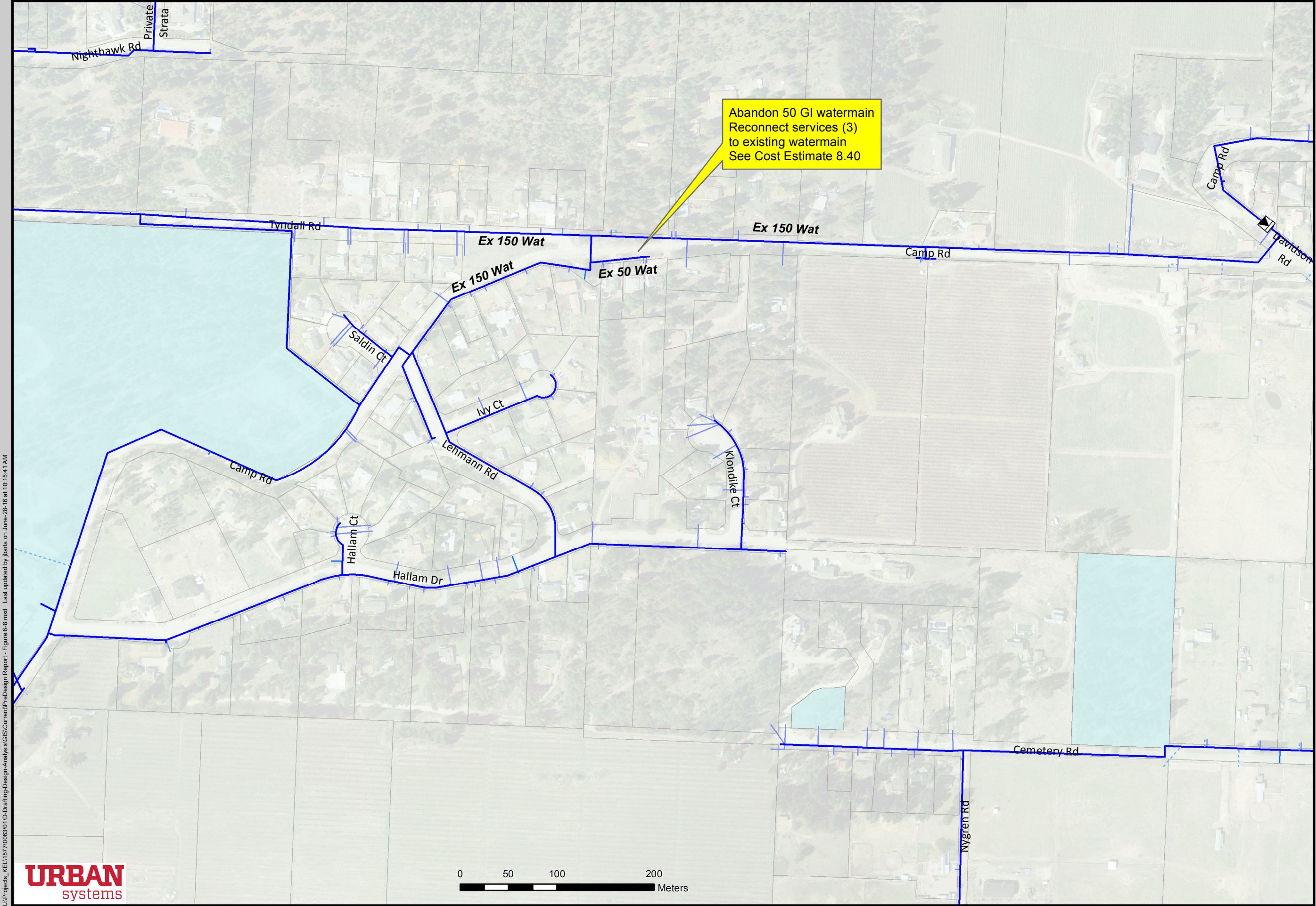
Key Plan

- Legend**
- Water Structures**
- Reservoir
 - PS Pump Station
 - Power Generation
 - Chlorinator
 - Intake
 - Pressure Tank
 - Cistern
 - PRV Station
- Existing Watermains**
- Watermain
 - Hydrant Lead
 - Domestic Service
 - Irrigation Service

Camp Road Watermain Upgrades

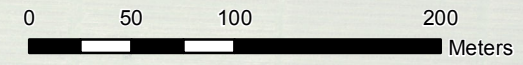
1:3,674
Mar 10, 2016

Figure 8-8



Abandon 50 GI watermain
Reconnect services (3)
to existing watermain
See Cost Estimate 8.40

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Key Plan

Legend

Water Structures

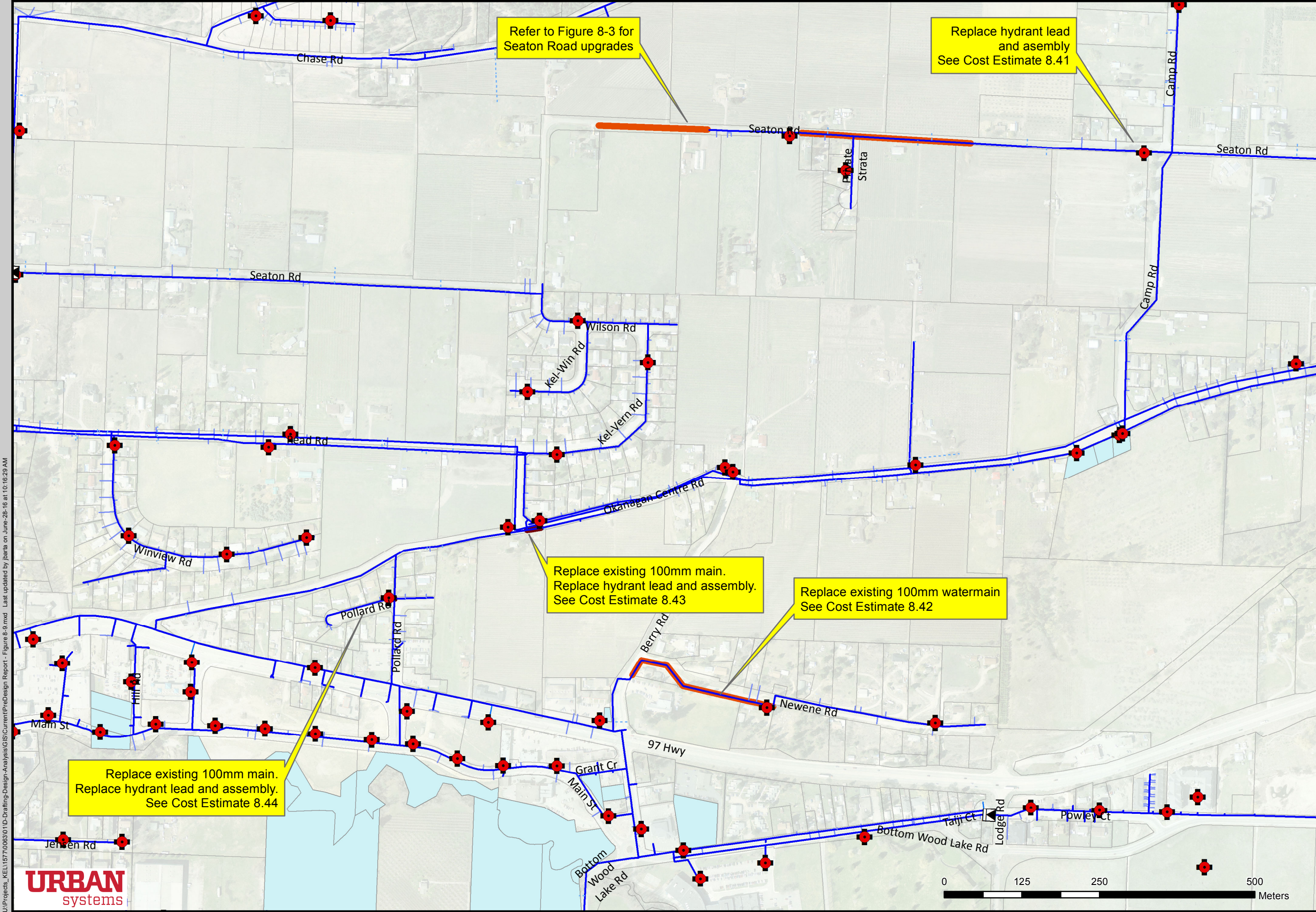
- Reservoir
- PS Pump Station
- Power Generation
- Chlorinator
- Intake
- Pressure Tank
- Cistern
- PRV Station
- Existing Hydrants

Existing Watermains

- Watermain
- Hydrant Lead
- Drain Lines
- Domestic Service
- Irrigation Service

Watermain Upgrades

- 150mm
- 200mm



Downtown Area Misc. Upgrades

1:5,766
Mar 10, 2016

Figure 8-9

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Key Plan

Legend

Water Structures

- Reservoir
- PS Pump Station
- Power Generation
- Chlorinator
- Intake
- Pressure Tank
- Cistern

PRV Stations

- PRV Station
- AirReleaseVacuum
- Existing Hydrants

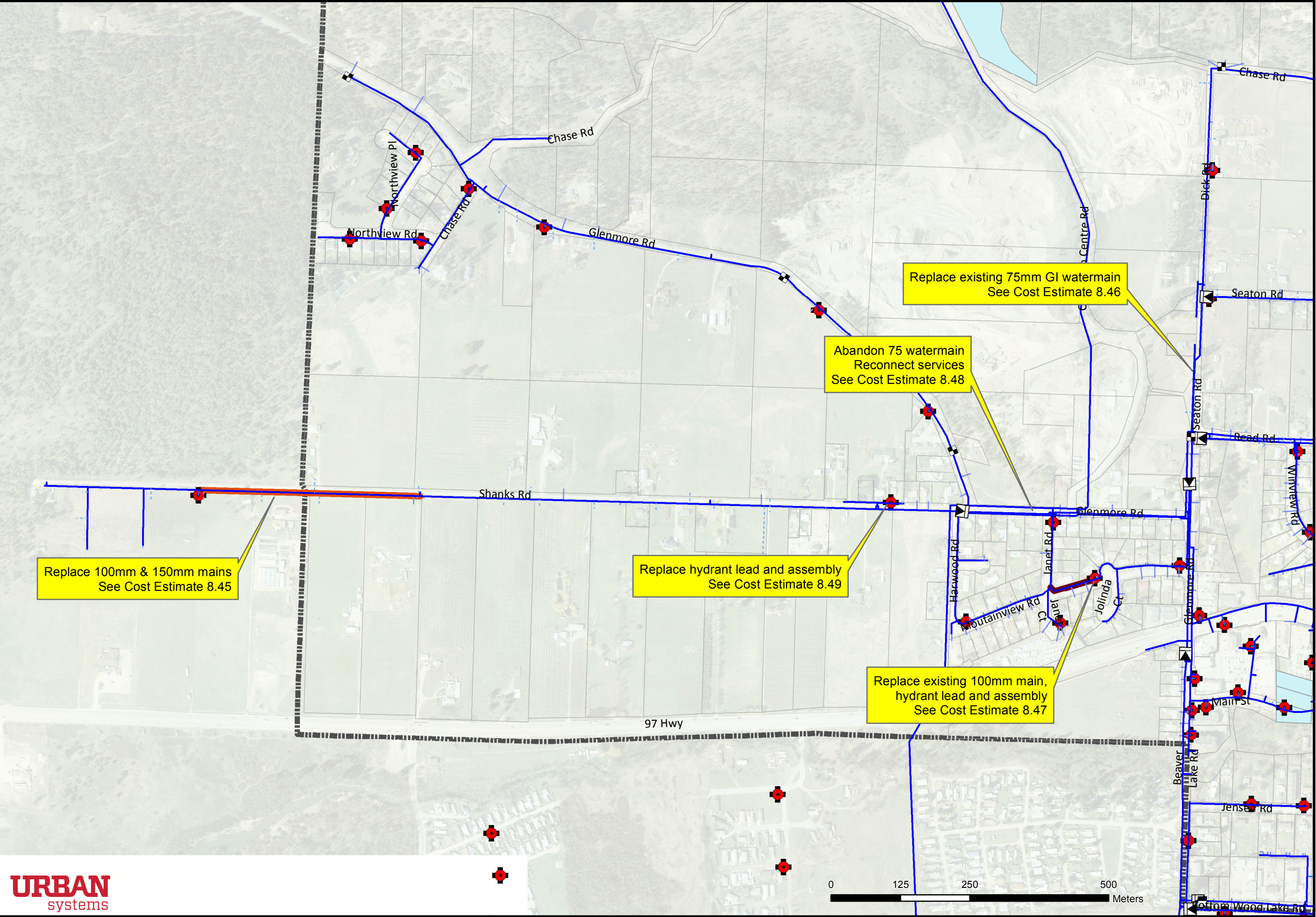
Existing Watermains

- Watermain
- Hydrant Lead
- Domestic Service
- Irrigation Service

Watermain Upgrades

- 150mm
- 200mm

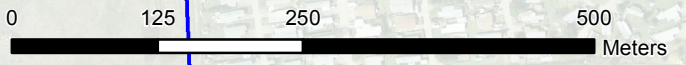
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Glenmore & Shanks Road Area Misc. Upgrades

1:6,489
Mar 10, 2016

Figure 8-10



5 Prioritized Capital Plan

Table 6 summarizes the required, alternative and optional costs associated with each of the water system areas.

Table 6: Upgrade Costing

Area	See Figure	Cost Estimate Option #1	Cost Estimate Option #2 ⁽¹⁾	Optional Replacements
Okanagan Centre Area	1-1, 1-2	4,300,000	\$4,020,000	\$2,180,000 ⁽²⁾
Oyama Isthmus Area	2	\$1,230,000	\$1,230,000	n/a
Irvine Boosted Zone	3	\$2,210,000	\$2,210,000	n/a
Cornwall Road Area	4-1, 4-2	\$1,430,000	\$1,290,000	n/a
Carrs Landing Area	5-1, 5-2, 5-3	\$2,580,000	\$2,520,000	\$590,000 ⁽³⁾
Ponderosa Road Area	6	\$670,000	\$670,000	n/a
Coral Beach Area	7	\$1,370,000	\$1,370,000	n/a
Owls Nest Road Area	8-1, 8-2	\$1,590,000	\$1,730,000	n/a
Seaton Road Area	8-3	\$400,000	\$400,000	n/a
Oceola-Woodsdale Interconnect	8-4	\$1,170,000	\$1,170,000	n/a
Oyama-Broadwater Looping	8-5	\$950,000	\$950,000	n/a
Sub-Total		\$17,900,000	\$17,560,000	\$2,770,000
Miscellaneous Projects	8-6 to 8-10	\$2,080,000	\$2,080,000	\$2,240,000 ⁽⁴⁾
Total		\$19,980,000	\$19,640,000	\$5,010,000

⁽¹⁾ Where applicable. Where second option does not exist, option #1 cost duplicated

⁽²⁾ Optional PRV stations and optional AC watermain replacement projects

⁽³⁾ Cost includes Carrs Landing watermain oversizing and optional Goldie PRV replacement

⁽⁴⁾ Cost includes all eight major Oyama System PRV station replacements

The upgrades have been ranked and prioritized based on the number of deficient hydrants that are connected with each specific upgrade. Refer to Table 7 below.

Table 7: Prioritized Projects

Upgrade Area	Number of Existing Deficient Hydrants in Area	Total Number of Existing and Proposed Hydrants in Area	Priority for Upgrade
Okanagan Centre	12	27	1
Oyama Isthmus	14	14	2
Carrs Landing Road	6 ⁽¹⁾	10	3
Cornwall Road	6 ⁽¹⁾	8	4
Owls Nest Road	7	7	5
Coral Beach	3	6	6
Oyama Road	4	4	7
Irvine Boosted Zone	3	5	8
Ponderosa Road	3	3	9
Seaton Road	2	3	10
Woodsdale-Oceola	2	3	11

The prioritization of the major projects listed in table 7 above is a risk based on service area. Other factors that may change the priority of each project could be:

- Ability to combine with other capital works projects (roads and sewer);
- Available resources; or
- Watermain failure(s) within the project area.

In addition, a complete listing of all miscellaneous projects has been included in **Appendix G**. It is recommended that the District prioritize these upgrades.

6 Closure

This pre-design report identifies areas where the District can upgrade the water system to improve fire protection and water quality in a phased manner that aligns with the Water Master Plan.

Please contact the undersigned if you have any questions or require any clarification of the above information.

URBAN SYSTEMS LTD.

Reviewed by:



Jason Barta, B.Sc.
Municipal Infrastructure Analyst



Jeremy Clowes, P.Eng.
Water Engineer

/jb

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Appendix A

Areas Reviewed for the Small Diameter Watermains Analysis

TECHNICAL MEMORANDUM

Date: March 10, 2016
File: 1577.0063.01
Subject: Appendix A – Areas Reviewed for Small diameter Watermain Analysis
Page: 1 of 3



Date: March 10, 2016
To: Greg Buchholz, Kiel Wilkie; District of Lake Country
cc: Jeremy Clowes, Steve Brubacher; Urban Systems
From: Jason Barta
File: 1577.0063.01
Subject: **AREAS REVIEWED FOR SMALL DIAMETER WATERMAIN ANALYSIS**

This memo outlines the results of the data review and identifies high priority data gaps requiring further clarification or investigation by District staff.

Furthermore, this document sets out the analysis criteria to be used in conjunction with the hydraulic water models to determine existing and growth related deficiencies in the water system.

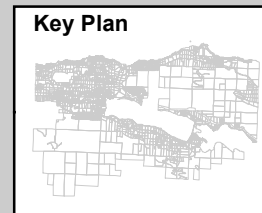
Scope of Analysis

The District of Lake Country (District) provided a list of six (6) discrete areas within the municipal boundary that are deficient in available fire flow as listed below:

- Okanagan Centre Area (Hare Road and to the west);
- Oyama Isthmus;
- Irvine Boosted Zone;
- Cornwall Road Area;
- Carrs Landing Area; and
- Ponderosa Road Area

These discrete areas are illustrated in **Figure A**. The District also requested a review of the Coral Beach water system.

Note: the fire flow analyses performed under this project focus only on the hydraulic limitations of the distribution system (main sizing and PRV capacities). A review of available supply (treated water storage and fire flow duration) is outside the scope of this work program.



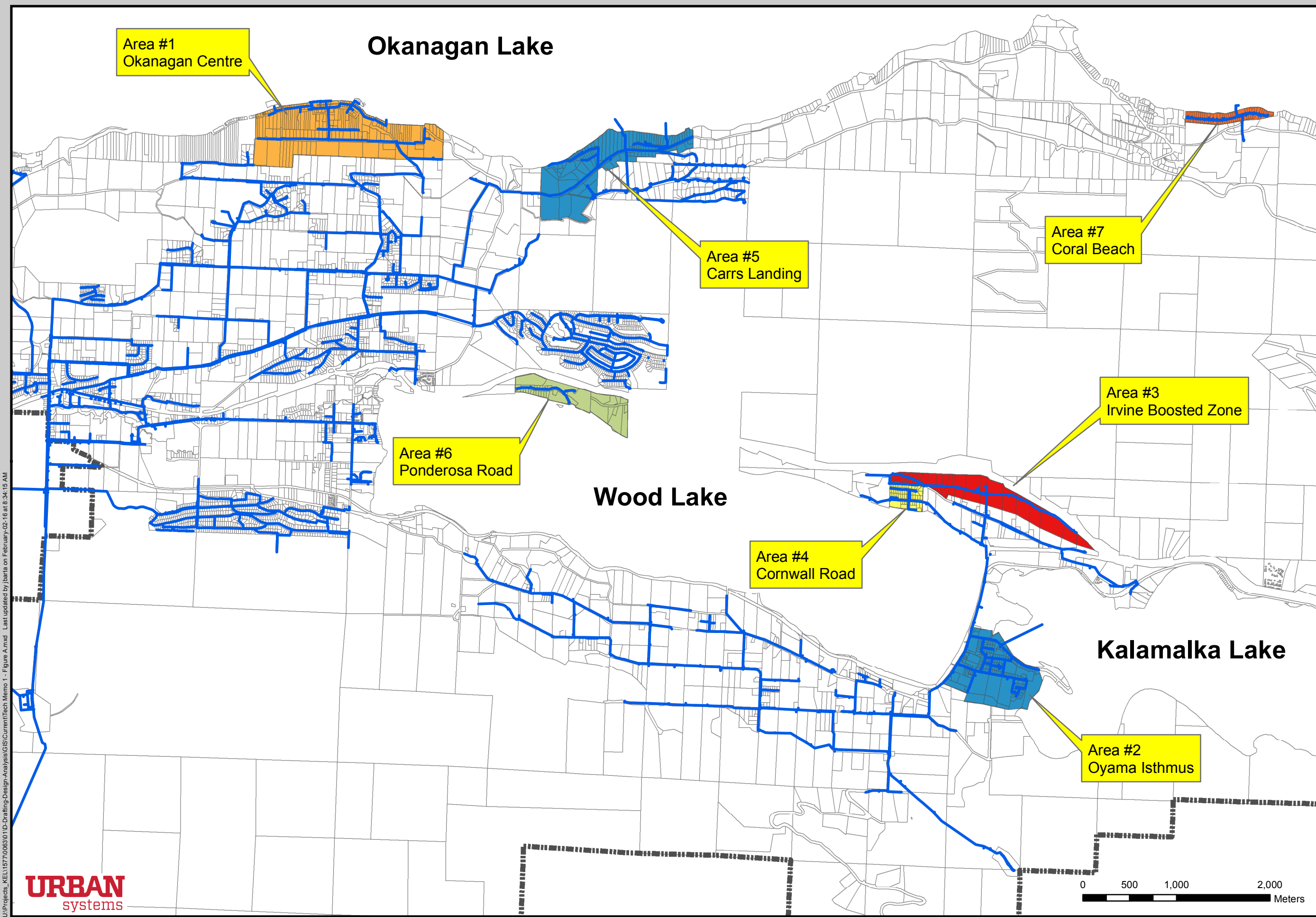
Legend

— Watermain

Analysis Areas

AreaLabel

- Carrs Landing
- Coral Beach
- Cornwall Road
- Irvine Boosted Zone
- Okanagan Centre
- Oyama Isthmus
- Ponderosa Road



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Small Diameter Watermain Analysis
 1:38,229
 January 18, 2016
 DRAFT
Figure A

TECHNICAL MEMORANDUM

Date: March 10, 2016
File: 1577.0063.01
Subject: Appendix A – Areas Reviewed for Small diameter Watermain Analysis
Page: 2 of 3



Data Review

Urban Systems collaborated with the District to update their GIS infrastructure datasets during 2011 through 2013. The same data was used to construct hydraulic water models for the Beaver/Okanagan Lake systems as well as the Kalamalka/Oyama Lake systems.

The Ravenridge (Lakepine) and Alto water systems were incorporated into the Beaver/Okanagan Lake hydraulic water model. The Coral Beach water system has not been modeled.

The marked-up maps provided by the District also included some corrections to the piping and valve infrastructure. The hydraulic models were updated to match the mark-ups.

Based on our review, the level of detail and quality of the data received is sufficient to undertake the hydraulic analyses and identify system deficiencies.

Analysis Criteria

The majority of the Districts small diameter watermains are located in established residential and rural neighbourhoods, which typically require only a minimum level of fire protection. The Districts' Subdivision and Development Servicing Bylaw #97-139 requires new subdivisions to provide a minimum fire flow of 3,600 L/min, unless additional flow is required per Fire Underwriters Survey (FUS) guidelines.

However, as noted in the start-up meeting notes - circulated January 7th, 2015 – the District has requested that the minimum available fire flow be set at 5,000 L/min for residential areas.

The same meeting notes also requested that the minimum fire flow to be provided to the Packing House area of 4th Street in the Okanagan Centre area, shall not be less than 10,000 L/min.

Update: The District has reduced the fire flow requirement for the Okanagan Centre area (Packing House) to 5,000 L/min since the fire department has the ability to supplement fire flow by pumping from Okanagan Lake.

All fire flow analyses will be conducted per the following criteria from the Districts current Subdivision and Development Servicing Bylaw #97-139 and its amendments:

- Fire flow demands shall be in addition to Maximum Day Demands (MDD);
- Minimum residual pressure at fire flow conditions 140 Kpa; and
- Maximum velocity in pipes under MDD+FF conditions shall not exceed 4.0 meters per second

Watermain looping shall be investigated, where possible, to minimize existing watermain upsizing.

TECHNICAL MEMORANDUM

Date: March 10, 2016
File: 1577.0063.01
Subject: Appendix A – Areas Reviewed for Small diameter Watermain Analysis
Page: 3 of 3



Cost Estimates

The cost estimates provided in the pre-design report and technical memorandums will vary depending on the type of infrastructure improvement. Typically, for high level analysis with limited site data, a class 'D' estimate is prepared.

Watermain upgrade cost estimates shall be provided at a Class 'C' level due to the uncertainty of other utilities in the ground and a lack of geotechnical information for each project. The estimates will include a 25% contingency allowance as well as 15% for engineering and construction.

Facility upgrades, such as a new PRV station, have fewer unknowns and uncertainties than in-ground pipe infrastructure projects. As such, facilities estimates will include a 20% contingency allowance as well as 15% for engineering and construction.

Cost estimates will not include land costs (such as securing new easements for a new watermain loop or purchasing land outside of the road right-of-way to construct a new above-ground PRV station on).

Please contact the undersigned if you have any questions or require any clarification of the above information.

URBAN SYSTEMS LTD.

Reviewed by:

A handwritten signature in blue ink, appearing to be 'J. Barta'.

Jason Barta, B.Sc.
Municipal Infrastructure Analyst

A handwritten signature in blue ink, appearing to be 'Jeremy Clowes'.

Jeremy Clowes, P.Eng.
Water Engineer

/jb

Appendix B

Available Fire Flow Mapping

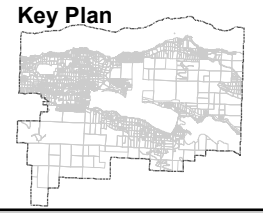
Available fire flows are theoretical maximum flows from the node adjacent to the hydrant with a residual pressure of 140 kPa (20psi). Specific hydrant flow losses may limit the available fire flow to less than those values listed.

Okanagan Lake

Ottley Rd

PS

PS



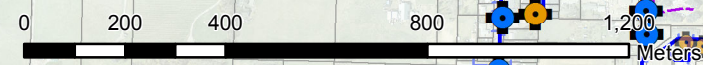
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- District Hydrants**
- No Flow Data
 - 0-499 IGPM
 - 500-999 IGPM
 - 1000-1499 IGPM
 - 1500+ IGPM
- Private Hydrants**
- No Flow Data
 - 0-499 IGPM
 - 500-999 IGPM
 - 1000-1499 IGPM
 - 1500+ IGPM

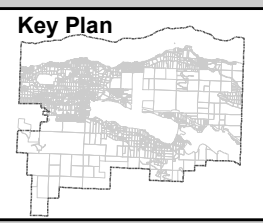
WATER SYSTEM FIRE FLOW

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MAP SERIES

MAP FF-1



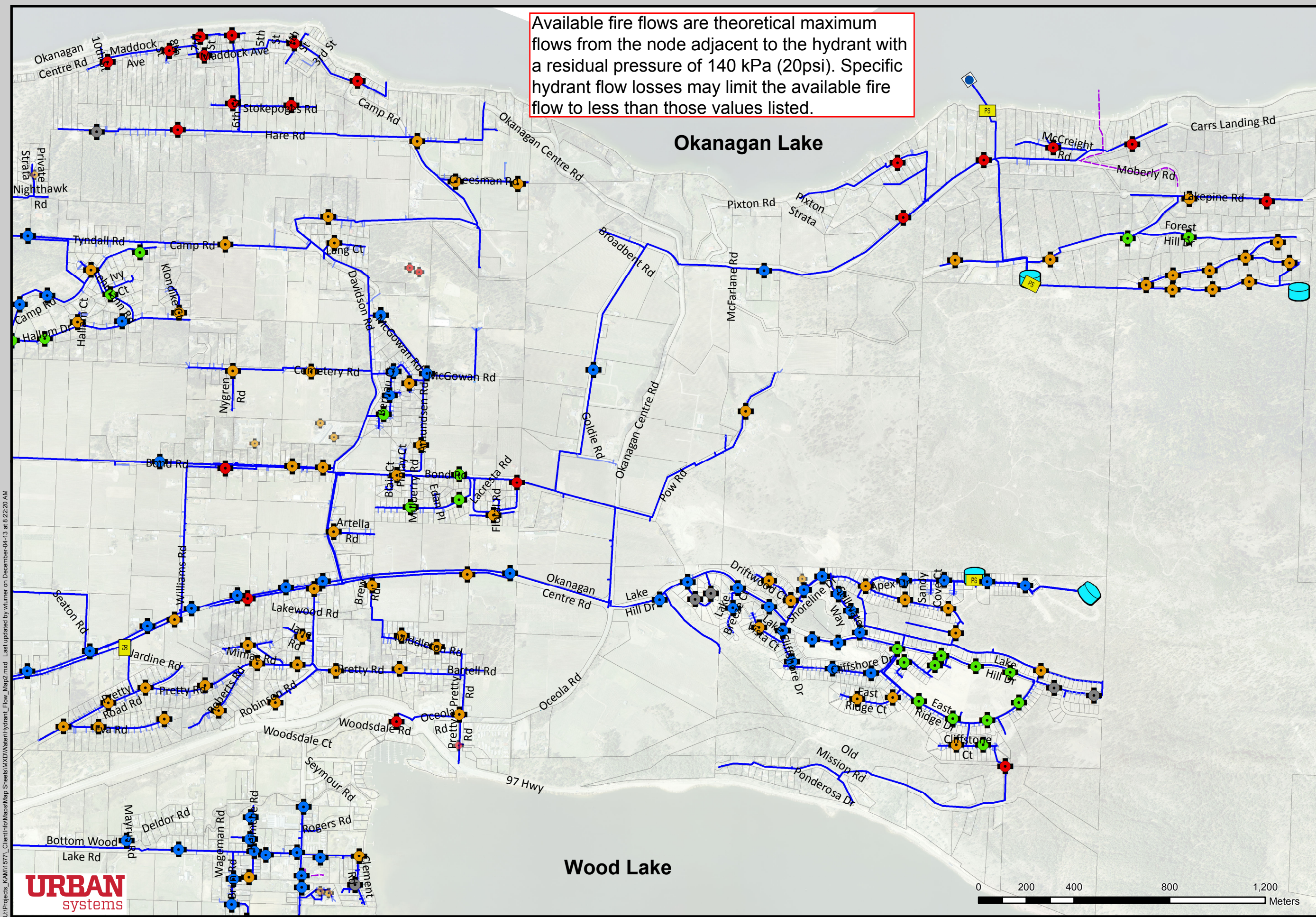
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Available fire flows are theoretical maximum flows from the node adjacent to the hydrant with a residual pressure of 140 kPa (20psi). Specific hydrant flow losses may limit the available fire flow to less than those values listed.

Okanagan Lake

Wood Lake



Legend

District Hydrants

- No Flow Data
- 0-499 IGPM
- 500-999 IGPM
- 1000-1499 IGPM
- 1500+ IGPM

Private Hydrants

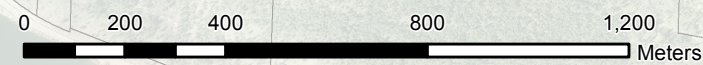
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- 1500+ IGPM

WATER SYSTEM FIRE FLOW

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MAP SERIES

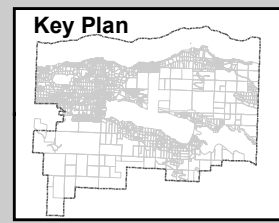
MAP FF-1



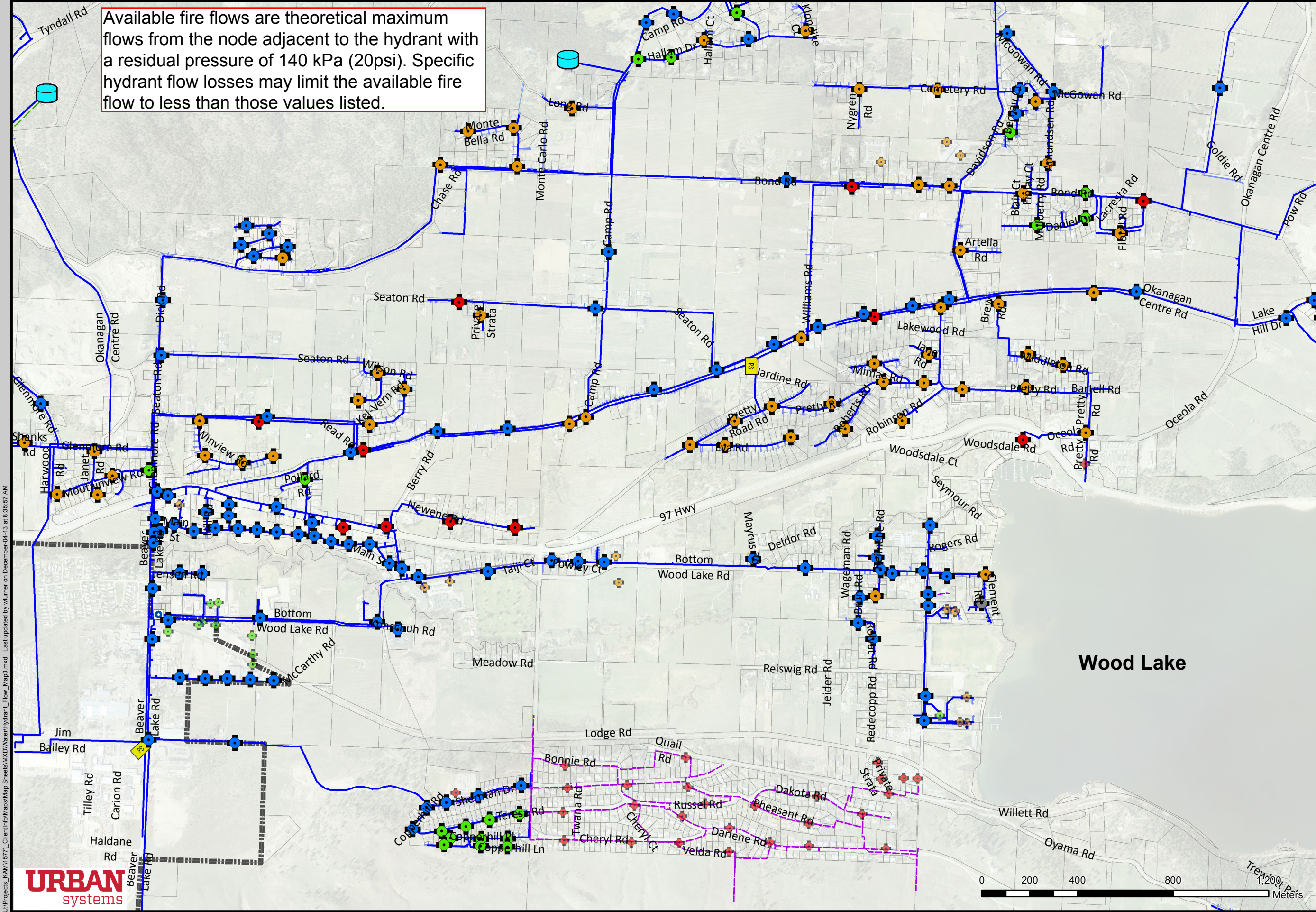
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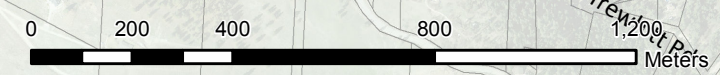
Available fire flows are theoretical maximum flows from the node adjacent to the hydrant with a residual pressure of 140 kPa (20psi). Specific hydrant flow losses may limit the available fire flow to less than those values listed.



- Legend**
- District Hydrants**
- No Flow Data
 - 0-499 IGPM
 - 500-999 IGPM
 - 1000-1499 IGPM
 - 1500+ IGPM
- Private Hydrants**
- No Flow Data
 - 0-499 IGPM
 - 500-999 IGPM
 - 1000-1499 IGPM
 - 1500+ IGPM



Wood Lake



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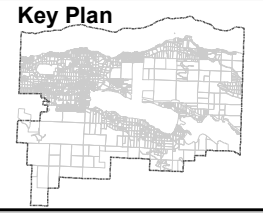
WATER SYSTEM FIRE FLOW

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MAP SERIES

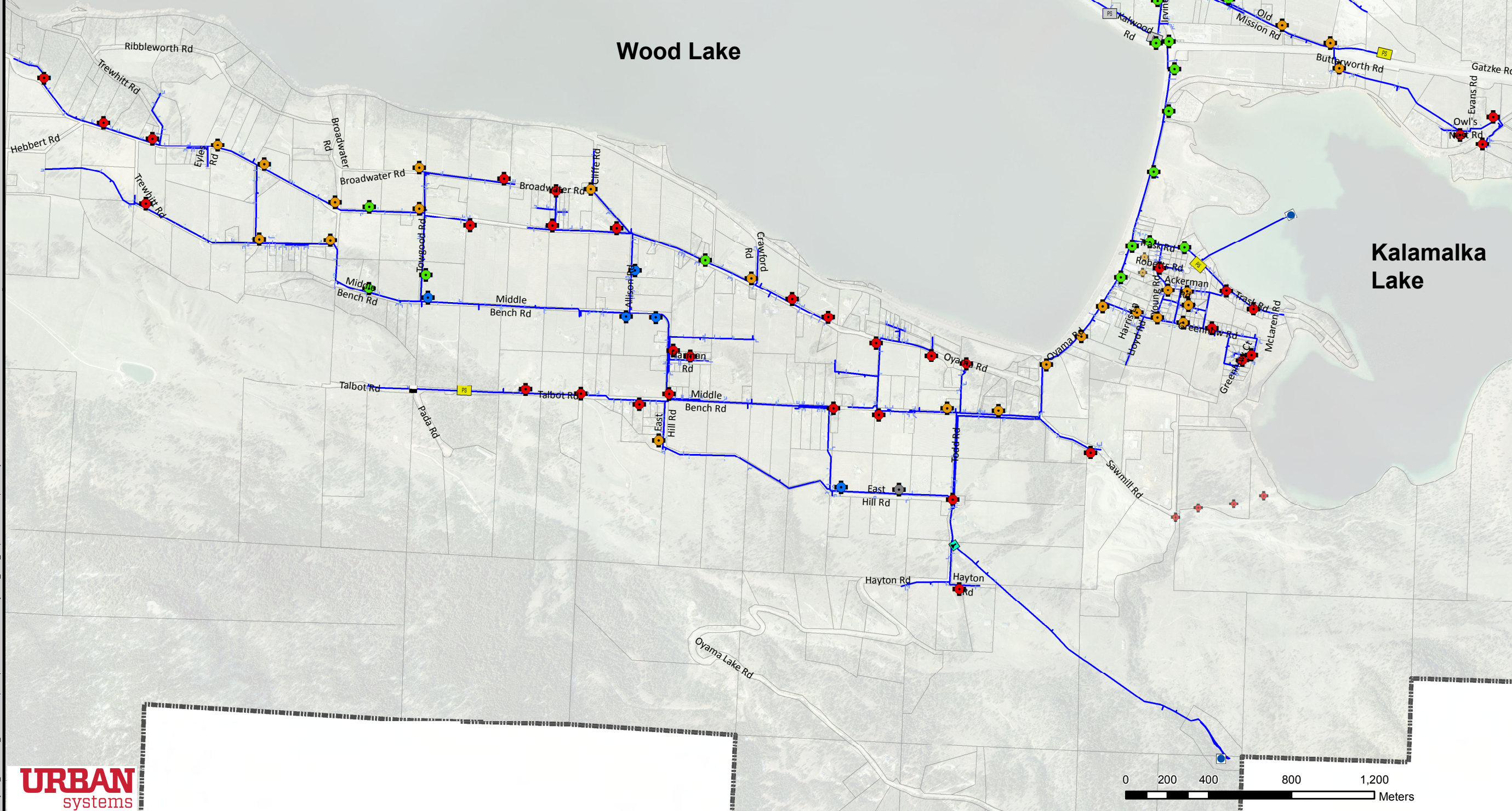
MAP FF-3

Available fire flows are theoretical maximum flows from the node adjacent to the hydrant with a residual pressure of 140 kPa (20psi). Specific hydrant flow losses may limit the available fire flow to less than those values listed.



Wood Lake

Kalamalka Lake



- Legend**
- District Hydrants**
- No Flow Data
 - 0-499 IGPM
 - 500-999 IGPM
 - 1000-1499 IGPM
 - 1500+ IGPM
- Private Hydrants**
- No Flow Data
 - 0-499 IGPM
 - 500-999 IGPM
 - 1000-1499 IGPM
 - 1500+ IGPM



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WATER SYSTEM FIRE FLOW

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MAP SERIES
MAP FF-4

Appendix C

Kalamalka Distribution System Upper Pressure Zone Review (Urban Systems)



Kalamalka Lake Water Distribution System – Upper Pressure Zone Review



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URBAN
systems

1577.0046.01
January 2013

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Contact: **Jeremy Clowes**
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REPORT

Kalamalka Lake
Water Distribution
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Pressure Zone
Review

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1.0 Introduction

Urban Systems Ltd. (Urban) was retained by the District of Lake Country (DLC) to review the Upper Pressure Zone (UPZ) located within the Kalamalka Lake Water Distribution System. The UPZ, which provides service to approximately 100 residents and has 18 irrigation services, is supplied by two booster stations that are located in buried chambers and draw water from the Irvine Reservoir. The two main objectives of the study are to:

- Identify any deficiencies within the UPZ distribution network (e.g., water quality concerns, inadequate fire protection, etc.)
- Complete the preliminary design of an above ground booster station to replace both existing booster stations and improve working conditions for DLC operators.

This report starts by providing an overview of the existing UPZ, then moves on to identify design criteria in Section 3, presents findings from the distribution network review in Section 4 and reviews the preliminary booster station design in Section 5. The report ends with cost estimate for the overall project.

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2.0 Existing System

2.1 Demands

The extents of the UPZ service area are shown in Figure 1. There are 38 residential and 18 irrigation services. The demands for the UPZ are summarized in Table 2-1. The methodology followed by Urban to establish the demands of the UPZ is included in Appendix 2-1. Irrigation demands account for over 95% of the maximum day demand. As such, the demands vary significantly throughout the year based on the seasonal irrigation use. There are three fire hydrants located at the south end of the UPZ. This area of UPZ contains single detached dwelling units. Accordingly, we have based our review of the distribution network on being able to deliver a minimum fire flow of 60 L/s which is consistent with the *MMCD Design Guideline Manual 2005* and also the minimum value permitted in the *DLC Subdivision and Development Servicing Bylaw No.97 – 139* (DLC SDSB).

Table 2-1 – UPZ Demands

Scenario	Demand (L/s)
Winter Average Day Demand ¹	0.4
Average Day Demand (ADD) ²	2.3
Maximum Day Demand (MDD)	46.0
Peak Hour Demand (PHD)	46.0
PHD – Domestic Only	3.1
Minimum Fire Flow	60

Notes:

1. Based on domestic indoor use
2. Based on domestic indoor and domestic irrigation uses

2.2 Distribution Network

The UPZ distribution network contains approximately 2.8 km of watermain which varies in size from 100 to 200 mm. As shown in Figure 1, the residential area within the UPZ is serviced by 100 mm watermain.

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2.3 Booster Stations

The locations of the two booster stations that service the UPZ are shown on Figure 1. The Site 2 Booster Station supplies the UPZ under normal operating conditions whereas the Site 1 Booster Station is used as backup to the Site 2 Booster and to supplement fire flows. A brief summary each station is included below.

Site 2 Booster Station

The Site 2 Booster Station consists of a buried chamber structure that was constructed in 1996 and is equipped with a 40 HP inline booster pump and a 3 HP end suction pump.

Site 2 Booster Station



The structure and mechanical equipment appear to be in good condition but the buried chamber causes a number of concerns most notable of which are listed below:

- **Safety** – The station is only accessible through a hatch a ladder that is approximately 4 m long. If a staff member was injured inside the chamber, the restricted means of entry will make any emergency response action more complicated to execute.
- **Operation and Maintenance** – The chamber is a confined space as defined by the Occupational Health and Safety Regulation. Entering a confined space versus a non-confined space requires additional staff, safety equipment and training to complete routine tasks.
- **Risk of Facility Being Damaged** – If the chamber was ever flooded (from a natural event, pipe leak, etc.), it is likely that all electrical equipment will have to be replaced. The repairs to bring the station back online would

be time consuming, costly and impact DLCs ability to maintain service to users in the UPZ.

Site 1 Booster Station

The Site 1 Booster Station is equipped with a 60 HP horizontal split case pump and a 1.5 HP multi-stage booster pump. This facility is older than the Site 2 booster and the mechanical equipment is nearing the end of its service life.

Site 1 Booster Station



The Site 1 Booster Station is also a buried chamber which raises the same concerns as identified above for the Site 2 Booster Station.

Site 1 Booster Station



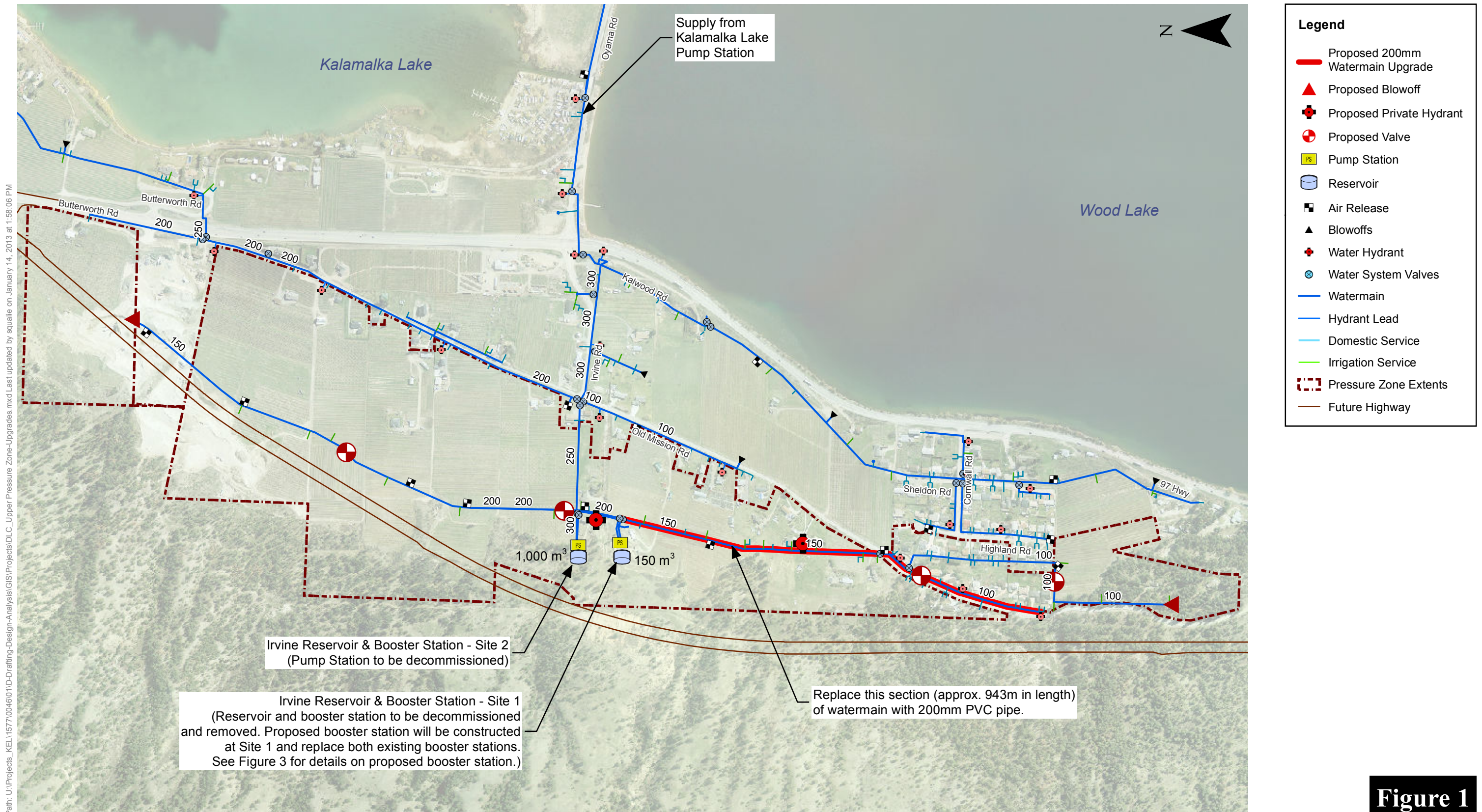
REPORT

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LAKE COUNTRY
Life. The Okanagan Way.

Kalamalka Lake Distribution System Upper Pressure Zone - Proposed Upgrades



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Figure 1

REPORT

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3.0 Design Criteria

Urban's review of the UPZ distribution network and preliminary booster station design is based on criteria identified in the following documents:

- DLC Subdivision and Development Servicing Bylaw No.97-139
- MMCD Design Guideline Manual 2005
- Hydraulic Institute Standards
- Fire Underwriters Survey Water Supply for Public Fire Protection 1999
- American Water Works Association

Specific sizing criteria that we followed are summarized below:

Pump Sizing

The proposed pumps have been designed to meet existing demands as presented in Table 2-1. This ensures that pumps will operate as efficiently as possible for existing conditions and can be upsized if future development occurs within the UPZ.

Watermain Sizing

All watermain (including booster station piping) has been sized to allow for a minimum 20% increase in demands. This approach provides DLC with the ability to accommodate some growth in the UPZ without undertaking significant capital works. Contrary to pump sizing, allowing for future growth in watermain sizing reduces energy consumption by decreasing frictional losses that have to be overcome by the pumping system.

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4.0 Distribution Network Review

Urban completed a review of UPZ distribution network to identify potential water quality concerns, adequacy of fire protect provided and for general compliance with the DLC SDSB.

4.1 Water Quality

The UPZ is divided into two braches as shown on Figure 1. The north branch has 6 irrigation services. The south branch contains all 38 residential services and the remaining 12 irrigation services. The north branch is most likely to cause a water quality problem as it is stagnant outside of irrigation season. To address this concern, Urban recommends that DLC isolate and drain the north branch after irrigation season. In the spring and prior to DLC allowing irrigation, the north branch would be flushed and disinfected in accordance with AWWA C651.

The south branch terminates in two separate dead-end mains—the south-east dead-end and south-west dead-end. We estimate that the water ages a maximum of a half day under winter ADD conditions in the south branch. We typically do not expect any water quality issues to arise unless water age exceeds 5 days when free chlorine is utilized for a residual disinfectant. DLC staff advised that they do not have any problems maintaining chlorine residual in the UPZ as Urban expected. The only item pertaining to the south branch that Urban raises for DLCs consideration is the fact that the south-east dead-end terminates with a section approximately 337 m long with 4 irrigation services. As per the north branch, this portion of watermain will be stagnant outside of irrigation season which may cause water quality problems. Urban recommends that DLC consider taking the same action as recommend above for the north branch to mitigate potential issues with stagnant water in the south-east dead-end main.

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4.2 Fire Protection

Urban reviewed available fire flow and hydrant coverage within the UPZ. Based on a past report prepared by Mould Engineering Services Ltd., the maximum available fire flow under existing conditions equals approximately 25 L/s at hydrant 530 (refer to Figure 2). The available fire flow will be less than this value at the other two hydrants due to the headloss caused by the 100 mm watermain in the residential area. As discussed under Section 2, we recommend that the UPZ be able to deliver a minimum fire flow of 60 L/s. In order to achieve this, the majority of the watermain in the south branch of the UPZ will have to be replaced with a larger 200 mm pipe—refer to the highlighted watermain in Figure 1. It is not feasible to convey the fire flow through the existing piping due to excessive amount of headloss that will occur.

Figure 2 shows the existing hydrant coverage based on the minimum recommended spacing (i.e., 180 m) identified by the Fire Underwriters Survey. Urban recommends that DLC consider adding at least 2 private hydrants, as shown on Figures 1 and 2 to improve hydrant coverage for existing structures.

4.3 Conformance to Subdivision and Development Servicing Bylaw

Urban reviewed Schedule C.5 of DLC SDSB and identified a few items that do not conform to the specified design criteria. These items relate to pipe sizing, dead-end watermains, blow-offs and valving.

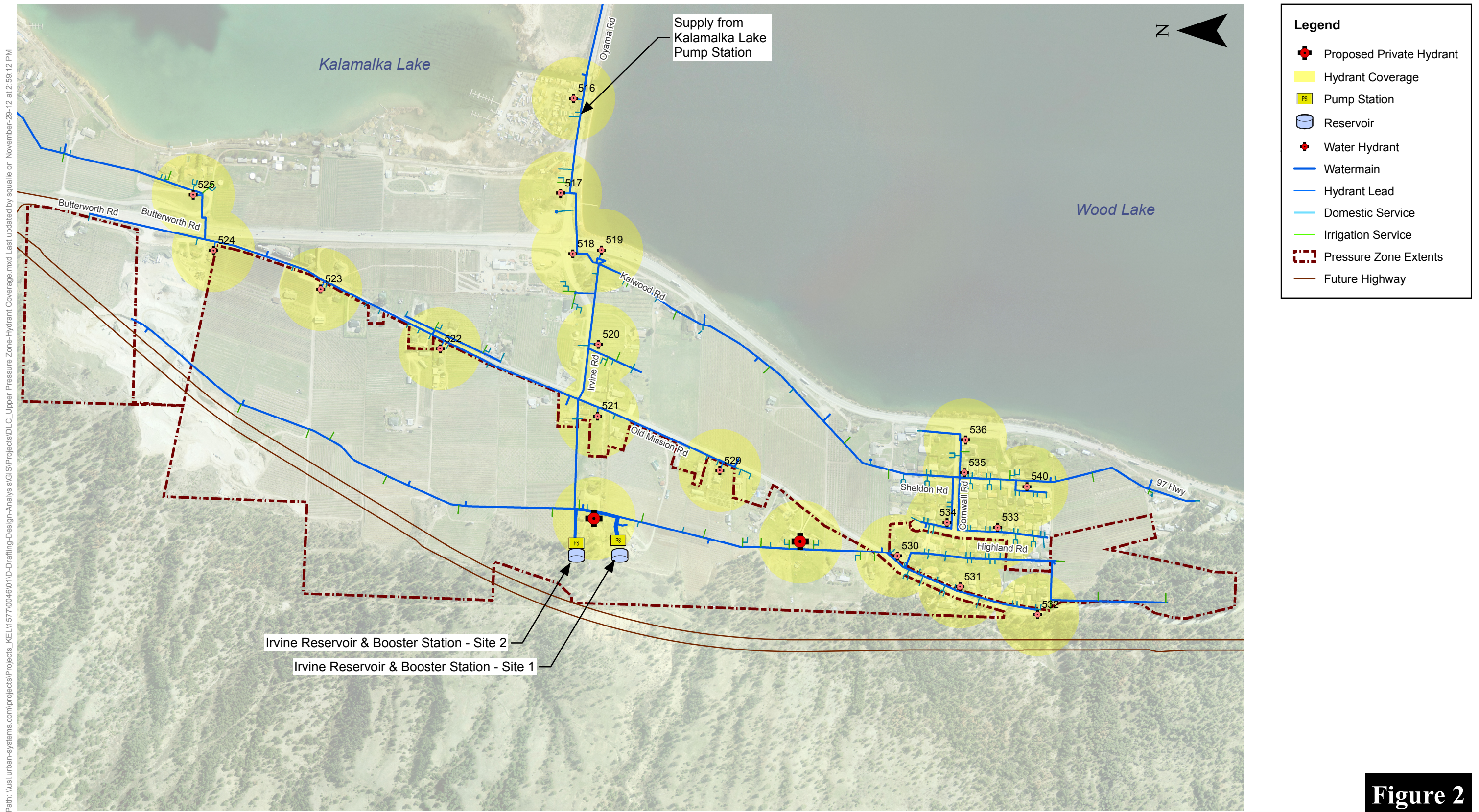
The minimum permitted pipe size is 200 mm. The majority of the UPZ does not meet this criterion. As per Section 4.2, the critically undersized pipe as it relates to fire protection is identified for replacement on Figure 1.

150 m is the maximum allowable length of a dead-end watermain. Section 4.1 identified two dead-ends of concern (north branch and south-east dead-end) and measures to mitigate potential water quality issues.

Blow-offs are required at the end of watermains. Urban recommends that a blow-off be added to the end of the north branch and the south-east dead end. The hydrant at the end of the south-west dead-end should eliminate the need for a blow off in that location.

If UPZ was constructed today, up to 10 additional valves would have been installed to comply with DLC SDSB. Urban has identified 4 additional valves that can be installed to add flexibility for operations and maintenance tasks.

Kalamalka Lake Distribution System Upper Pressure Zone - Hydrant Coverage



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Legend

- Proposed Private Hydrant
- Hydrant Coverage
- Pump Station
- Reservoir
- Water Hydrant
- Watermain
- Hydrant Lead
- Domestic Service
- Irrigation Service
- Pressure Zone Extents
- Future Highway

4.4 Summary of Distribution Network Recommendations

REPORT

Kalamalka Lake
Water Distribution
System – Upper
Pressure Zone
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Table 4-1 – Summary Distribution Network Recommendations

Review Focus	Recommendations
Water Quality	North branch and South-east dead end of UPZ – Isolate main outside of irrigation season when not in use and flush and disinfect before returning the watermain to service.
Fire Protection	Upsize watermain to all hydrants as shown in Figure 1.
	Install 2 private hydrants as shown on Figure 1. Additional coverage provided by these hydrants is shown on Figure 2.
Conformance to Subdivision and Development Servicing Bylaw	Install 2 blow-offs as shown on Figure 1.
	Add 4 valves as shown on Figure 1.

REPORT

Kalamalka Lake
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5.0 Booster Station – Proposed Upgrade

Urban has completed the preliminary design of a booster station that will replace both existing booster stations servicing the UPZ. The proposed booster will address concerns raised in Section 2.3. The preliminary design was aided by Interior Instrument Engineering Services and Advanced Surveying Ltd. Interior Instrument Engineering Services provided input on the electrical and instrumentation requirements. Advanced Surveying Ltd. completed a topographical survey of the proposed site for the replacement booster station. The preliminary design is summarized in this section.

5.1 System Analysis

Urban has identified pumping requirements with a Bentley WaterCad V8i water model. These results are preliminary and are to be verified at the detailed design stage. Duty pump sizing is based on being able to deliver PHD with a single pump. Fire flow conditions allow for both duty pumps to be operating in parallel. The jockey pump has been sized to provide the domestic PHD.

Table 5-1 – Pumping Requirements

Pump and Scenario	Flow (L/s)	Total Dynamic Head (m)	Estimated Power Requirement (HP)
Duty Pump			
PHD	46.0	41.5	36
MDD + Fire Flow	106.0	28.9	29 ¹
Jockey Pump			
Domestic PHD	3.1	41.0	2.4

Notes:

1. Power requirement is per duty pump and based on maintaining a minimum system pressure of 20 psi (140 kPa).
2. Duty and jockey pump sized to maintain an equivalent system pressures to existing Site 2 facility (i.e., 64 psi at an elevation of approx. 469 m).
3. All scenarios are based on the Irvine Reservoir level being at 472.85 m (70% full) and all pumps at the Kalamalka Pump Station being turned off.
4. Total dynamic head estimates are based on watermain being replaced to all hydrants with 200 mm pipe as shown on Figure 1.

Urban has selected a duty pump (40 HP - Aurora 410) and jockey pump (3 HP – 15SVGoulds) that satisfy the requirements identified in Table 5-1. Refer to Appendix 5-1 for additional information on the selected pumps. The suitability of these pump selections should be reviewed again at the detailed design stage.

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For transient conditions, it is expected that the impacts can be mitigated through air/vacuum valves located throughout the distribution network and pressure relief or surge anticipating valve located in the booster station. There are 8 existing air valves located in the distribution system. Urban has allowed for an additional 2 air/vacuum valves to be installed and for all existing valves to be replaced if needed (e.g., if valves are only air releases, undersized, in poor condition, etc.). Urban recommends that a transient analysis be completed at the detailed design stage to confirm if a pressure relief or surge anticipating valve is required and to identify the most critical air valves. Urban also recommends that DLC confirm the size, style (i.e., air release or air/vacuum) and condition of the existing air valves.

5.2 Site Design

Refer to Figure 3 for a preliminary site plan. The proposed booster station will be located next to the existing Site 1 booster station. Site 1 has been selected as the preferred location for the proposed booster station for two reasons: 1) Site has a lower elevation than Irvine reservoir which provides higher suction head for the pumps and 2) Site is relatively large and has sufficient space to accommodate the proposed building. Urban has proposed that the station be cut into the hillside near the east property limit. This has two benefits including: 1) Keeping the finished floor elevation as low as possible to ensure the pumps receive adequate suction pressure and 2) Limiting the amount of access road that has to be constructed. The proposed building location will require a retaining wall to maintain a floor elevation of 455.5 m and allow for ground level access. Urban recommends that the site layout be reviewed again when a geotechnical investigation has been completed. Urban notes that DLC is currently using a private driveway to access both Irvine Reservoir sites as shown in drawings of the existing facilities included Appendix 5-2. Urban recommends that DLC consider constructing an access road to the sites where they have secured a right-of-way. This is beyond the scope of this project and has not been allowed for in the estimated project costs provided in Section 6 of this report.

5.3 Booster Station Design

The booster station will be divided into two rooms, a mechanical room and an electrical room. This separation helps minimize the risk of electrical equipment being damaged from water in the event that any piping leaks or fails.

DLC has advised that minimal amenities are required at the station due to its proximity to other DLC facilities. As such, the preliminary layout does not include a washroom or have any designated space for an office or working station. The electrical room should have sufficient space to accommodate a working station if desired in the future.

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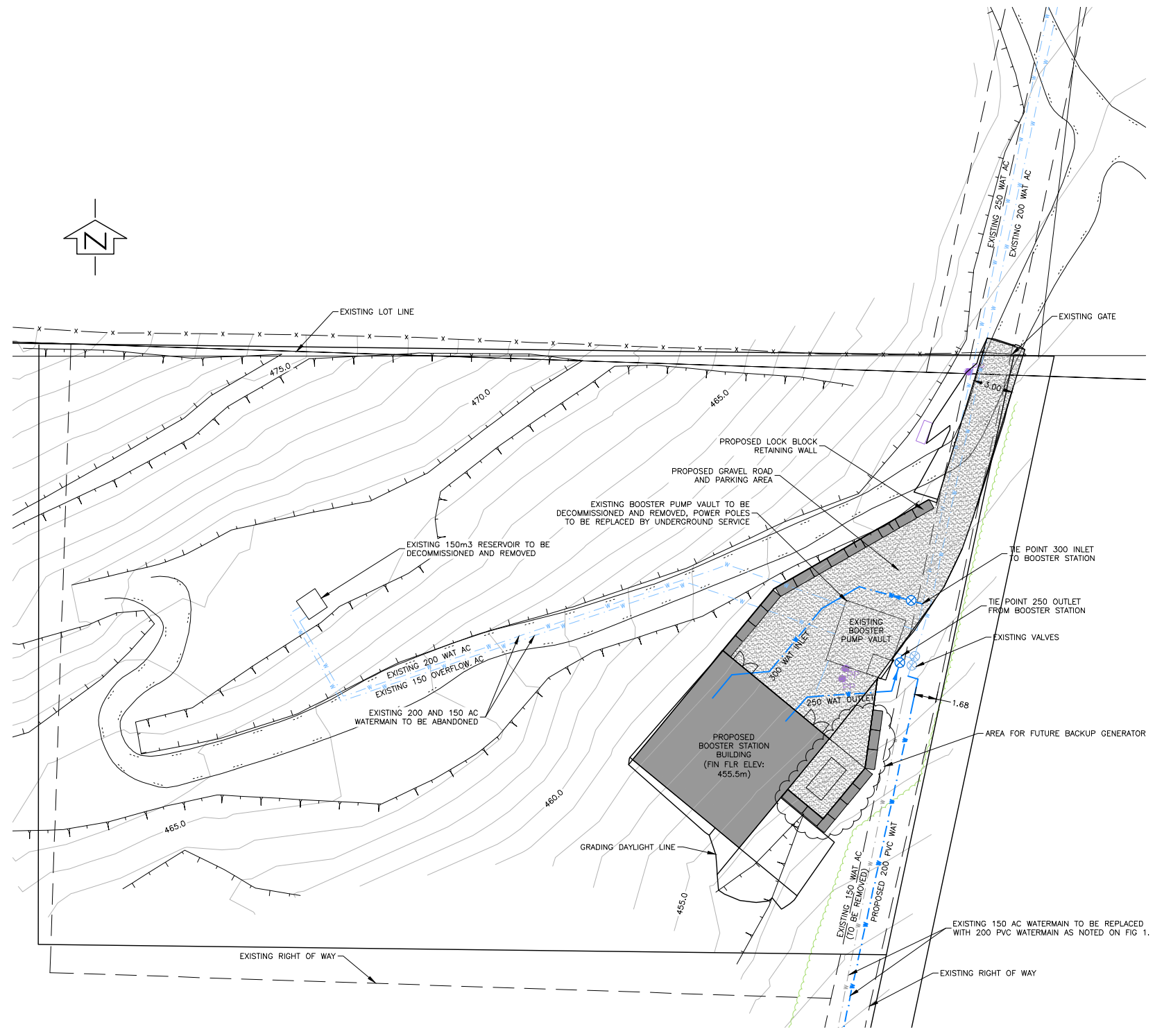
1577.0046.01
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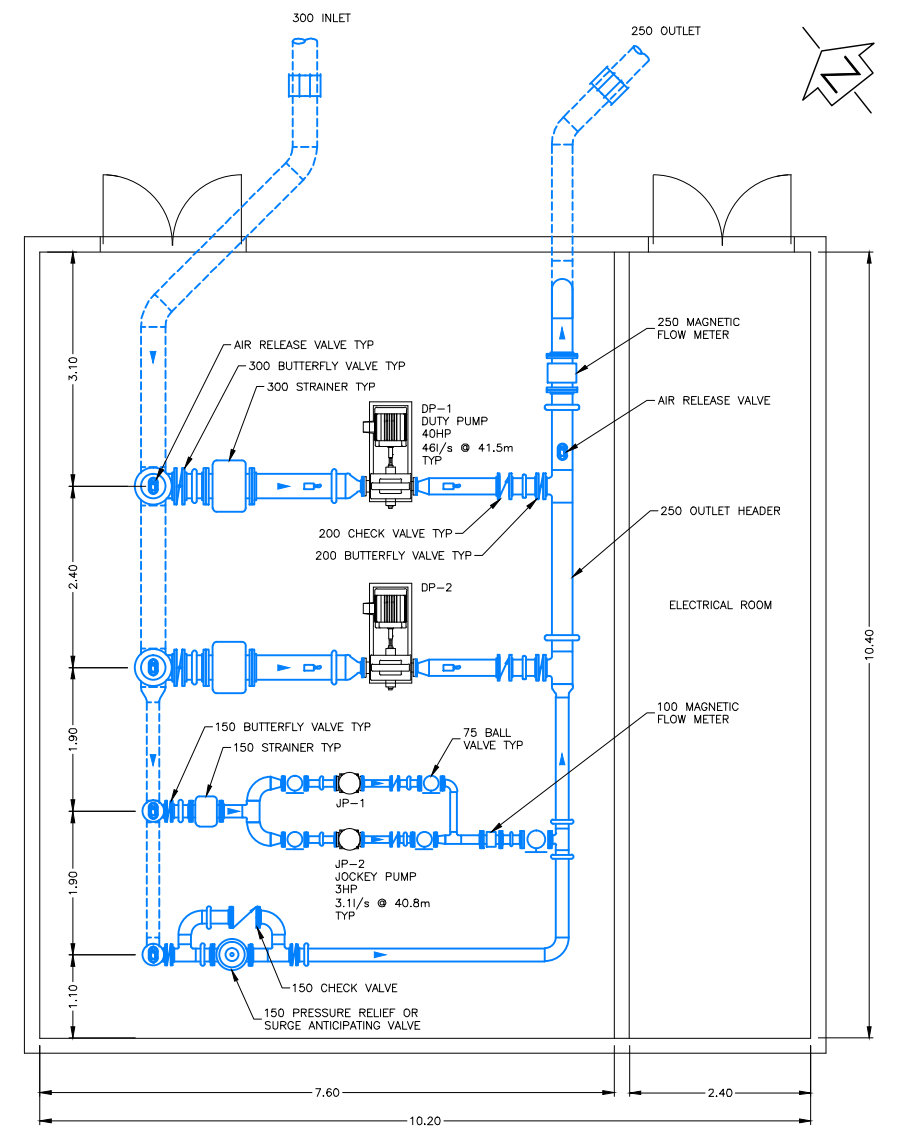
Full redundancy is provided for the duty and jockey pumps (i.e., one duty and one standby unit). Urban recommends using a horizontal split case style pump for the duty pumps based on the low head requirement. A vertical inline booster style pump is recommended for the jockey pumps. Strainers have been included to protect all pumps from solids. The process piping allows for a pressure relief or surge anticipating valve to be installed. The check valve shown on the relief line has been included so gravity flow can be maintained to residential users in the event of a power failure. Based on discussion with DLC staff, Urban has not provided space for a chlorine boosting system or allowed for any online analyzers.

Proposed booster will require a 600V, three phase 200A service from BC Hydro. Station will be equipped with a MCC that incorporates BC Hydro metering, surge arrestor, lighting panel and VFD motor control. Proposed PLC panel is to be installed to include SCADAPack PLC, radio equipment to interface with the District SCADA System, and UPS. PLC will be programmed to interface into the upgraded Kalamalka Lake Water Distribution System. Instrumentation will include 2-flowmeters, 2-pressure transmitters, and a room temperature transmitter. Provisions will be made to facilitate the installation of a future standby generator with minimal interference to the proposed building.

Costs identified in Section 6 allow for an HVAC system that includes energy efficient variable speed exhaust fan and intake damper.

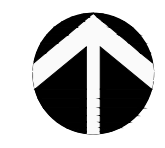


SITE PLAN
1:200



BUILDING FLOOR PLAN
1:50

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Client/Project		
DISTRICT OF LAKE COUNTRY IRVINE RESERVOIR		
Scale	Date	Figure
AS NOTED	2012/11/09	FIG 3
1577.0046.01		Title

**BOOSTER STATION
SITE AND BUILDING LAYOUT**

REPORT

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5.4 Constructability

The pumps are sized based on the watermain, as highlighted in Figure 1, being replaced. As such, the proposed watermain will have to be installed and put into service before the proposed booster station is commissioned. Construction of both the watermain and booster station can occur at the same time but appropriate completion dates will have to be coordinated to address this matter.

Maintaining service to the UPZ while the proposed booster station is constructed will be relatively straightforward. Refer to Figure 1. The Site 2 existing booster station adjacent to the Irvine Reservoir will be able to operate for the duration of the project. The Site 1 existing booster, which acts as back up to the Site 2 booster, can be left in service for the majority of the project.

A portion of the watermain that is to be replaced (approximately 250 of 943 m shown on Figure 1) will disturb agricultural land and require the removal of fruit trees. This section of pipe could be installed with a trenchless method if needed to minimize surface disturbance.

5.5 Regulatory

Urban recommends providing Interior Health (IH) with the finalized copy of this report so they are aware of this upcoming project. If IH is able to provide feedback at this stage it should help expedite the construction permit application.

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6.0 Cost Estimate

Table 6-1 contains a preliminary cost estimate for the UPZ proposed upgrades. A detailed cost estimate breakdown has been included in Appendix 6-1.

Table 6 -1 – UPZ Proposed Upgrades Cost Estimate

Item	Description	Total
1	Removals	\$70,000
2	Booster Station	\$640,000
3	Watermain Replacement	\$355,000
	Subtotal	\$1,065,000
	Contingency and Engineering (35%)	\$372,750
	Total (Rounded)	\$1,440,000

Notes

- Item No.1 includes decommissioning and removal of the 150 m³ reservoir and booster station from Site 1 and removal of mechanical and electrical equipment from the Site 2 booster station. Refer to Figure 1.
- Watermain replacement includes allowance for replacing existing air/vacuum valves and installation of proposed air/vacuum valves as noted in Section 5.1.
- Watermain replacement does not include any costs related to compensation for fruit trees that will be disturbed or removed when the watermain is installed.
- Costs for optional proposed distribution network upgrades are included in Appendix 6-1. The optional items include the 2 private hydrants, 2 blow-offs and 4 isolation valves as shown on Figure 1.

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Appendix 2-1

Water Model Background

MEMORANDUM

Date: January 14, 2013
To: District of Lake Country
From: Jonathan Lowe, P.Eng., Jeremy Clowes, P.Eng.
File: 1577.0046.01
Subject: Model Notes – Kalamalka Lake Distribution System - Upper Pressure Zone

1.0 INTRODUCTION

The purpose of this memorandum is to summarize:

- The methodology used for allocating the water demands across the upper pressure zone within the existing computer model of the Kalamalka Lake Water System and
- The analysis that was completed to determine pumping requirements for the proposed Irvine Booster Station.

2.0 WATER DEMANDS

The serviced lots/parcels within the Kalamalka Water System are not metered; therefore water demands were calculated based upon theoretical values provided by the DLC. The water demands were divided as follows:

- Indoor domestic – water used indoors for washing, drinking etc.
- Outdoor domestic – residential irrigation for lawns, landscaping etc.
- Agricultural irrigation – irrigation for agricultural crops within the ALR.

Domestic demands were based upon the information presented in the District of Lake Country (DLC) Engineering Water Conservation Initiative Study 2011 (referred to here after as “DLC Study”).

All parcels with a domestic service connection were considered active (as per direction received from the DLC). The indoor domestic demand and occupancy per domestic service was assumed to be 330 L/p/d and 2.6 people/household respectively as per the DLC Study. Therefore, the indoor (i.e. winter demand) per domestic service was 858 L/day/parcel or 0.010 L/s/parcel.

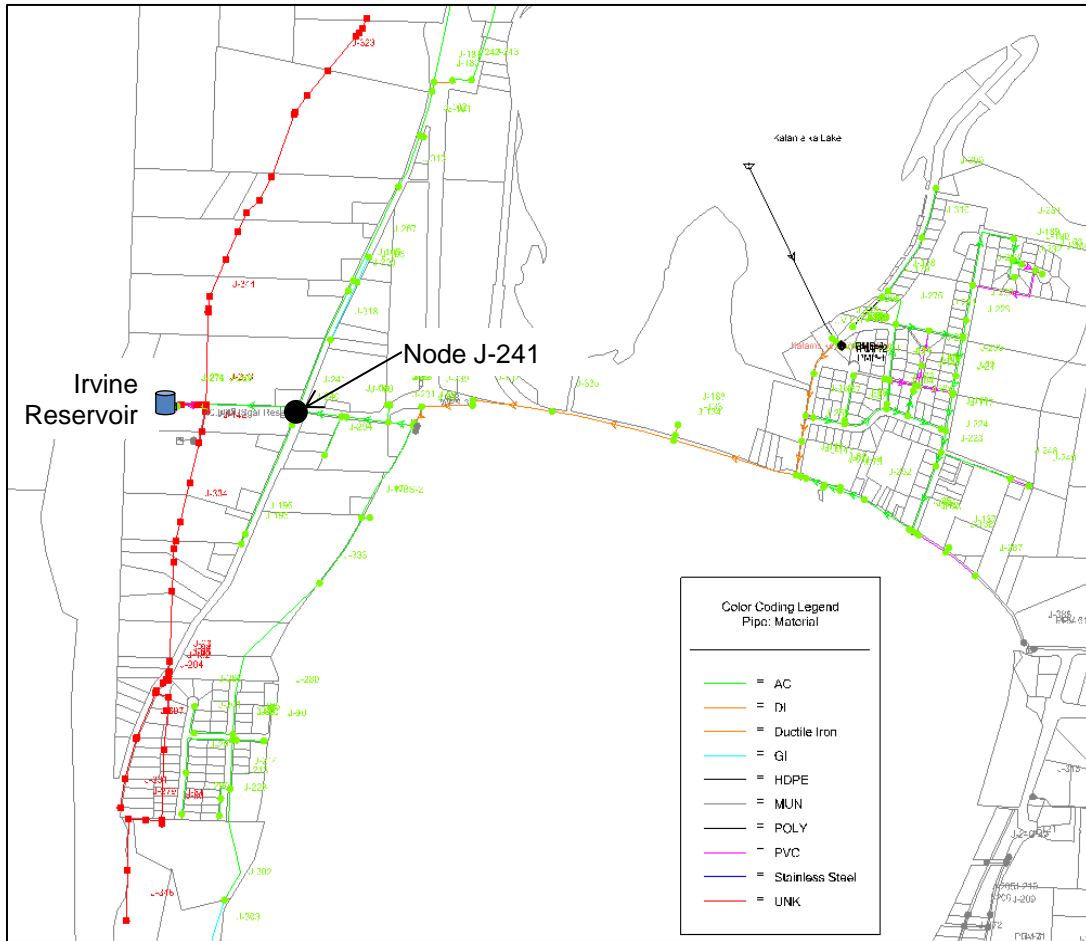
All domestic service connections were assumed to be unmetered as directed by the DLC. An outdoor irrigation demand per capita of 1,640 L/p/d for non-metered connections was used as per the DLC Study. As with the indoor domestic demand, an occupancy of 2.6 people/household was used. Therefore, the outdoor domestic irrigation demand per domestic service was 4,264 L/day/parcel or 0.049 L/s/parcel.

Agricultural irrigation demands were based upon the maximum allowable irrigation allotment per agricultural service connection of 970mm/year stated in the DLC bylaw. The irrigable area per parcel was determined based upon mapping of the current active irrigable areas provided by the DLC. The duration of the irrigation season was assumed to span 120 days per year. The unit agricultural irrigation demand used in the model was therefore 80.8 m³/day/ha or 0.94 L/s/ha.

MEMORANDUM

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 Subject: Model Notes – Kalamalka Lake Distribution System - Upper Pressure Zone
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Below is a screen shot showing the upper pressure zone pipe distribution network highlighted in red.



During the initial set up of the model back in November 2011, all the demands for the Kalamalka Water System were allocated to a single junction in the model (J-241) because at the time there was no need to spatially distribute the demands.

The unit demands allocated to J-241 are summarized in the table below. There are a total of 296 domestic parcels with an active service connection and 97.0 ha of active irrigable area.

	Number of Unit Demands	Unit Demand	Unit Demand Unit	Demand (Base) (L/s)	Pattern (Demand)
1	296.000	Dom_Indoor		2.939	Fixed
2	296.000	Dom_Outdoor		14.608	Fixed
3	97.000	Ag_Ir_ha		90.713	Fixed
*					

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 Subject: Model Notes – Kalamalka Lake Distribution System - Upper Pressure Zone
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The water demands in the upper pressure zone are summarized below:

Demand Group	Unit	Unit Demand	Demand (L/s)
Indoor domestic	38 parcels	0.010 L/day/parcel	0.38
Outdoor domestic	38 parcels	0.049 L/day/parcel	1.88
Agricultural irrigation	46.36 ha	0.94 L/s/ha	43.6

2.1 Water Demand Allocation Methodology

To simplify the demand allocation process within the WaterCAD model, water demands along the upper pressure zone were grouped together into "Demand Allocation Areas" and assigned to central locations along the distribution system.

Three demand scenarios were created in the model:

1. Maximum Daily Demand (MDD) - agricultural and indoor and outdoor domestic demands
2. Domestic indoor and outdoor demands
3. Winter Daily Demands (WDD) - indoor domestic demand only

3.0 PUMPING REQUIREMENTS

The analysis completed to determine pumping requirements is summarized below:

Demand Scenario	Irvine Reservoir Level (m)	Suction Head at J-390 (psi)	Discharge Head at J-394 (psi)	TDH (m)	Flow (L/s)
MDD	472.85	23.6	82.5	41.5	45.6
MDD + Fire Flow ¹	472.85	19.9	60.9	28.9	105.6
PHD (domestic only)	472.85	24.6	82.8	41.0	3.1

Notes:

1. Fire flow allocated to J-397 (Hydrant 532). Minimum system pressure of 21 psi at J-397.

4.0 CLOSURE

If you have any questions regarding the above, please don't hesitate to contact the undersigned.

Regards,

URBAN SYSTEMS LTD.

Jonathan Lowe, P.Eng
 Water & Wastewater Engineer

/jdl

Jeremy Clowes, P.Eng.
 Water Resources Engineer

\\usl.urban-systems.com\projects\Projects_KEL\1577\0046\01D-Drafting-Design-Analysis\Water Model\2012-11-28 MEMO Kalamalka Upper Pressure Zone Demand Allocation.docx

REPORT

Kalamalka Lake
Water Distribution
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Appendix 5-1

Pump Selections

Company: Natpro
 Name: Irvine Booster Station
 Date: 11/20/2012

Potable Water Pump

AURORA®

Pump:

Size: 4x5x15
 Type: 410 1 STG SPLIT CASE
 Synch speed: 1800 rpm
 Curve: 2PC-117373A
 Specific Speeds:
 Dimensions:
 Speed: 1775 rpm
 Dia: 316 mm
 Impeller: 444V326
 nq: 14
 S: 102
 Suction: 125 mm
 Discharge: 100 mm

Search Criteria:

Flow: 46 l/s Head: 41.5 m

Fluid:

Water
 SG: 1
 Viscosity: 1.104 cP
 NPSHa: ---
 Temperature: 15.6 °C
 Vapor pressure: 1.773 kPa a
 Atm pressure: 101.4 kPa a

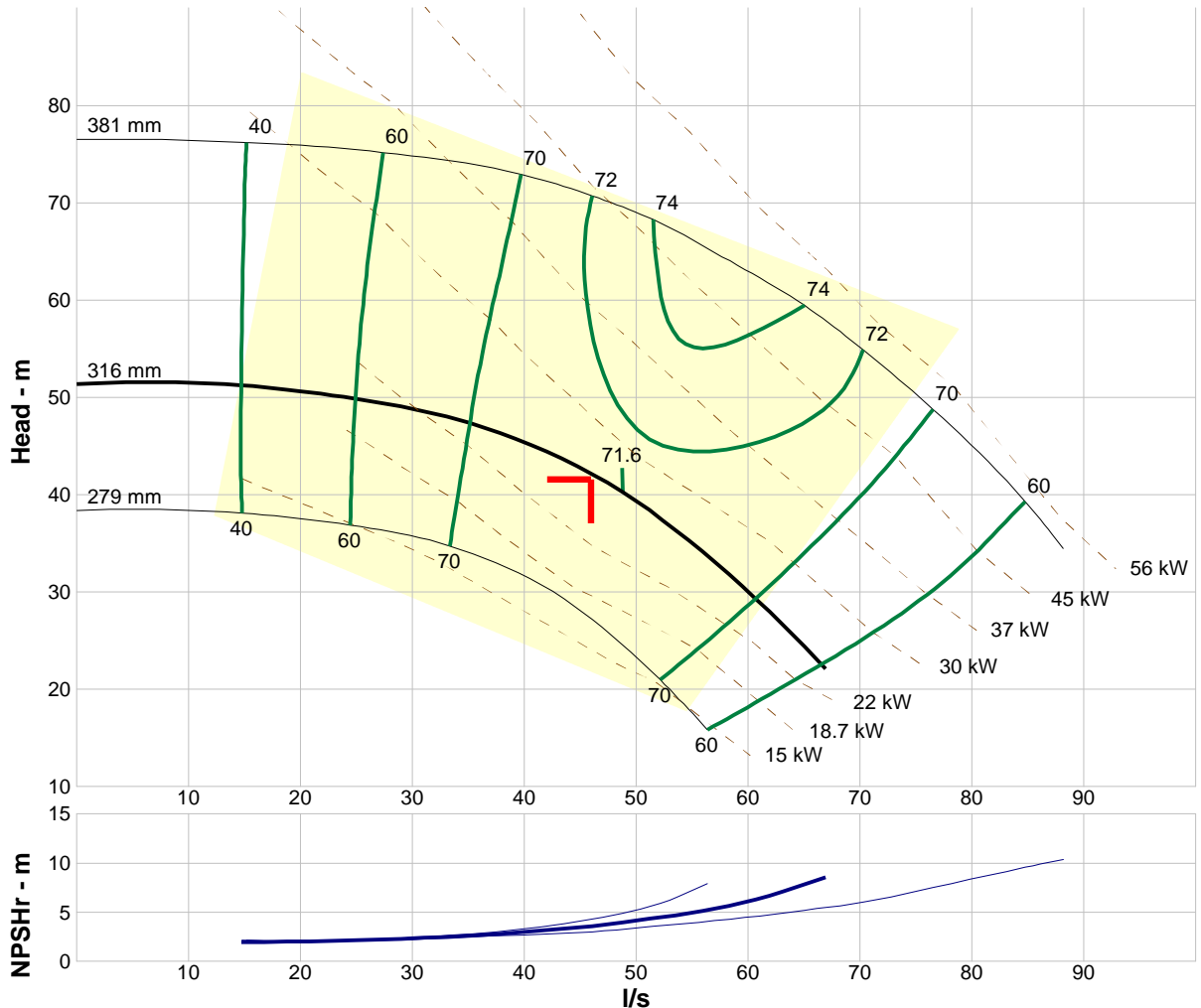
Motor:

Standard: NEMA
 Enclosure: ODP
 Sizing criteria: Max Power on Design Curve
 Size: 30 kW
 Speed: 1800
 Frame: 324T

Pump Limits:

Temperature: 135 °C
 Pressure: 1724 kPa g
 Sphere size: 15.9 mm
 Power: ---
 Eye area: ---

---- Data Point ----	
Flow:	46 l/s
Head:	41.7 m
Eff:	71%
Power:	26.1 kW
NPSHr:	3.71 m
---- Design Curve ----	
Shutoff head:	51.4 m
Shutoff dP:	502 kPa
Min flow:	---
BEP:	72% @ 48.8 l/s
NOL power:	26.8 kW @ 48.8 l/s
-- Max Curve --	
Max power:	54.4 kW @ 84.7 l/s



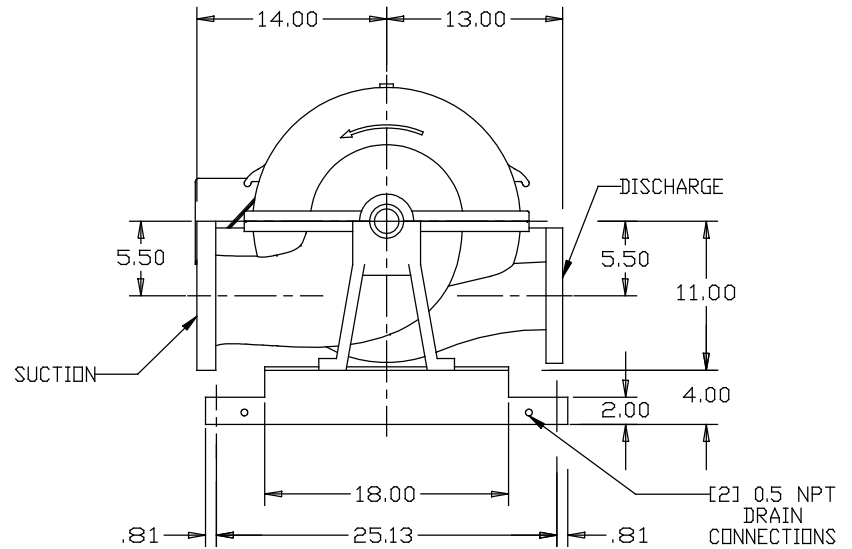
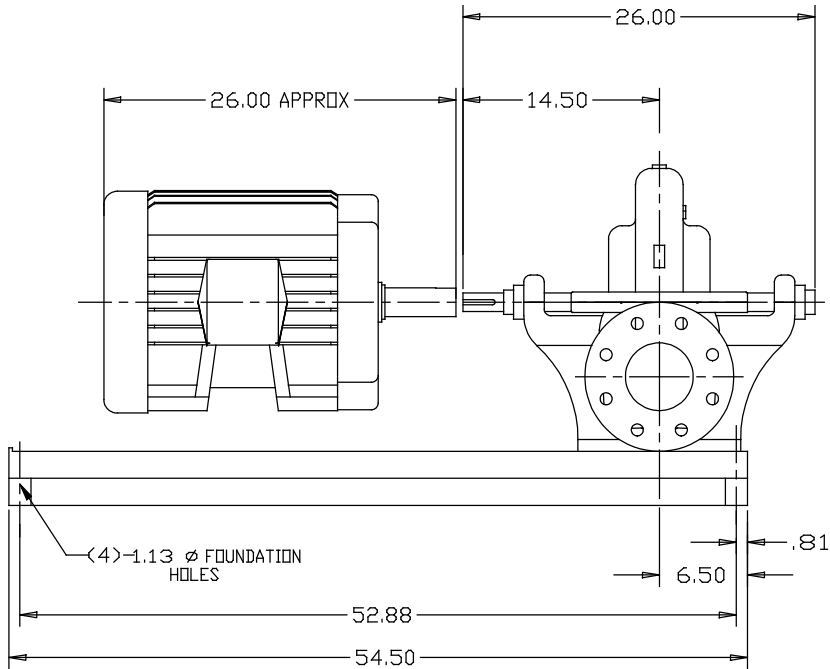
Curve efficiencies are typical. For guaranteed values, contact Aurora Pump or your local distributor. Las eficiencias en curvas son típicas. Para valores garantizados contacte a Aurora Pump o a su distribuidor local.

Performance Evaluation:

Flow l/s	Speed rpm	Head m	Efficiency %	Power kW	NPSHr m
55.2	1775	34.3	71	25.8	5.24
46	1775	41.7	71	26.1	3.71
36.8	1775	46.5	70	23.7	2.8
27.6	1775	49.2	63	21	2.29
18.4	1775	50.7	47	19.1	2.04

PART NUMBER
4114X5X15_RH25-125DR324T

ITEM	QTY	PART NUMBER	DESCRIPTION
1	1	324T_MOTOR	
2	1	411_4X5X15_RH_125-125	
3	1	DR_BASE_18X54X4	



NOTE :
COUPLING GAP MAY VARY FROM .13 TO 2.06

REV	ZONE	DATE	REVISION DESCRIPTION	BY	CK'D

MATERIAL:		OP AURORA® PENTAIR PUMP GROUP	
COMPANY CONFIDENTIAL Information contained herein is confidential. It is the property of Aurora Pumps & is to be used solely for the purpose provided and is not to be disclosed to others without prior written consent of Aurora Pump.			
DO NOT SCALE DRAWING		411 ON DRIP RIM BASE	
PLOT SCALE 0.150		DRAWN BY MAHADEVAN	CHECKED BY JLF
DATE 10/07/03		APPROVED BY CSJ	
PATTERN NUMBER -	SIZE C	PART NUMBER 4114X5X15_RH25-125DR324T	REV -

AGGRESSIVE PUMP & SUPPLY

November 20, 2012

Proposal No: AS12-11-20 01

Item No: ITEM 001

Job:

Series: e-SV Size: 15SV QTY: 1 Stage No: 11

Operating conditions

SERVICE

LIQUID

CAPACITY 50.0 gpm

HEAD 134.0 (ft)

Performance at 1750 RPM

PUBLISHED EFFY 71.7% (CDS)

RATED EFFY 71.7%

RATED POWER 2.40 hp (Run out 2.5 hp)

NPSHR 2.6 ft

DISCHARGE PRESSURE 59.9 psi g (79.7 psi g @ Shut off) Based on 0.0 psi g Suc.press

PERF. CURVE 15SV-1800-0

SHUT OFF HEAD 184.2 ft

Quoted Features

CONSTRUCTION Round-304 (SVB)

MECHANICAL SEAL Carbon/Silicon Carbide/Viton

Driver : Electric motor Manufacturer : Factory Choice

FURNISHED BY Pump Mfg MOUNTED BY Pump Mfg

RATING 3.0 hp (2.2 KW) ENCLOSURE TEFC-PE

PHASE/FREQ/VOLTS 3/60 Hz/575 SPEED 1800 RPM

INSULATION/SF FRAME 180TC

MOTOR PART NO V09A34H5BD2S

Program Version 1.39.0.0

Submittal Data

Multi-Stage Pumps

MODEL : 15SV11FF8E60

Hydraulic Data					Motor Data	e-SV Vertical Model	Qty.
Maximum Flow	Flow at Duty Point	Maximum TDH	TDH at Duty Point	NPSHR	Voltage/Phase/Enclosure		
63.7 gpm	50.0 gpm	184.2 ft	134.0 ft	2.6 ft	575V 3 PH TEFC-PE	15SV11FF8E60	1

Submittal Prepared for: AGGRESSIVE PUMP & SUPPLY

Job:

Proposal No: AS12-11-20 01

Engineer:

Contractor:

Item No: ITEM 001

Submittal Prepared by: Aggressive Pump Sales

Company:

Date: November 20, 2012

Submittal Date: November 20, 2012

Approved by:

Engineering Data

Pump Code: 15SV11FF8E60

Pump Size: 15SV

Stage No: 11

Pump Horsepower at Rating Point: 2.40 hp

Pump Shut Off Head: 184.2 ft

Efficiency: 71.7 %

Motor Speed: 1800

System Input Power: 3 / 575 V

Motor Rated Horsepower: 3.0 hp

Max.Frequency: 60Hz

Frame Size: 180TC

Motor Part No: V09A34H5BD2S

Discharge Size: 2.00 in

Impeller Construction: Round-304 (SVB)

Impeller Type: Radial Impeller

Shaft Seal: Carbon/Silicon Carbide/Viton

Standard Equipment / Capability:

PUMP

The e-SV pump is a non-self priming vertical multistage pump coupled to a standard motor. The liquid end, located between the upper cover and the pump casing, is held in place by tie rods. The pump casing is available with different configurations and connection types.

- Delivery: up to 700.0 gpm/ Head: up to 1200. ft
- Temperature of pumped liquid: -20.0 deg F to 250.0 deg F standard version
- Direction of rotation: clockwise looking at the pump from the top down (marked with an arrow on the adapter and on the coupling).

MOTOR

- Standard NEMA 180TC Frame motor totally enclosed fan cooled
- 1800 RPM nominal
- Three phase version, 4 pole: 575 V, 60Hz, 3.0 hp
- F-AISI Round Flange/304 with 580.0 psi g MAWP
- Reduced axial thrusts enable the use of standard NEMA TC motors that are easily found in the market
- Seal housing chamber designed to prevent the accumulation of air in the critical area next to the mechanical seal
- Mechanical seal according to EN 12756 (ex DIN 24960) and ISO 3069
- Versions with ANSI flanges that can be coupled to ANSI counter-flanges
- Threaded oval counter-flanges made of stainless steel are standard supply for the T versions
- Easy maintenance. No special tools required for assembly or disassembly
- Standard version for temperatures ranging from: 0 deg F to 250 deg F (optional to 300 deg F)

Unit Dimensions

Multi-Stage Pumps

MODEL : 15SV11FF8E60

Hydraulic Data					Motor Data	e-SV Vertical Model	Qty.
Maximum Flow	Flow at Duty Point	Maximum TDH	TDH at Duty Point	NPSHR	Voltage/Phase/Enclosure		
63.7 gpm	50.0 gpm	184.2 ft	134.0 ft	2.6 ft	575V 3 PH TEFC-PE	15SV11FF8E60	1

Submittal Prepared for: AGGRESSIVE PUMP & SUPPLY

Job:

Proposal No: AS12-11-20 01

Engineer:

Contractor:

Item No: ITEM 001

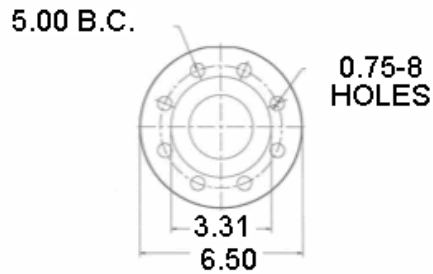
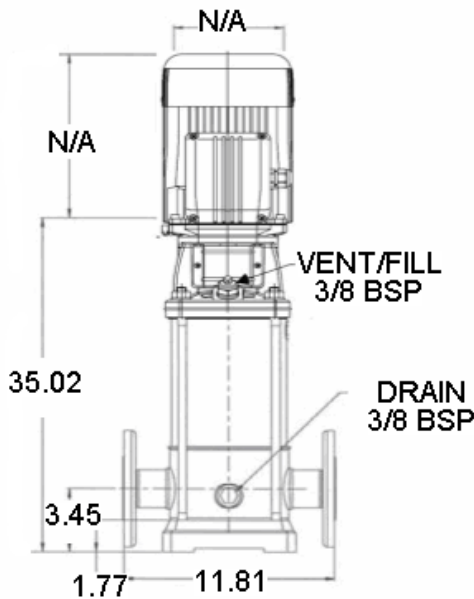
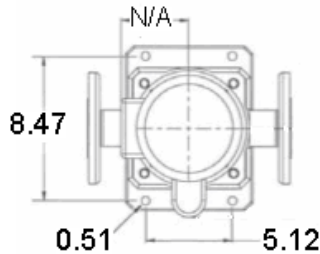
Submittal Prepared by: Aggressive Pump Sales

Company:

Date: November 20, 2012

Submittal Date: November 20, 2012

Approved by:



• Dimensions are subject to change. Not to be used for construction purposes unless certified

Performance Data

Multi-Stage Pumps

MODEL: 15SV11FF8E60

Hydraulic Data					Motor Data	e-SV Vertical Model	Qty.
Maximum Flow	Flow at Duty Point	Maximum TDH	TDH at Duty Point	NPSHR	Voltage/Phase/Enclosure		
63.7 gpm	50.0 gpm	184.2 ft	134.0 ft	2.6 ft	575V 3 PH TEFC-PE	15SV11FF8E60	1

Submittal Prepared for: AGGRESSIVE PUMP & SUPPLY

Job:

Proposal No: AS12-11-20 01

Engineer:

Contractor:

Item No: ITEM 001

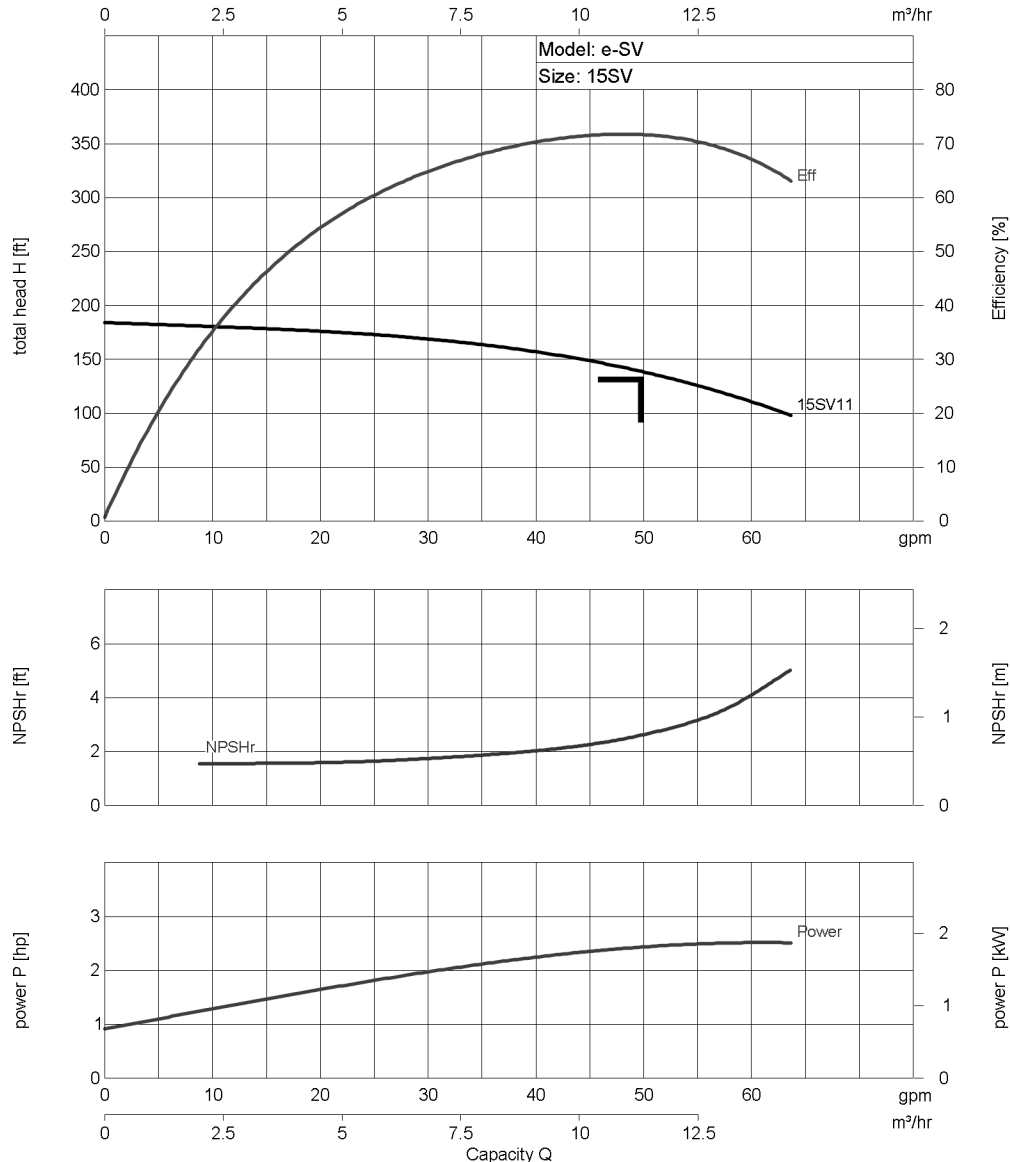
Submittal Prepared by: Aggressive Pump Sales

Company:

Date: November 20, 2012

Submittal Date: November 20, 2012

Approved by:



Performance Data

Multi-Stage Pumps

MODEL: 15SV11FF8E60

Hydraulic Data					Motor Data	e-SV Vertical Model	Qty.
Maximum Flow	Flow at Duty Point	Maximum TDH	TDH at Duty Point	NPSHR	Voltage/Phase/Enclosure		
63.7 gpm	50.0 gpm	184.2 ft	134.0 ft	2.6 ft	575V 3 PH TEFC-PE	15SV11FF8E60	1

Submittal Prepared for: AGGRESSIVE PUMP & SUPPLY

Job:

Proposal No: AS12-11-20 01

Engineer:

Contractor:

Item No: ITEM 001

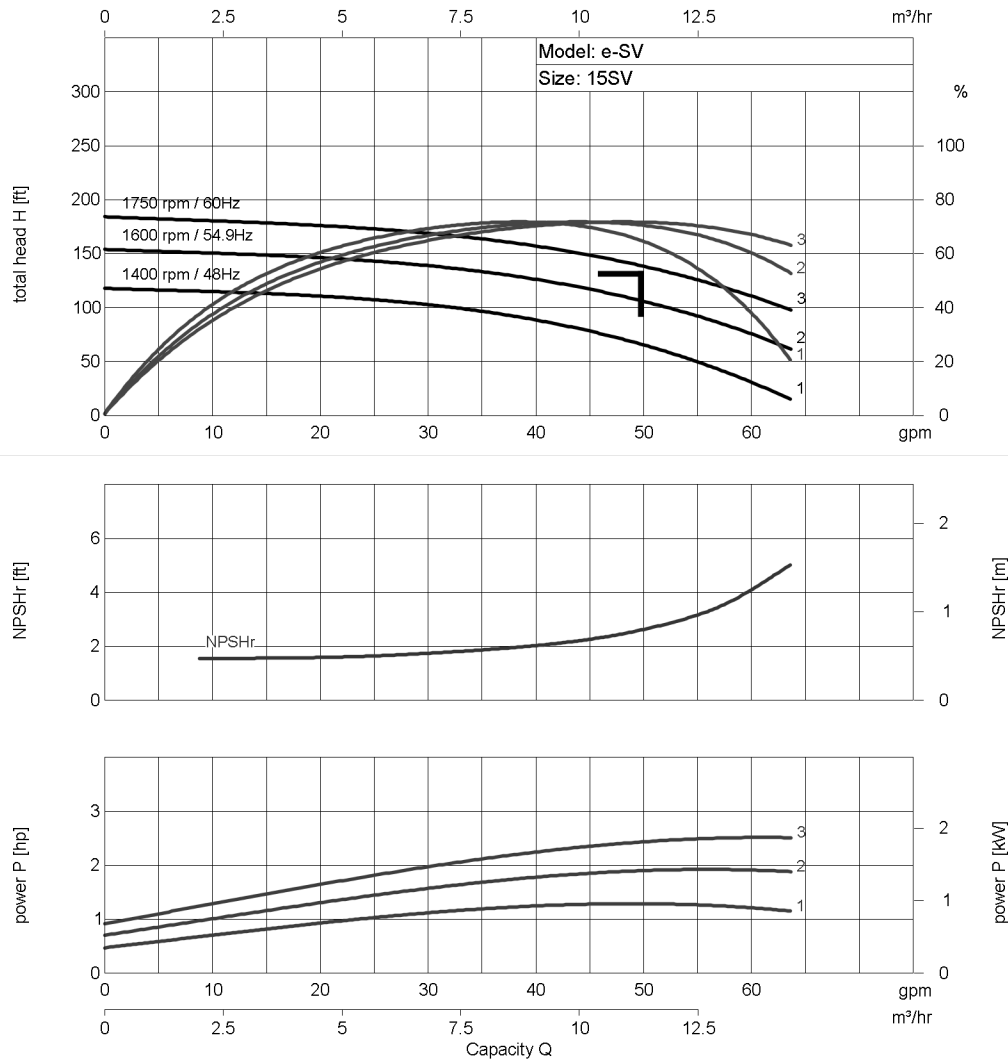
Submittal Prepared by: Aggressive Pump Sales

Company:

Date: November 20, 2012

Submittal Date: November 20, 2012

Approved by:



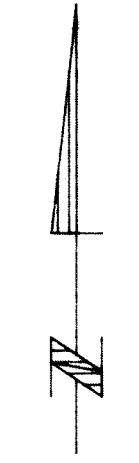
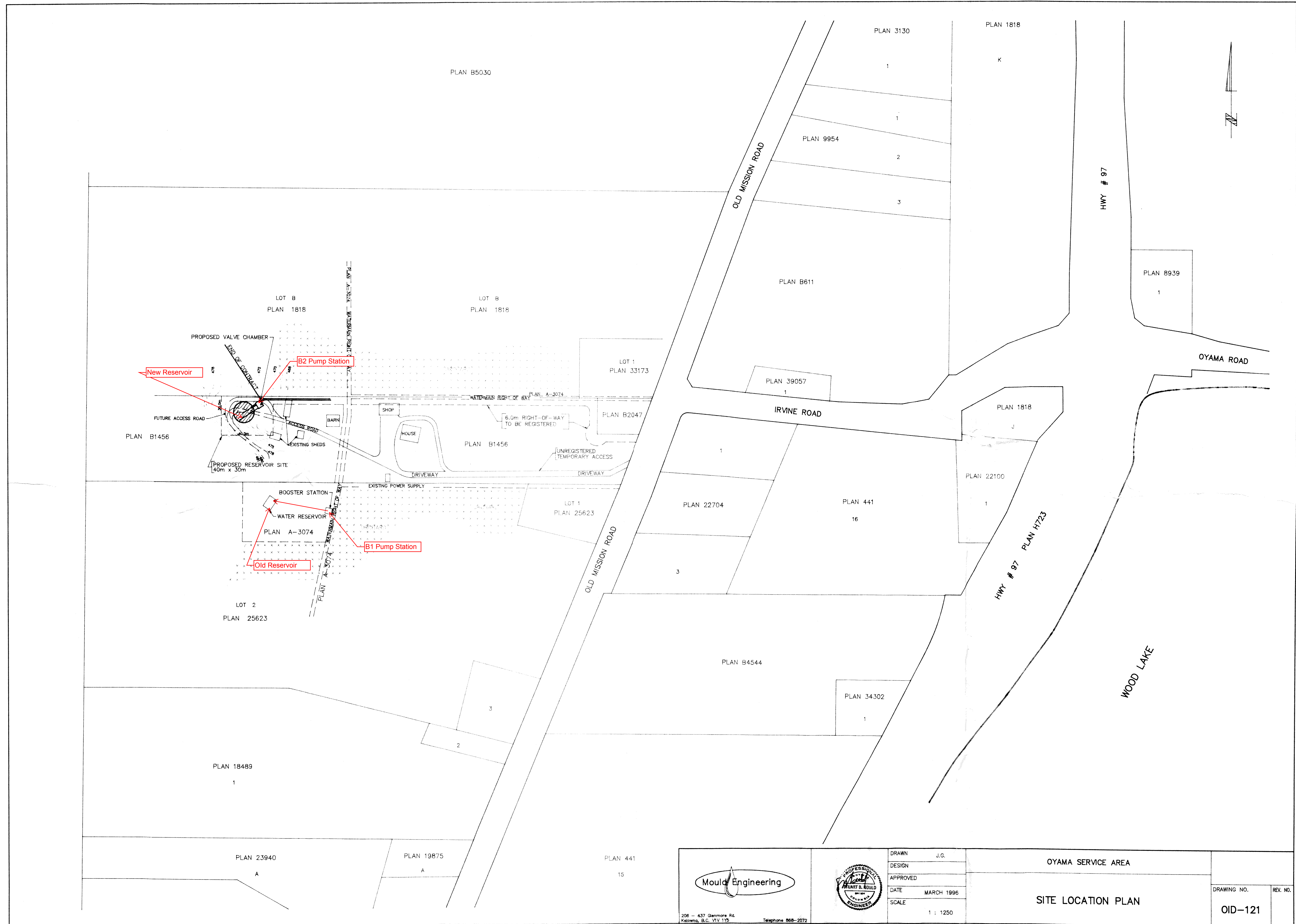
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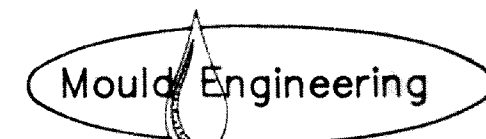
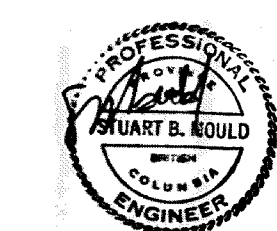
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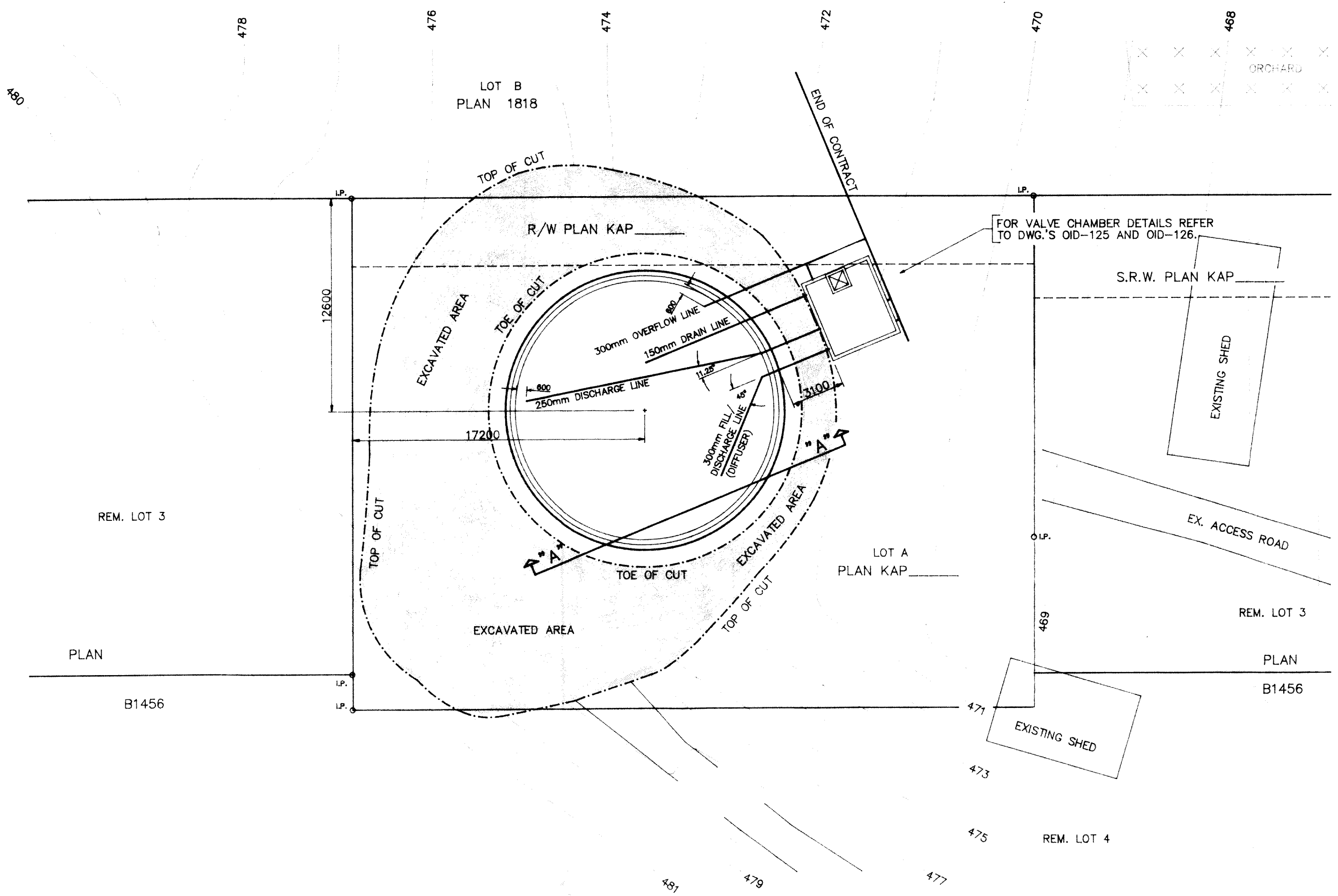
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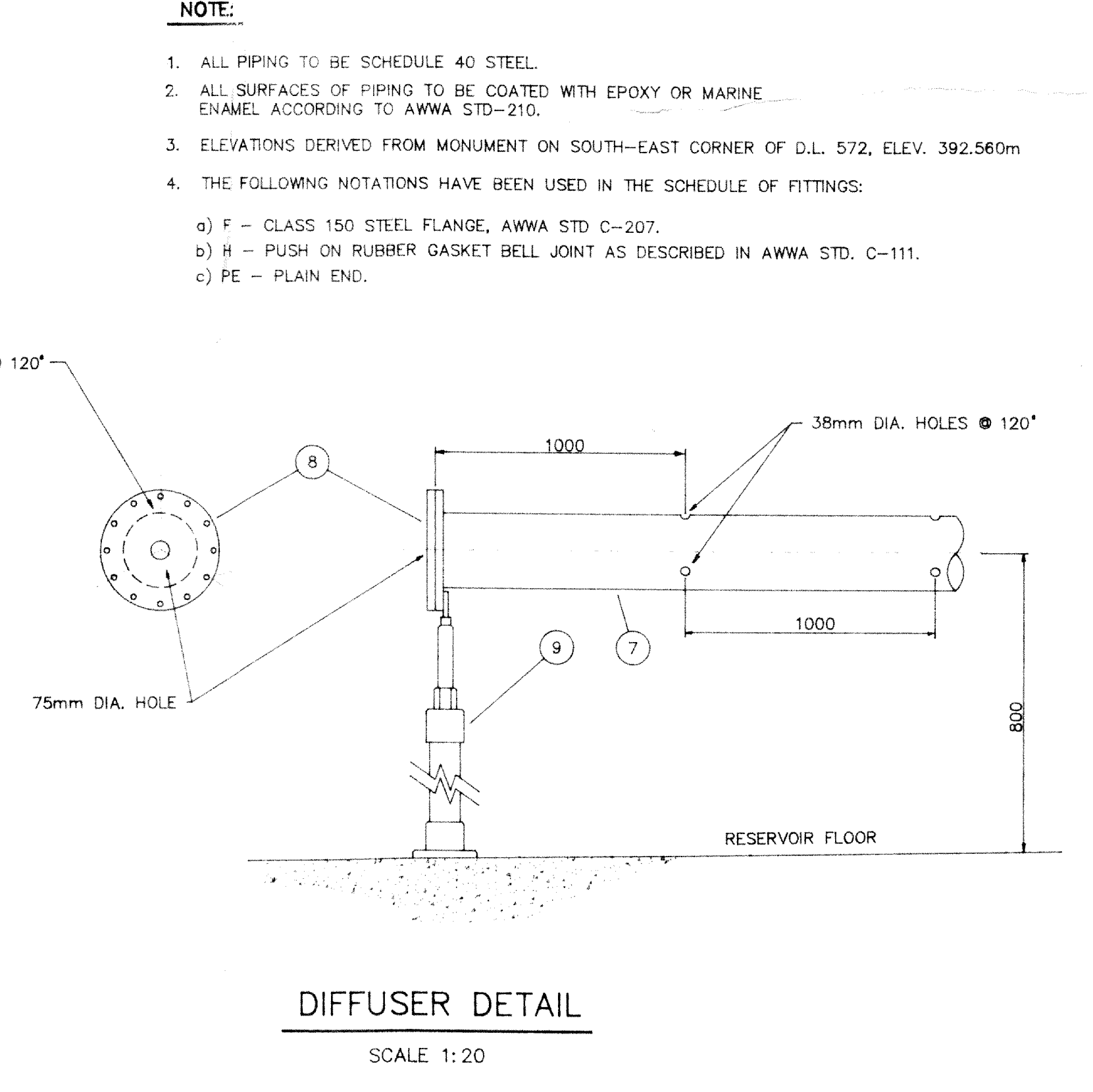
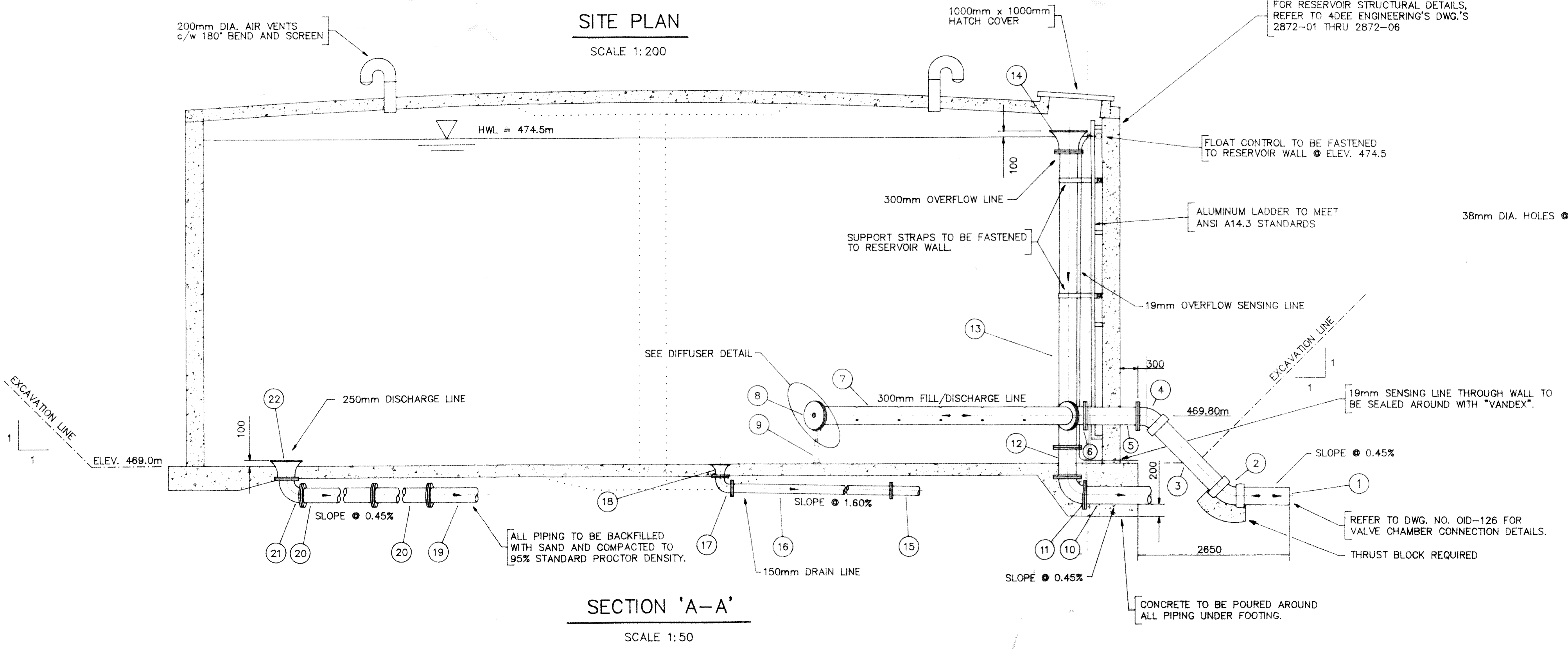
Appendix 5-2 Record Drawings



		DRAWN J.G. DESIGN APPROVED DATE MARCH 1996 SCALE 1 : 1250	OYAMA SERVICE AREA SITE LOCATION PLAN	DRAWING NO. OID-121 REV. NO.	
		206 - 437 Genmore Rd. Kelowna, B.C. V1Y 1Y5 Telephone 868-2072			

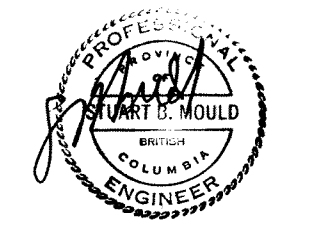
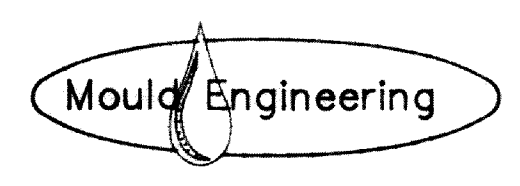


SCHEDULE OF FITTINGS		
ITEM NO.	NO. REQ'D	DESCRIPTION
1	1	300mm DIA. PVC SPOOL PIECE, 1000mm LONG, CUT TO SUIT IN FIELD.
2	1	300H x H 45° BEND
3	1	300mm DIA. PVC SPOOL PIECE, 1600mm LONG, CUT TO SUIT IN FIELD.
4	1	300H x F 45° BEND
5	1	300F x F SPOOL PIECE, 900mm LONG.
6	1	300F x F 45° BEND
7	1	300F x F SPOOL PIECE, 6000mm LONG, c/w 38mm DIA. HOLES DRILLED 120° INTERVALS ON 1000mm CENTERS.
8	1	300mm DIA. BLIND FLANGE, c/w 75mm DIA. HOLE DRILLED THRU CENTER OF FLANGE.
9	1	ADJUSTABLE FLANGE PIPE SUPPORT
10	1	300F x PE SPOOL PIECE, 2300mm LONG.
11	1	300F x F 90° BEND
12	1	300F x F SPOOL PIECE, 500mm LONG.
13	1	300F x F SPOOL PIECE, 4975mm LONG.
14	1	300mm STRAIGHT STANDARD BELL MOUTH CASTING
15	1	150F x PE SPOOL PIECE, 3500mm LONG.
16	1	150F x F SPOOL PIECE, 6000mm LONG.
17	1	150F x F 90° BEND
18	1	150mm STRAIGHT STANDARD BELL MOUTH CASTING
19	1	250F x PE SPOOL PIECE, 5040mm LONG, c/w 11.25° WELDED BEND AT 3240mm FROM PE.
20	2	250F x F SPOOL PIECE, 6000mm LONG.
21	1	250F x F 90° BEND
22	1	250mm STRAIGHT STANDARD BELL MOUTH CASTING



- NOTE:**
- ALL PIPING TO BE SCHEDULE 40 STEEL.
 - ALL SURFACES OF PIPING TO BE COATED WITH EPOXY OR MARINE ENAMEL ACCORDING TO AWWA STD-210.
 - ELEVATIONS DERIVED FROM MONUMENT ON SOUTH-EAST CORNER OF D.L. 572, ELEV. 392.560m
 - THE FOLLOWING NOTATIONS HAVE BEEN USED IN THE SCHEDULE OF FITTINGS:
 - a) F - CLASS 150 STEEL FLANGE, AWWA STD C-207.
 - b) H - PUSH ON RUBBER GASKET BELL JOINT AS DESCRIBED IN AWWA STD. C-111.
 - c) PE - PLAIN END.

NO.	DATE	BY	REVISION
A	JAN/96	C.C.	CHANGED LOCATION OF FILL/DISCHARGE LINE



DRAWN	C.C.
DESIGN	J.G.
APPROVED	S.M.
DATE	MARCH 96
SCALE	AS SHOWN

OYAMA SERVICE AREA		DRAWING NO.	REV. NO.
OYAMA RESERVOIR		OID-120	A
SITE PLAN AND			
MECHANICAL DETAILS			

206 - 437 Glenmore Road, Kelowna, B.C. V1V 1Y5 Telephone 888-2072

GENERAL SPECIFICATIONS

The following specifications are a summary of the "Tender Documents", any conflicting statements are to be superseded by the "Tender Documents" specification.

Any deviation from these specifications or drawings without the written consent of the engineer will result in the contractor / owner correcting the deviation.

1. DESIGN LOADS:

ALL LOADS ARE IN KPa (P.S.F.)
 ROOF LIVE LOAD: 2.4 KPa (50 P.S.F.)
 SUPERIMPOSED ROOF DEAD LOAD: 8 KPa (120.0 P.S.F.)
 ROOF OVERBURDEN DENSITY: NOT TO EXCEED 20 KN/CU. METER (125 LBS./CU.FT)

Footings are proportioned for soil bearing capacity of 192.0 KPa (4000 LBS./SQ.FT)
 The footings bear on natural inorganic earth. No footings bear higher than on a 1.5 horizontal to 1.0 vertical slope above any adjacent excavation, planned or existing.

2. REFERENCE STANDARDS:

1. British Columbia Building Code 1992
2. National Building Code of Canada 1990
3. G30.12-M1977, "Billet Steel Bars for Concrete Reinforcement"
4. CAN3-A371-M84, "Masonry Construction for Buildings"
5. CAN3-A185.1-M85, "Concrete Masonry Units"
6. CAN/CSA-A82.1-M87, "Burned Clay Bricks" (Solid masonry units made from clay or shale)
7. CAN/CSA-A23.2-84, "Methods of Test for Concrete"
8. CAN/CSA-A23.2-84, "Methods of Test for Concrete"
9. ACI Standard 301-72, "Specifications for Structural Formwork"
10. CAN/CSA-A23.3-M84, "Design of Concrete Structures for Concrete Buildings"
11. CAN/CSA-A5-M88, "Portland Cement"
12. S288.1-1975, "Trade Work for Construction Purposes"
13. CAN/CSA-O122-M88, "Structural Glue Laminated Timber"
14. CAN/CSA-Q40.21-M91, "Structural Quality Steels"
15. CSA-W69-M1984, "Welded Steel Construction" (Metal Arc Welding)
16. CSA W48, "Welding Electrodes"
17. CAN/CSA-S16.1-84, "Limit States Design for Steel Structures"

3. FIELD REVIEW BY 4 DEE ENGINEERING (1993) LTD:

4 Dee Engineering (1993) Ltd. provides field review only for the work shown on these structural drawings. This review is not a "Full Time" review, but is periodic review at the sole discretion of 4 Dee Engineering (1993) Ltd. in order to ascertain that the work is in general conformance with the plans and supporting documents prepared by 4 Dee Engineering (1993) Ltd. Field review by 4 Dee Engineering (1993) Ltd. is not carried out for the contractor's benefit, nor does it make 4 Dee Engineering (1993) Ltd. guarantors of the contractor's work. It remains the contractor's responsibility to build the work in conformance with the contract documents.

4. EXCAVATION:

Excavate to lines and levels necessary to complete the work. The contractor shall provide shoring during excavation, as required. Comply with governing safety regulations. Protect subgrade footings/slab against changes in moisture and/or freeze-thaw action during construction.

5. FOOTINGS:

Footings are proportioned for a soil bearing capacity of 4,000 P.S.F. Footings shall bear on natural, inorganic earth. No footing shall bear higher than on a 1.5 horizontal to 1.0 vertical slope above any adjacent excavation, planned or existing. Bearing capacity to be confirmed in writing by Geotechnical Engineer.

6. REINFORCING STEEL:

Reinforcing steel to CSA-G30.12-M1977, Grade 400. Welded wire fabric to CSA-G30.5-M1977. Reinforcing work should be in accordance with CAN/CSA-A23.3. Lap horizontal bars 30 x bar diameter; vertical bars 24 x bar diameter. Increase bar laps 20% for a bar spacing less than 6". Minimum 2-15M reinforcing bars around openings; extend reinforcing bars 24" minimum past corners. Provide corner bars for all walls reinforcing.

REINFORCEMENT BAR SPLICE LENGTH TABLE

REBAR SIZE (MPa)	GRADE	COMPRESSION SPLICE (MIN.)	TENSION SPLICE LENGTH (MIN.) PER CONCRETE STRENGTH			
			20 MPa	25 MPa	30 MPa	35 MPa
10M	400	330mm (13")	460mm (18")	460mm (18")	480mm (18")	480mm (18")
15M	400	480mm (19")	635mm (25")	635mm (25")	635mm (25")	635mm (25")
20M	400	585mm (23")	890mm (35")	790mm (31")	790mm (31")	790mm (31")
25M	400	740mm (29")	1145mm (45")	1285mm (51")	1170mm (46")	1095mm (43")
30M	400	865mm (34")	1205mm (47")	1805mm (71")	1850mm (73")	1525mm (60")
35M	400	1040mm (41")	3000mm (118")	2590mm (102")	2385mm (93")	2185mm (86")

6. REINFORCING STEEL (cont.):

1. Top bar splice to be increase by a factor of 1.3. Top bar splice as any horizontal bar with 12" or more of concrete below.
 2. Where splices are dimensioned on the drawings, such dimensions shall apply.
- Support slab reinforcing as suitable chairs or supports not further than 48" in either direction. Supply support bars, chairs, and covers as necessary.

TYPICAL CLEARANCE FOR REINFORCING STEEL:

Surface Poured against Ground	75mm (3")
Formed Surfaces Exposed to Ground or Weather	50mm (2")
Beams	50mm (2") TO MAIN STEEL
Columns	50mm (2") TO MAIN STEEL
Slabs, Walls	50mm (2")

7. CONCRETE:

Concrete materials and methods of construction to CAN/CSA-A23.1-94. Provide concrete for and cooperate in the preparation of test cylinders. Take 3 cylinders for every 100 cubic yards or less of concrete placed. A minimum of one test of 3 cylinders for each pour.

All concrete normal weight, 150 P.C.F., type 10 cement with Min. 15% replacement type F flyash.

Submit proposed mix design to the engineer for approval.

Do not use admixtures other than air entrainment, standard water reducers, or super plasticizers without prior approval of the engineer. Super plasticizer is required.

Reject all concrete when time between batching and placing exceeds 2 hours.

Water may be added to the concrete on site to adjust the slump to specified range.

Concrete to be pumped with pipe "in wall". "Elephant Trunk" is NOT to be used.

Consolidate all concrete using mechanical vibrators.

Construct formwork in accordance with WCB IHAS 34.28 and CSA-S288.1-1975. Formwork design is the responsibility of the contractor.

Protect concrete from adverse weather conditions in accordance with CSA-A23.1 or as determined by the engineer.

In cold, below freezing temperatures, approved cold weather concreting procedures must be followed. Do not place concrete on frozen ground or frozen reinforcing bars.

TYPE OF CONCRETE	STRENGTH @ 28 DAYS	AIR CONTENT	MAX. SLUMP	MAX. AGG. SIZE	EXPOSURE CLASS	WATER/CEMENT RATIO
ALL RESERVOIR CONCRETE	32MPa (4600 PSI)	5% - 6%	63.5mm (2.50")	20mm (0.75")	C-1	0.40

Non structural cracks are a function of concrete's intrinsic materials, and tend to be unpredictable. A majority of these cracks can be limited or controlled, although circumstances do arise when they cannot be avoided.

8. CONCRETE BOND BREAKERS:

Oil based bond breaker is NOT to be used on construction or control joints.

9. CONCRETE ROOF SLAB & MAIN ROOF BEAM:

All slabs and beams to be combined .002 X clear span (inches).

10. GROUT

Shall be non-shrink, minimum strength 35 MPa @ 28 days.

11. STRUCTURAL STEEL:

Structural steel shall be in accordance with CSA Standards CAN/CSA-G40.21-M91, grade 300W. All U.S.S. columns shall be grade 350W.

12. PRIMERS:

For all steel: C.I.S.C./C.P.M.A. Standard 1-73A primer, or other pre-approved. Field touch up all welds with primer after slag has been removed.

13. STRUCTURAL DRAWINGS:

Read structural drawings in conjunction with the Geotechnical and Mechanical drawings for detailed dimensions of doors, ducts, openings, pipes etc.

Check and verify all dimensions with the drawings before commencing with any work. Notify 4 Dee Engineering (1993) Ltd. of any errors or omissions.

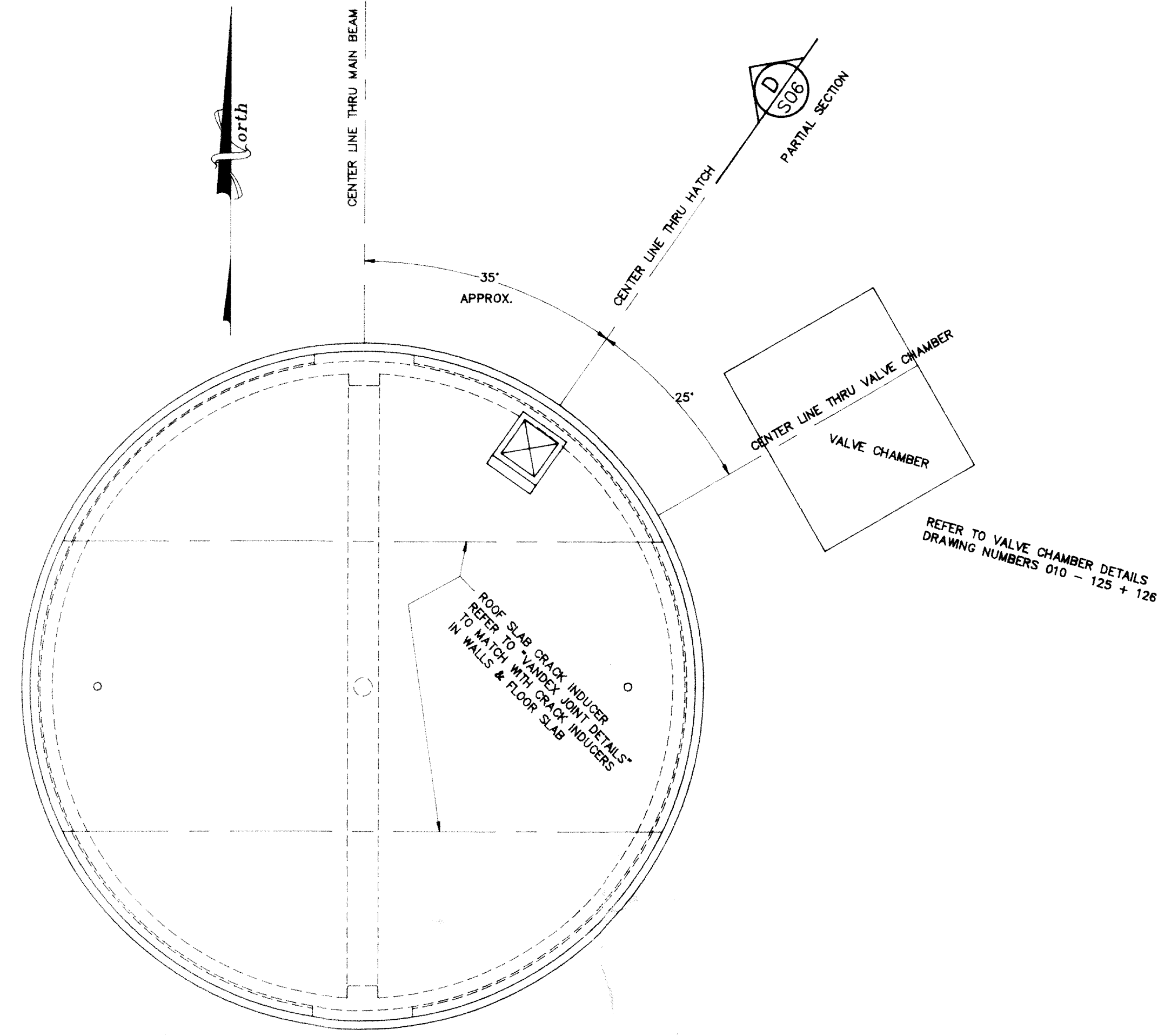
Drawings show completed structures only. Temporary bracing for construction loading conditions is the responsibility of the contractor.

Do not construct from these drawings unless marked "ISSUED FOR CONSTRUCTION"

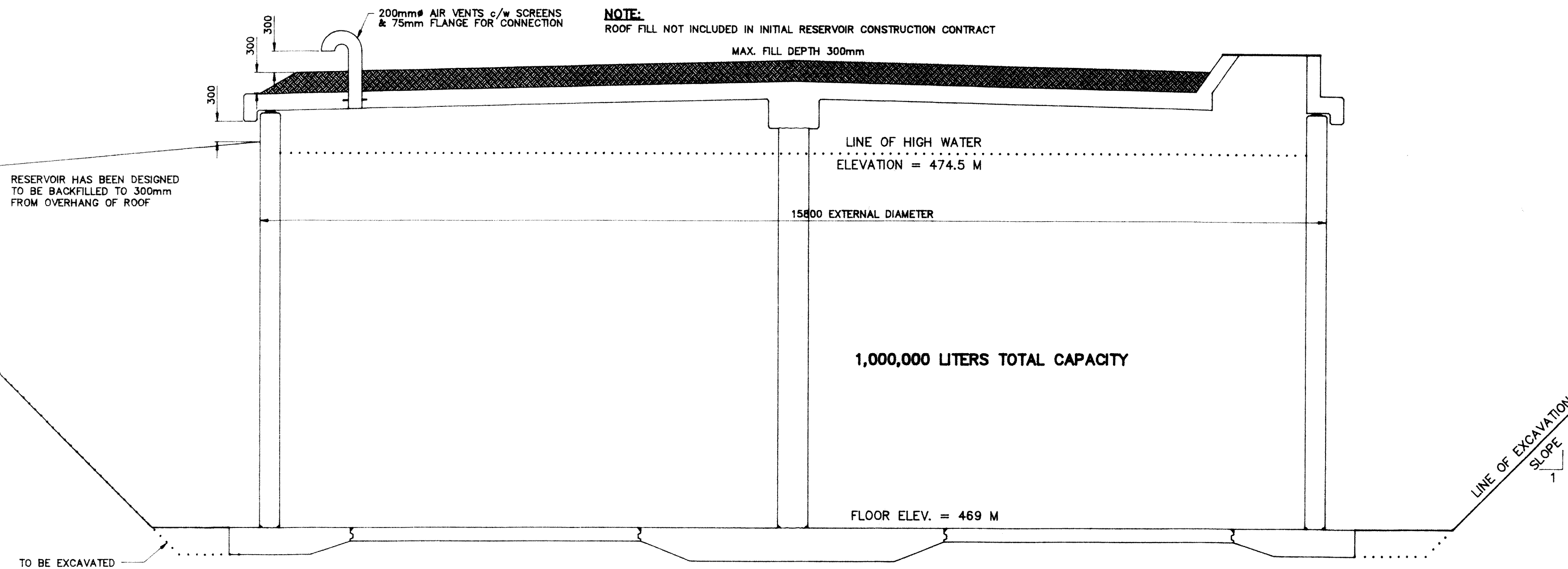
14. NOTICE OF INSPECTIONS:

4 Dee Engineering (1993) Ltd., Mould Engineering Ltd & Geotechnical Engineer to be notified 48 hours in advance for the following inspections & approval

REINFORCING STEELS	BEFORE EACH CONCRETE POUR
PIPING	BEFORE EACH CONCRETE POUR
FOUNDATION SOILS	BEFORE BACKFILLING



PLAN VIEW
1:100



ELEVATION
1:50

4Dee ENGINEERING (1993) LTD.
 200-1974 MOSS COURT
 KELOWNA, B.C. V1Y-9L3
 PHONE:(604) 762-6488 FAX:(604) 762-8311

THE GENERAL CONTRACTOR SHALL CHECK THE DRAWING FOR ERRORS & OMISSIONS PRIOR TO CONSTRUCTION. REPORT ALL DISCREPANCIES TO THE ENGINEER IMMEDIATELY. UNAUTHORIZED REPRODUCTION PROHIBITED.

SEAL:

 THIS DESIGN IS FOR THE COMPLETED STRUCTURE ONLY.

PROJECT TITLE
REGIONAL DISTRICT OF CENTRAL OKANAGAN OYAMA RESERVOIR
 OYAMA B.C.

PROJECT CONSULTANT

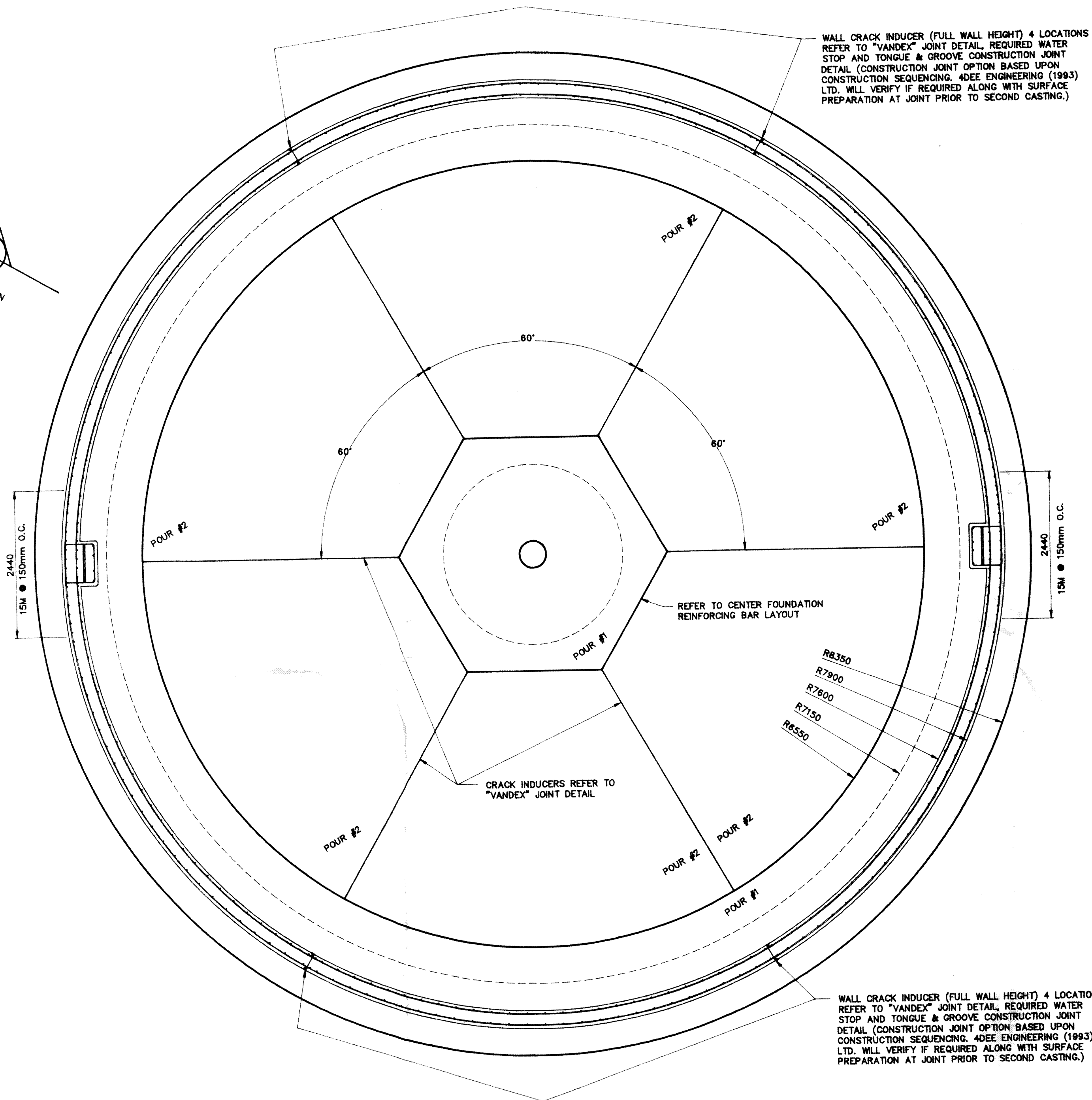
 206 - 437 GLENMORE RD. KELOWNA, BC V1Y 1Y5

DRAWING TITLE
ROOF PLAN VIEW CROSS SECTION ELEVATION

DATE	FILE No.
1995-09-08	95-2872
DRAWN BY	DESIGNED BY
T.M.K.	K.M.E.
SCALE	ISSUED
AS SHOWN	1996-03-07
DRAWING NUMBER	OF
2872-01	6 S01

A
S03
PARTIAL SECTION

B
S04
PARTIAL SECTION



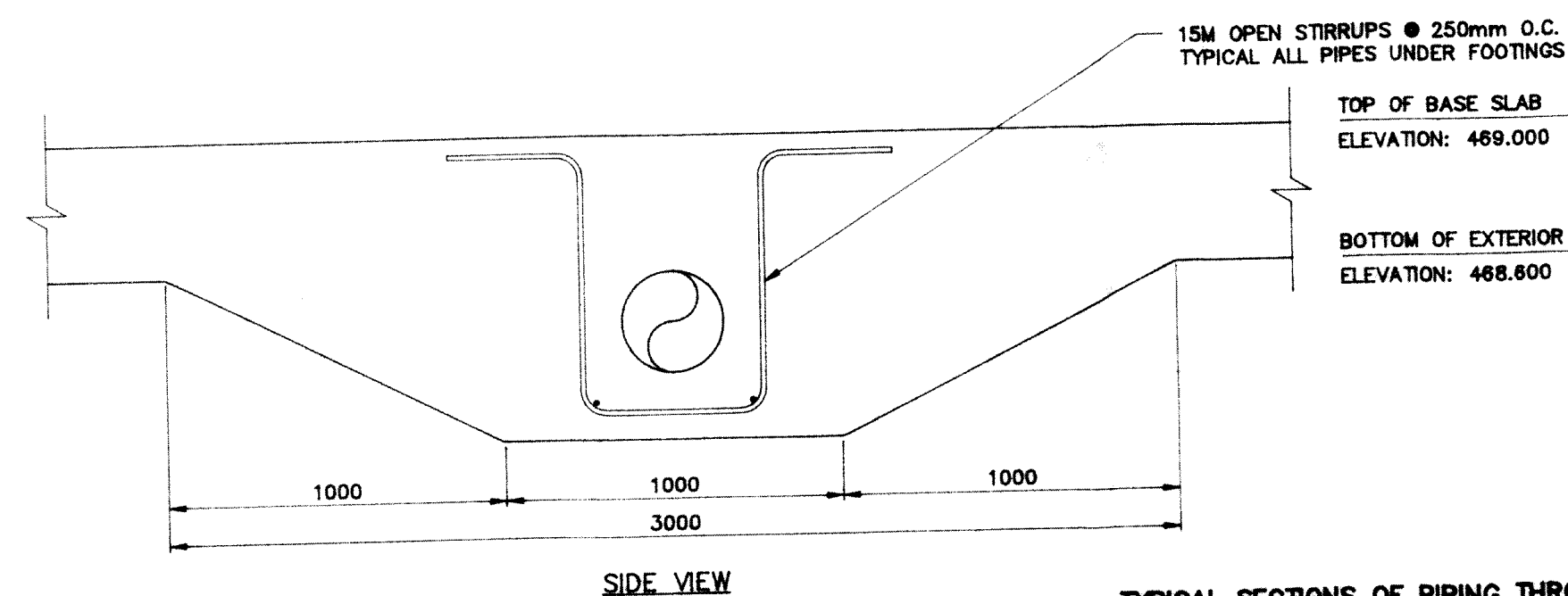
FOUNDATION PLAN
SCALE 1:50

WALL CRACK INDUCER (FULL WALL HEIGHT) 4 LOCATIONS
REFER TO "VANDEX" JOINT DETAIL, REQUIRED WATER
STOP AND TONGUE & GROOVE CONSTRUCTION JOINT
DETAIL (CONSTRUCTION JOINT OPTION BASED UPON
CONSTRUCTION SEQUENCING, 4DEE ENGINEERING (1993)
LTD. WILL VERIFY IF REQUIRED ALONG WITH SURFACE
PREPARATION AT JOINT PRIOR TO SECOND CASTING.)

WALL CRACK INDUCER (FULL WALL HEIGHT) 4 LOCATIONS
REFER TO "VANDEX" JOINT DETAIL, REQUIRED WATER
STOP AND TONGUE & GROOVE CONSTRUCTION JOINT
DETAIL (CONSTRUCTION JOINT OPTION BASED UPON
CONSTRUCTION SEQUENCING, 4DEE ENGINEERING (1993)
LTD. WILL VERIFY IF REQUIRED ALONG WITH SURFACE
PREPARATION AT JOINT PRIOR TO SECOND CASTING.)

NOTE: ALL PIPE PENETRATIONS THROUGH CONCRETE

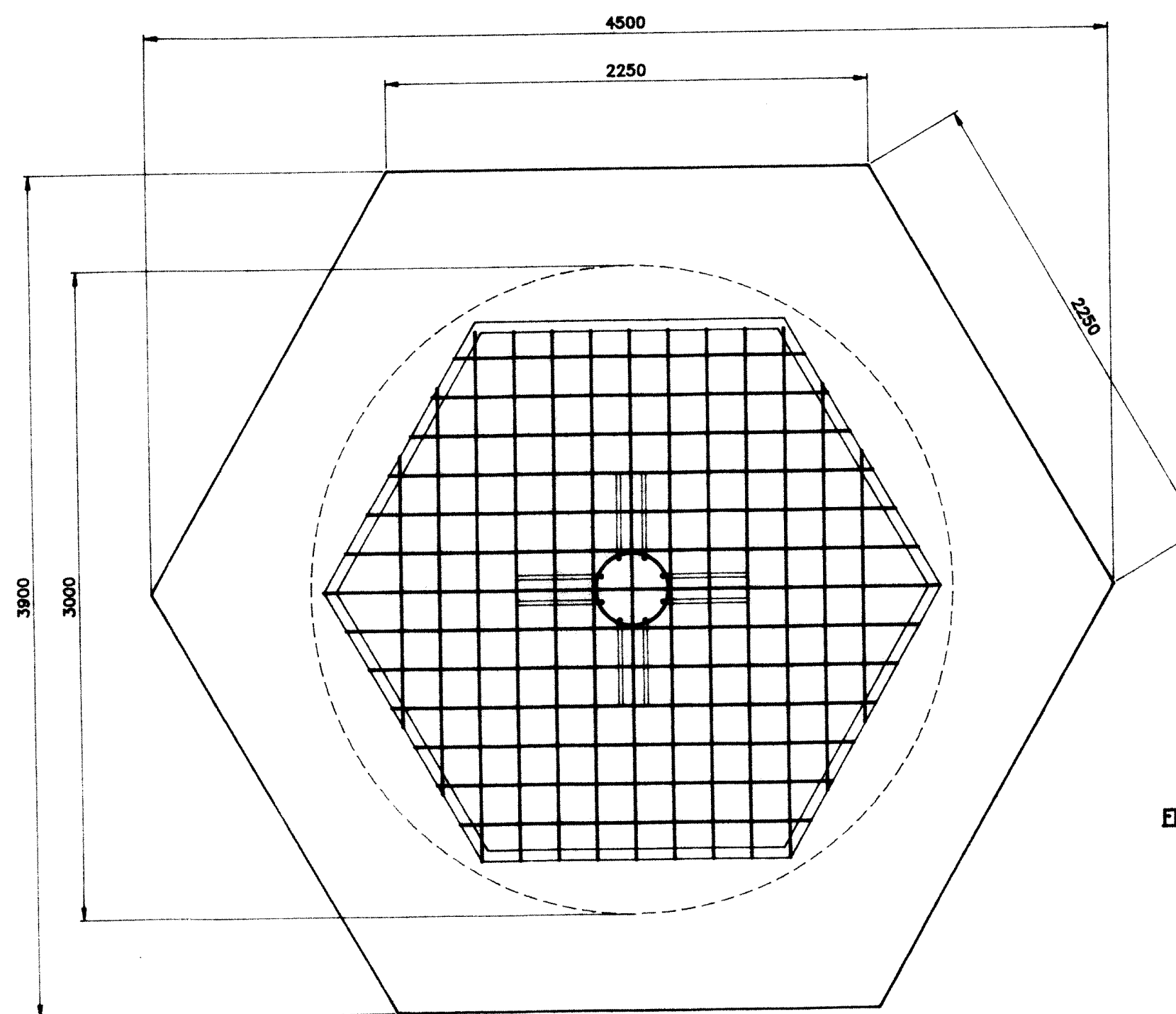
ALL PIPES PASSING THROUGH EITHER THE CONCRETE SLAB-ON GRADE
OR CONCRETE WALLS TO CONTAIN A "VOLCLAY" SEALING "WATERSTOP-RX"
STRIP 18mm x 18mm LOCATED IN THE CENTER OF WALL / FLOOR SLAB
WITH 25mm² FOAM ROD TIED AROUND EXPOSED SURFACES OF THE PIPE. AFTER
THE CONCRETE IS CURED THE VOID FROM THE FOAM ROD IS PACKED WITH "VANDEX"
(% TOP OF FLOOR SLAB, INSIDE & OUTSIDE OF WALLS) INSTALL 2 - 10MM DIAGONALS
1000mm LONG (CENTERED) TO SPECIFIED REINFORCING



TYPICAL SECTIONS OF PIPING THROUGH FOUNDATIONS
SCALE 1:50

TYPICAL SECTION

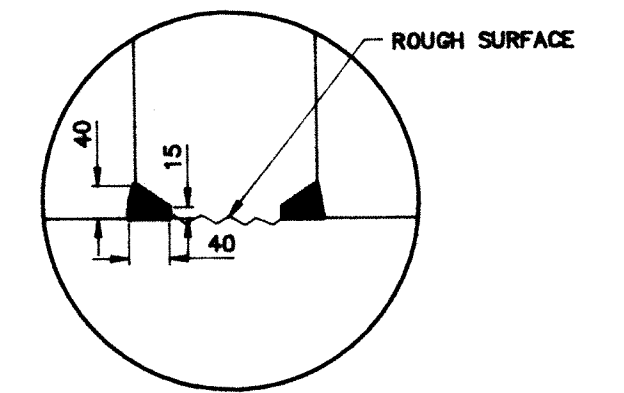
FOR RESERVOIR MECHANICAL DETAILS
REFER TO DWG. NO 010 TO 120A



REFER TO DRAWING #S3 FOR REINFORCING BAR SIZE
CENTER FOUNDATION REINFORCING BAR LAYOUT
SCALE 1:25

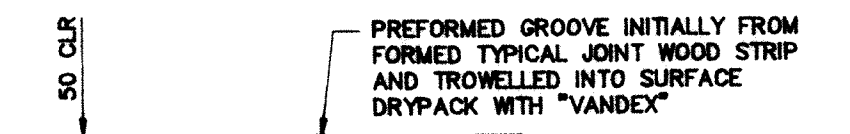
GENERAL CONSTRUCTION NOTES

- FOUNDATIONS TO BE CAST AS PER SEQUENCE INDICATED ON FOUNDATION PLAN
- NO HORIZONTAL COLD JOINTS ALLOWED IN THE EXTERIOR WALL. (CAST FULL HEIGHT)
- THE EXTERIOR WALL MAY BE CAST IN VERTICAL SECTIONS WITH THE COLD JOINTS IN THE INDICATED LOCATIONS ON THE FOUNDATION PLAN. NO ADDITIONAL COLD JOINTS ALLOWED IN WALL WITHOUT WRITTEN APPROVAL BY 4DEE-ENGINEERING
- ROOF SLABS & FLOOR SLABS / FOUNDATIONS TO BE POWER TROWELLED FINISHED WITH A WET CURE TO FOLLOW. WET CURE FOR A MIN. OF 7 DAYS WITHIN A TEMPERATURE RANGE OF +4° TO +20°. BURLAP IS TO BE USED TO MAINTAIN THE MOISTURE WITH A POLY COVER AND / OR INSULATED BLANKET COVER.

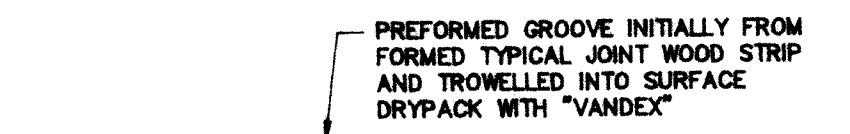


NOTE:
GROOVE FILLED WITH "VANDEX" CONCENTRATE "DRY-PACK"
AND PACKED TIGHTLY. (LEFT PROUD)

PREFORMED JOINT DETAIL



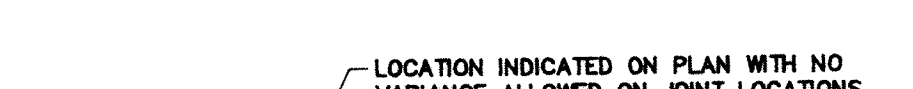
ROOF JOINT



FLOOR SLAB ON GRADE (PERPENDICULAR TO FOOTINGS)



FLOOR JOINT (TYPICAL EXTERIOR & INTERIOR FOOTINGS)



WALL JOINT WITHOUT TONGUE & GROOVE CONSTRUCTION JOINT



WALL JOINT WITH TONGUE & GROOVE CONSTRUCTION JOINT



NOTE:
THE TONGUE & GROOVE CONSTRUCTION JOINTS FOR THE WALLS TO BE FORMED
WITH A 2x6 AND THE FLOOR SLAB WITH A 2x4 WITH THE CORNERS BACK CUT @
22.5°. THE ROOF SLAB IS TO BE CAST MONOLITHICALLY WITH THE PREFORMED JOINT
DETAIL CAST IT INTO THE POUR.

TONGUE & GROOVE CONSTRUCTION JOINTS
SCALE 1:20

CAST-IN-PLACE CONCRETE SURFACE TREATMENT

A slurry of "VANDEX" or approved equivalent shall be applied at a rate of 1 kg/sq meter in two consecutive coats to the entire structure (inside & outside) based on the following requirements: "DRY-PACK" all routed voids

Horizontal Surfaces:

1. Rout out all viable cracks exceeding .254mm (.01") in width to a depth of 19mm (3/4")
2. Remove all laitance (cement soom), dirt, dust and then rinse with water to all concrete surfaces.
3. All free laying water must be removed the day prior to the slurry application.

Vertical Surfaces:

1. Remove all form scale, oil, form release agents and any other foreign materials likely to affect the bond, penetration, and performance of the "VANDEX" waterproofing.
2. Form ties holes shall be left approx. 25mm back of surface and patched prior to slurry application.
3. Honeycomb pockets and faulty construction joints shall be routed out to sound concrete.
4. Construction joints and visible cracks in concrete surfaces exceeding half-the size .254 (.01") shall be routed out to 19mm deep.
5. Rince all surfaces to be waterproofed thoroughly with water the day prior to the "VANDEX" application. Moisture must be present in the concrete strata to achieve maximum initial penetration of the active "VANDEX" chemicals. Surfaces shall be moist only (not wet), when the coatings are to be applied.

4Dee ENGINEERING (1993) LTD.
200-1974 MOSS COURT
KELOWNA, B.C. V1Y-9L3
PHONE:(804) 762-6488 FAX:(804) 762-8311

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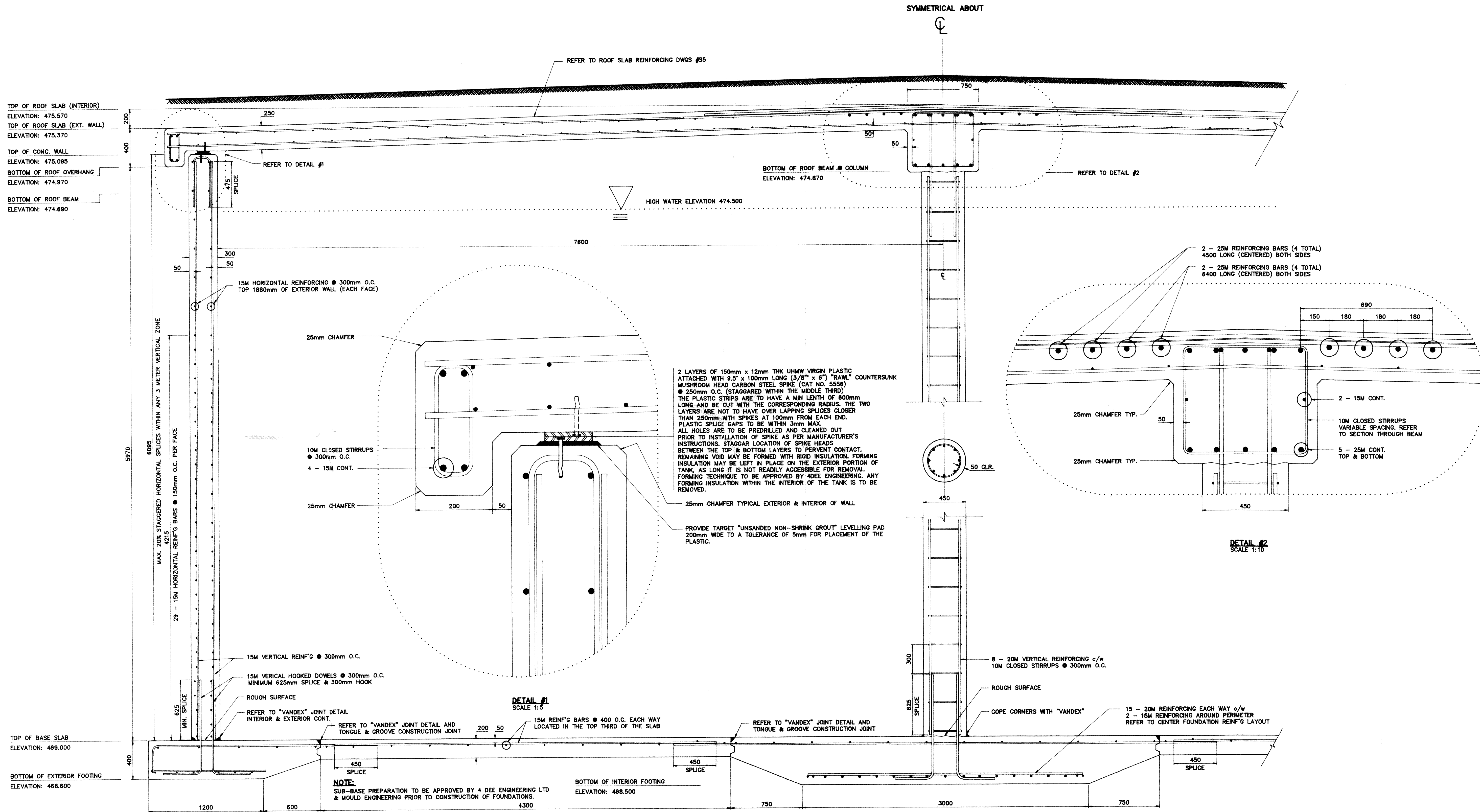
SEAL:
PROFESSIONAL ENGINEER
DENIS FRIE
BRITISH COLUMBIA

PROJECT TITLE
REGIONAL DISTRICT OF CENTRAL OKANAGAN
OYAMA RESERVOIR
OYAMA B.C.

PROJECT CONSULTANT
Mould Engineering
206 - 437 GLENMORE RD.
KELOWNA, BC
V1Y 1Y5

DRAWING TITLE
FOUNDATION PLAN
SECTIONS & DETAILS

DATE	1995-09-08	FILE No.	95-2872
DRAWN BY	T.M.K.	DESIGNED BY	K.M.E.
SCALE	AS SHOWN	ISSUED	1996-03-07
DRAWING NUMBER	2872-02	OF	6 S02



2 LAYERS OF 150mm x 12mm THK UHMW VIRGIN PLASTIC ATTACHED WITH 9.5" x 100mm LONG (3/8" x 6") "RAWL" COUNTERSUNK MUSHROOM HEAD CARBON STEEL SPIKE (CAT. NO. 5558) @ 250mm O.C. (STAGGERED WITHIN THE MIDDLE THIRD) THE PLASTIC STRIPS ARE TO HAVE A MIN LENGTH OF 600mm LONG AND BE CUT WITH THE CORRESPONDING RADIUS. THE TWO LAYERS ARE NOT TO HAVE OVER LAPPING SPLICES CLOSER THAN 250mm WITH SPIKES AT 100mm FROM EACH END. PLASTIC SPLICE GAPS TO BE WITHIN 3mm MAX. ALL HOLES ARE TO BE PREDRILLED AND CLEANED OUT PRIOR TO INSTALLATION OF SPIKE AS PER MANUFACTURER'S INSTRUCTIONS. STAGGER LOCATION OF SPIKE HEADS BETWEEN THE TOP & BOTTOM LAYERS TO PREVENT CONTACT. REMAINING VOID MAY BE FORMED WITH RIGID INSULATION. FORMING INSULATION MAY BE LEFT IN PLACE ON THE EXTERIOR PORTION OF TANK, AS LONG IT IS NOT READILY ACCESSIBLE FOR REMOVAL. FORMING TECHNIQUE TO BE APPROVED BY 4DEE ENGINEERING. ANY FORMING INSULATION WITHIN THE INTERIOR OF THE TANK IS TO BE REMOVED.

PROVIDE TARGET "UNSANDED NON-SHRINK GROUT" LEVELLING PAD 200mm WIDE TO A TOLERANCE OF 5mm FOR PLACEMENT OF THE PLASTIC.

PARTIAL LONGITUDINAL SECTION (A) SCALE 1:20

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 KELOWNA, B.C. V1Y-9L3
 PHONE:(604) 782-8488 FAX:(604) 782-8311

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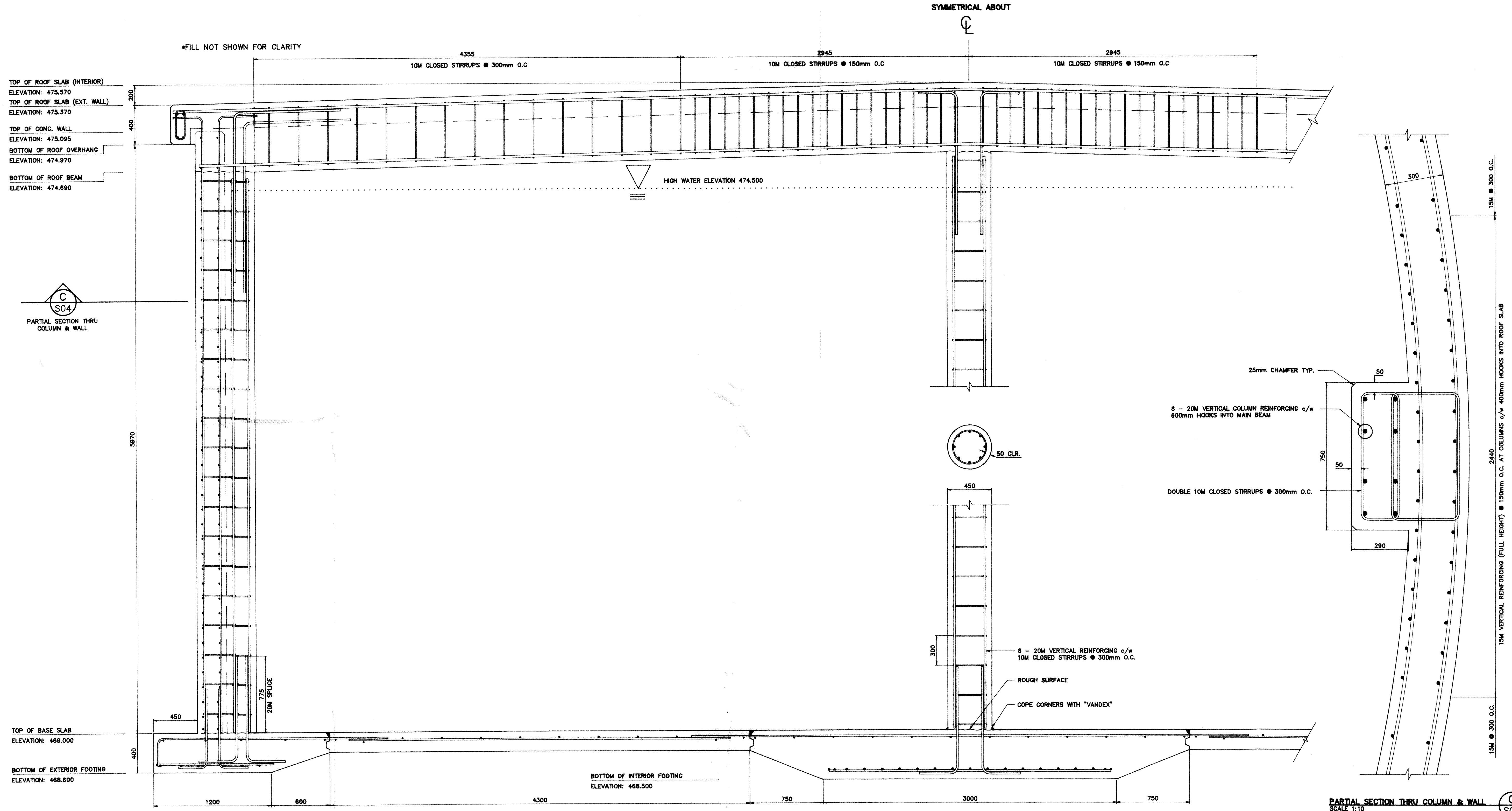
SEAL:

PROJECT TITLE
REGIONAL DISTRICT OF CENTRAL OKANAGAN OYAMA RESERVOIR
 OYAMA B.C.

PROJECT CONSULTANT

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DRAWING TITLE	
SECTIONS & DETAILS	
DATE 1995-09-08	FILE No. 95-2872
DRAWN BY T.M.K.	DESIGNED BY K.M.E.
SCALE AS SHOWN	ISSUED 1998-03-07
DRAWING NUMBER 2872-03 OF 6 S03	



*FILL NOT SHOWN FOR CLARITY

TOP OF ROOF SLAB (INTERIOR)
ELEVATION: 475.570
TOP OF ROOF SLAB (EXT. WALL)
ELEVATION: 475.370
TOP OF CONC. WALL
ELEVATION: 475.095
BOTTOM OF ROOF OVERHANG
ELEVATION: 474.970
BOTTOM OF ROOF BEAM
ELEVATION: 474.690

C
S04
PARTIAL SECTION THRU
COLUMN & WALL

TOP OF BASE SLAB
ELEVATION: 489.000
BOTTOM OF EXTERIOR FOOTING
ELEVATION: 468.600

PARTIAL LONGITUDINAL SECTION
SCALE 1:20
B
S02

PARTIAL SECTION THRU COLUMN & WALL
SCALE 1:10
C
S04

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200-1974 MOSS COURT
KELOWNA, B.C. V1Y-9L3
PHONE:(604) 782-8488 FAX:(604) 782-8311

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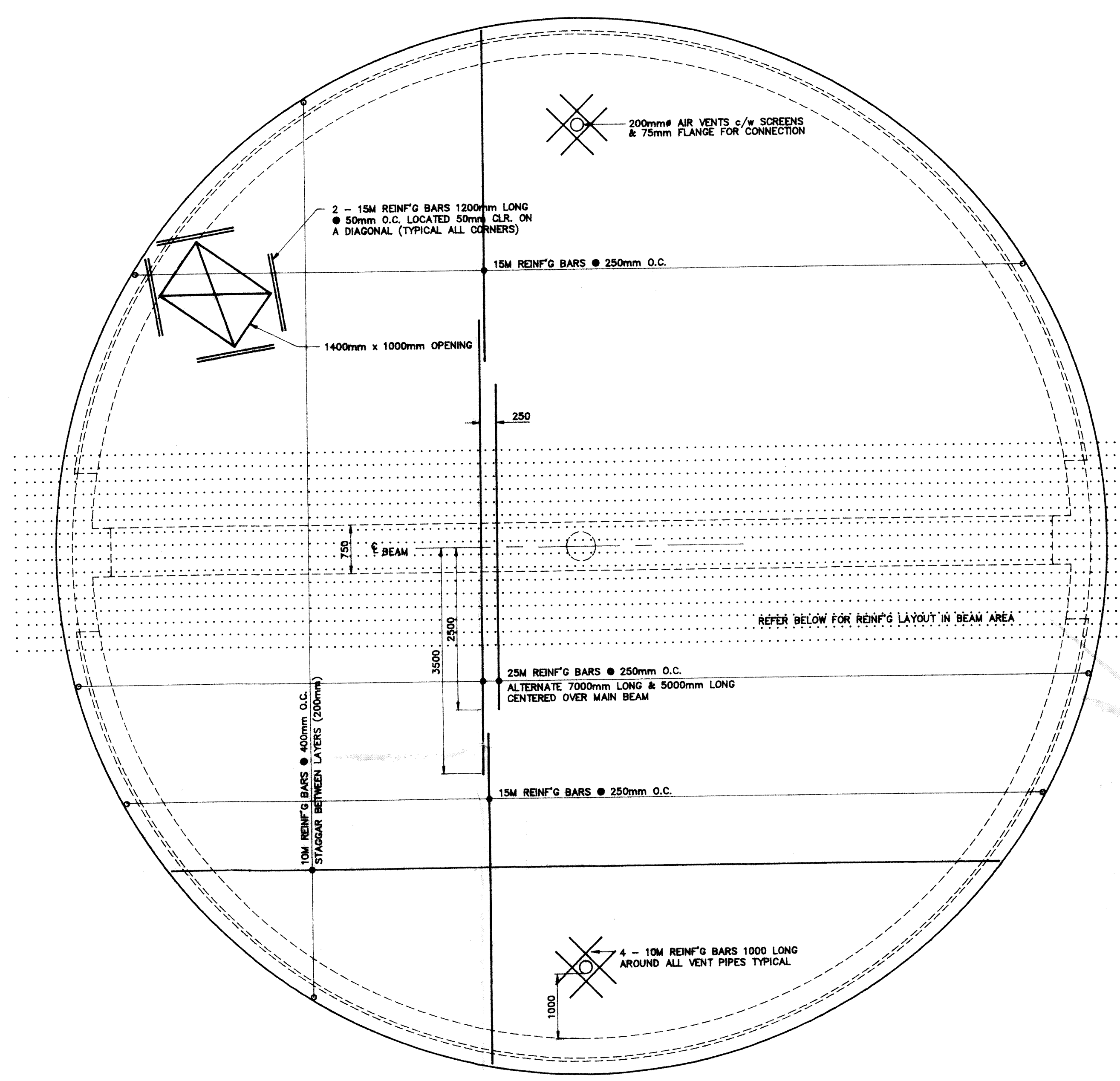
SEAL: PROFESSIONAL ENGINEER DENIS FRIE BRITISH COLUMBIA

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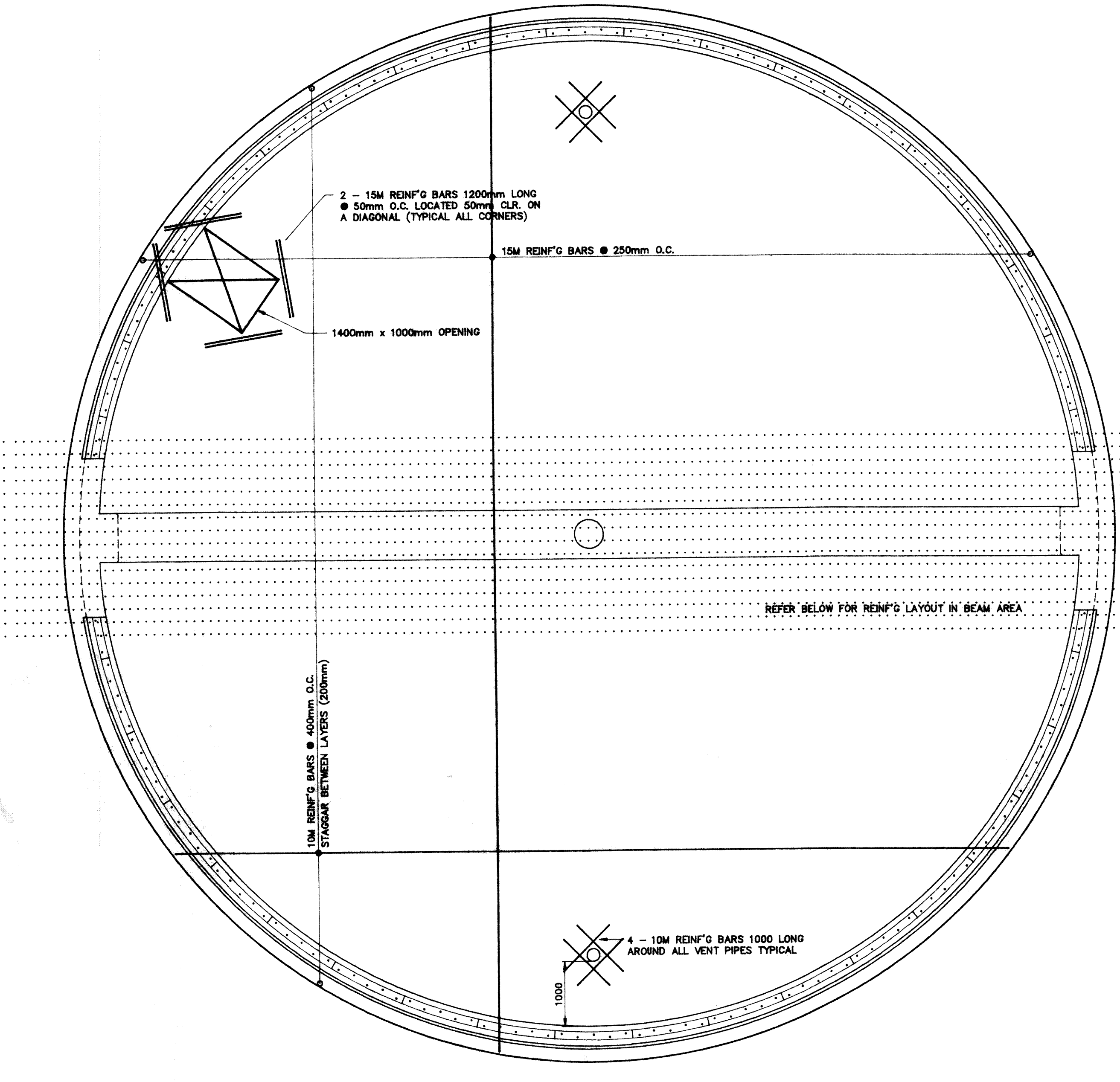
PROJECT TITLE
REGIONAL DISTRICT OF CENTRAL OKANAGAN OYAMA RESERVOIR
OYAMA B.C.

PROJECT CONSULTANT
Mould Engineering
208 - 437 GLENMORE RD.
KELOWNA, BC
V1Y 1Y5

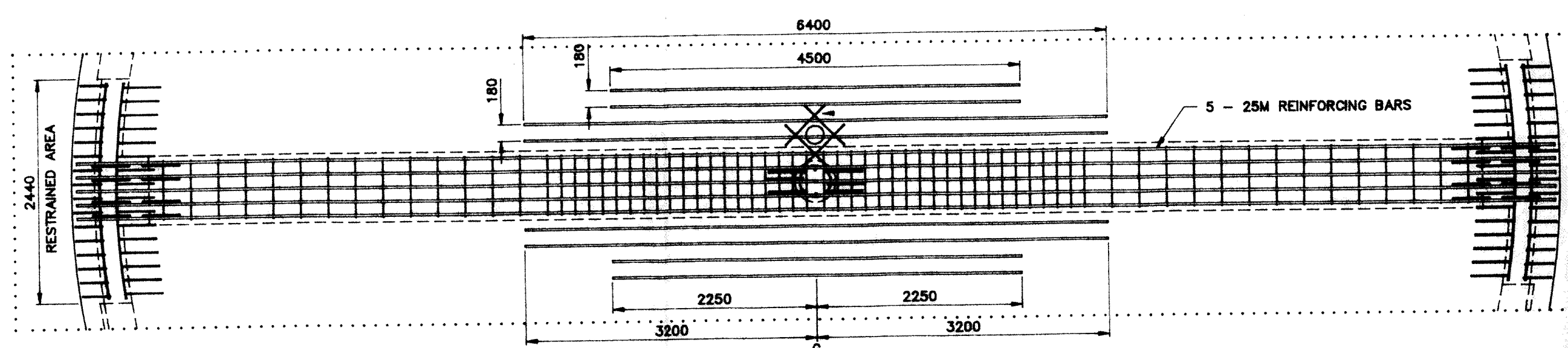
DRAWING TITLE	
SECTIONS & DETAILS	
DATE 1995-09-08	FILE No. 95-2872
DRAWN BY T.M.K.	CHECKED BY K.M.E.
SCALE AS SHOWN	ISSUED 1996-03-07
DRAWING NUMBER 2872-04 OF 6 S04	



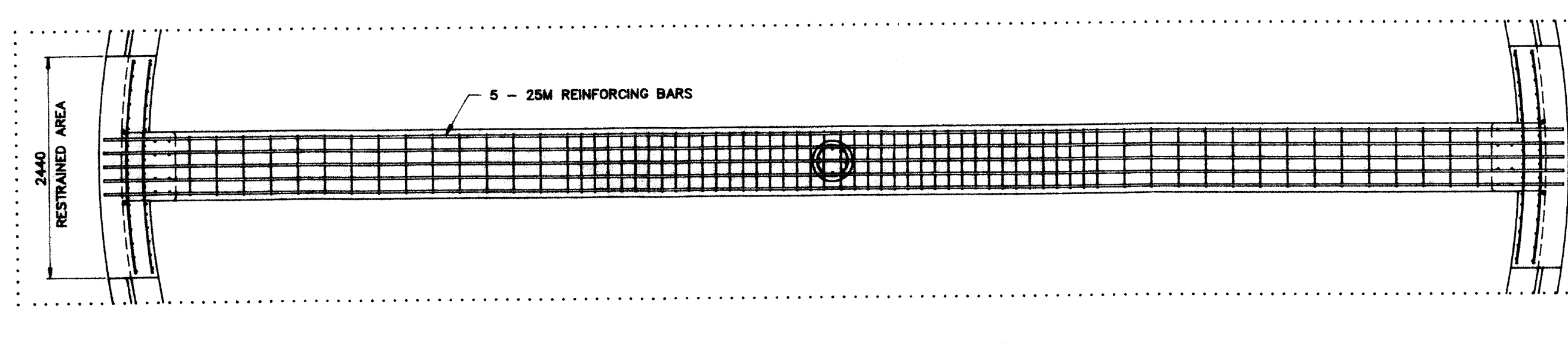
ROOF SLAB TOP REINFORCING LAYOUT
SCALE 1:50



ROOF SLAB BOTTOM REINFORCING LAYOUT
SCALE 1:50



ROOF BEAM TOP REINFORCING LAYOUT
SCALE 1:50



ROOF BEAM BOTTOM REINFORCING LAYOUT
SCALE 1:50

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200-1974 MOSS COURT
KELOWNA, B.C. V1Y-9L3
PHONE:(604) 782-6488 FAX:(604) 782-8311

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SEAL: PROFESSIONAL ENGINEER DENIS FRIE BRITISH COLUMBIA

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PROJECT TITLE
REGIONAL DISTRICT OF CENTRAL OKANAGAN OYAMA RESERVOIR OYAMA B.C.

PROJECT CONSULTANT
Mould Engineering
206 - 437 GLENMORE RD. KELOWNA, BC V1Y 1Y5

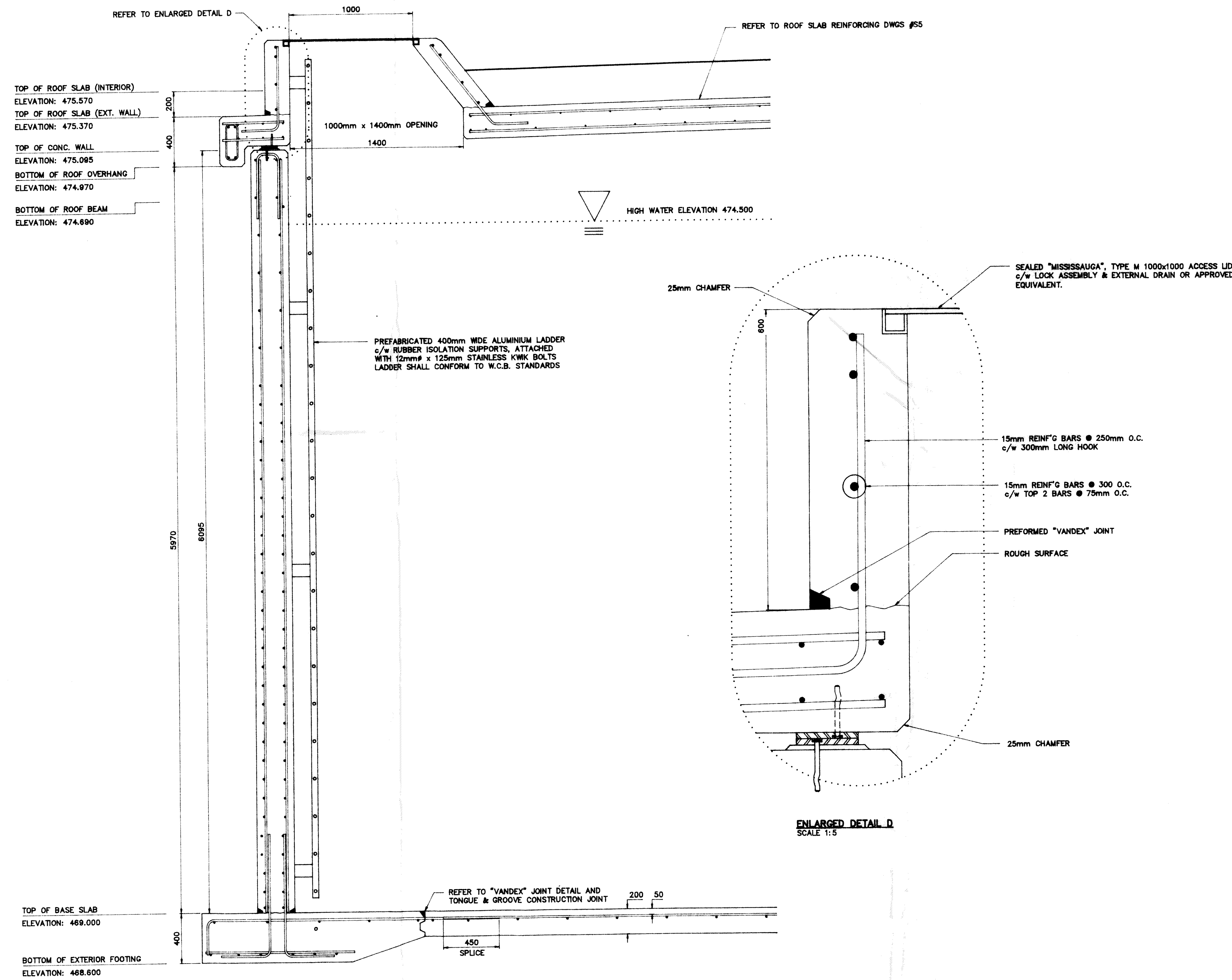
DRAWING TITLE
ROOF PLAN VIEW
CROSS SECTION ELEVATION

DATE 1995-09-08 FILE No. 95-2872

DRAWN BY T.M.K. DESIGNED BY K.M.E.

SCALE AS SHOWN ISSUED 1998-03-07

DRAWING NUMBER 2872-05 OF 6 S05

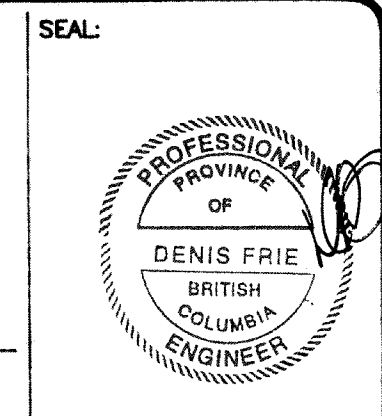


PARTIAL LONGITUDINAL SECTION D
S01
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ENLARGED DETAIL D
SCALE 1:5

4Dee ENGINEERING (1993) LTD.
 200-1974 MOSS COURT
 KELOWNA, B.C. V1Y-9L3
 PHONE:(804) 782-8488 FAX:(804) 782-8311

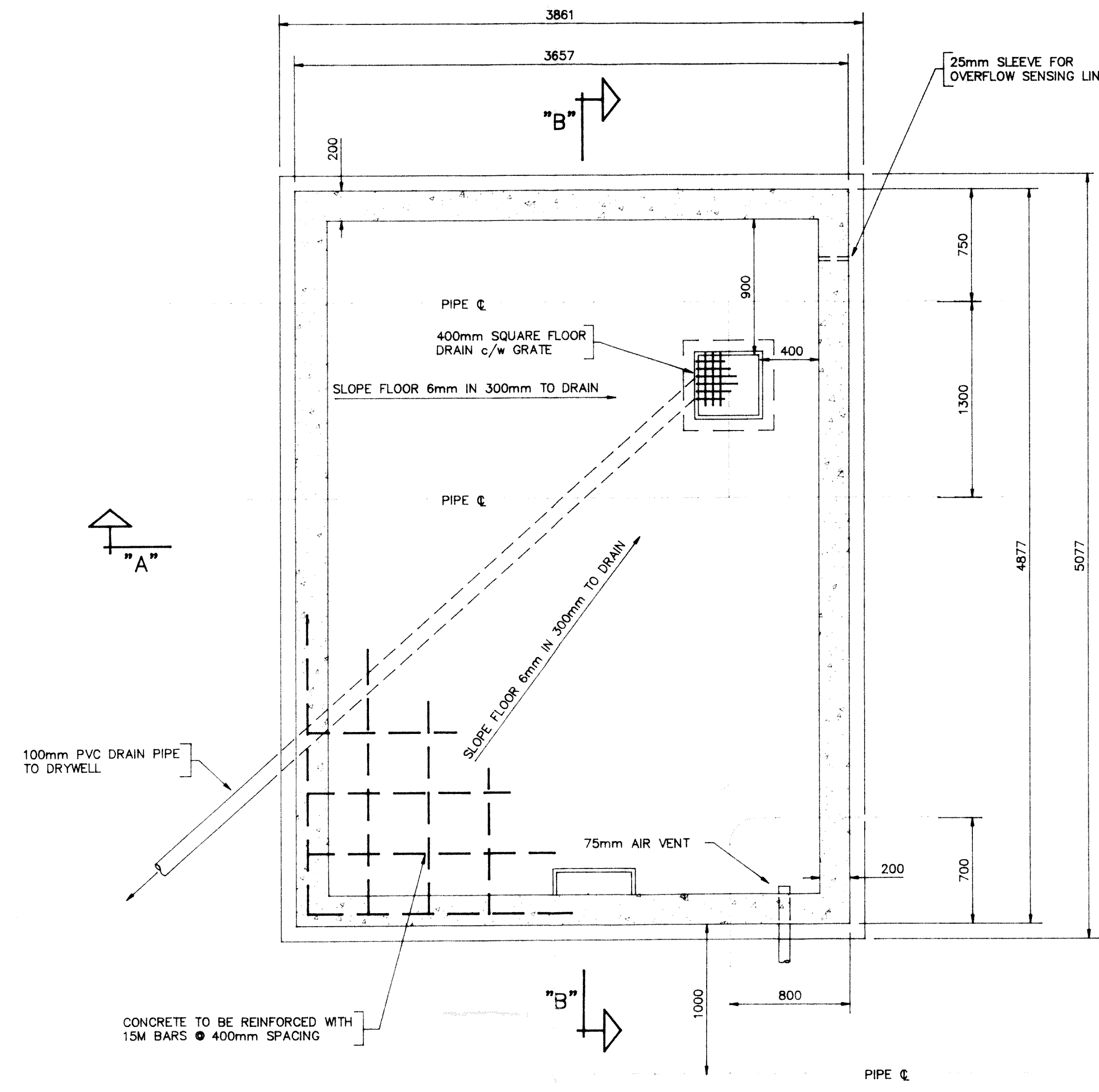
THE GENERAL CONTRACTOR SHALL CHECK THE DRAWING FOR ERRORS & OMISSIONS PRIOR TO CONSTRUCTION. REPORT ALL DISCREPANCIES TO THE ENGINEER IMMEDIATELY. UNAUTHORIZED REPRODUCTION PROHIBITED.



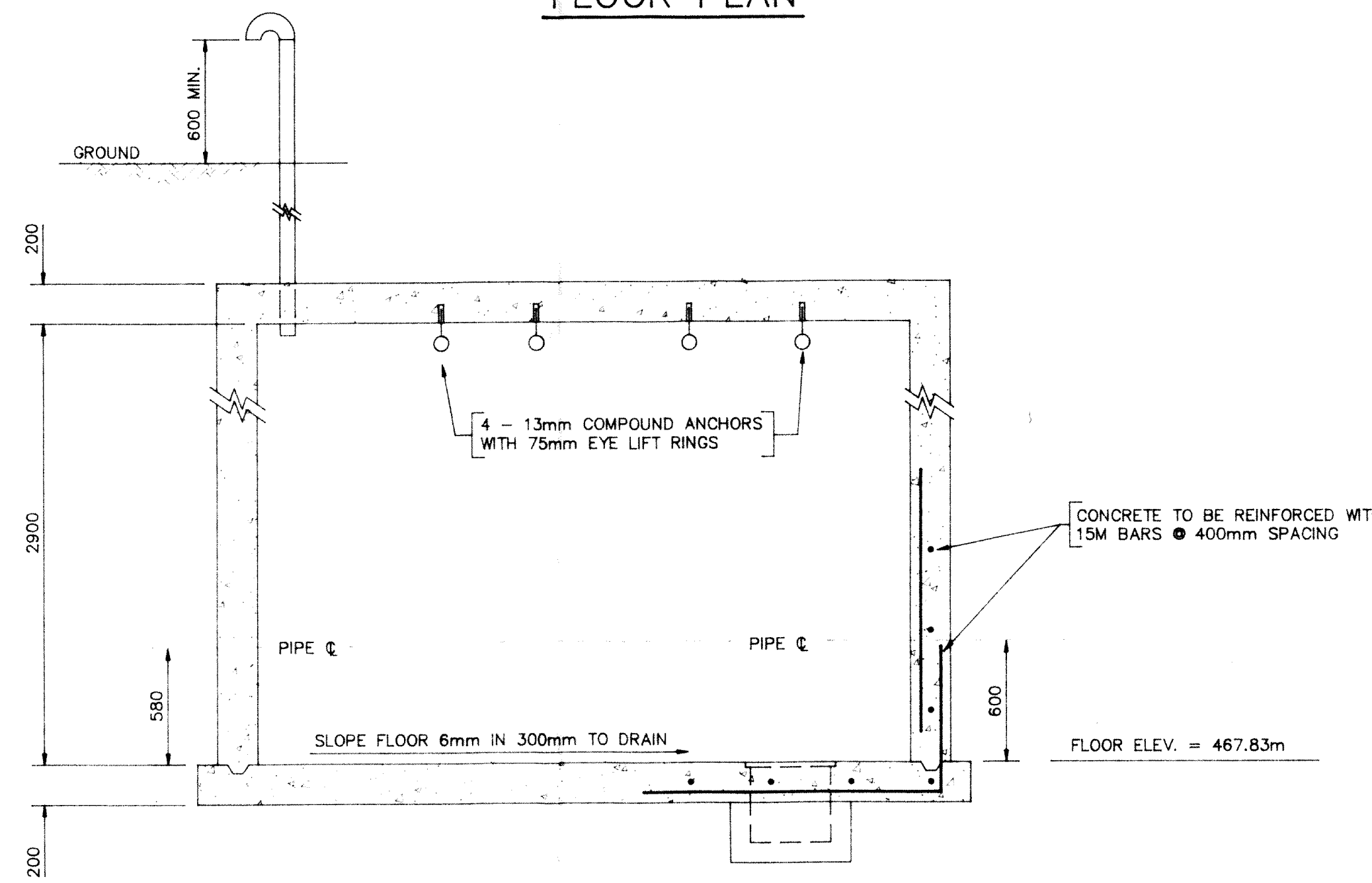
PROJECT TITLE
REGIONAL DISTRICT OF CENTRAL OKANAGAN OYAMA RESERVOIR OYAMA B.C.

PROJECT CONSULTANT
Mould Engineering
 208 - 437 GLENMORE RD.
 KELOWNA, BC
 V1Y 1Y5

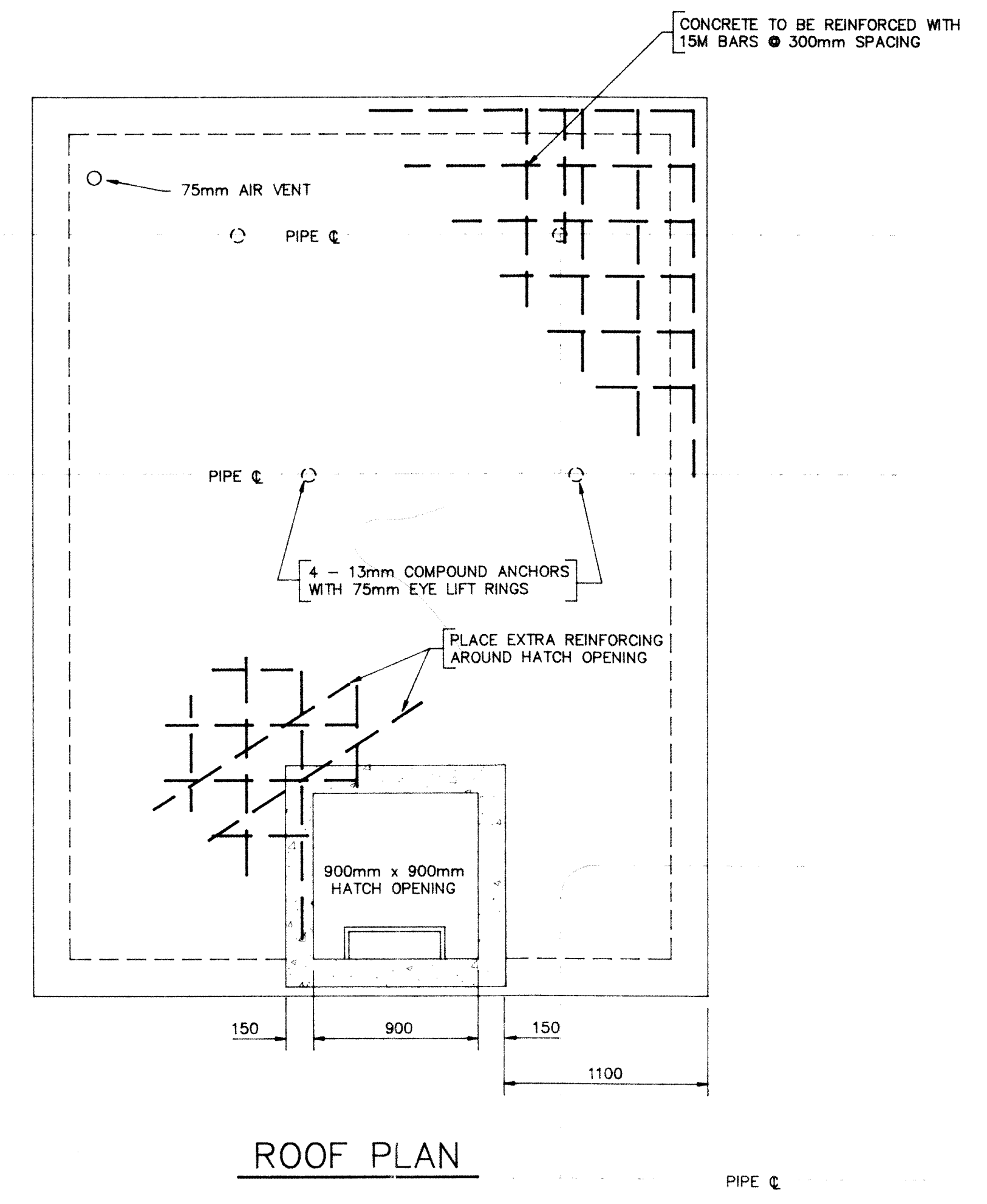
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ACCESS HATCH SECTIONS & DETAILS	
DATE 1995-09-08	FILE No. 95-2872
DRAWN BY T.M.K.	DESIGNED BY K.M.E.
SCALE AS SHOWN	ISSUED 1996-03-07
DRAWING NUMBER OF 2872-06 OF 6 S06	



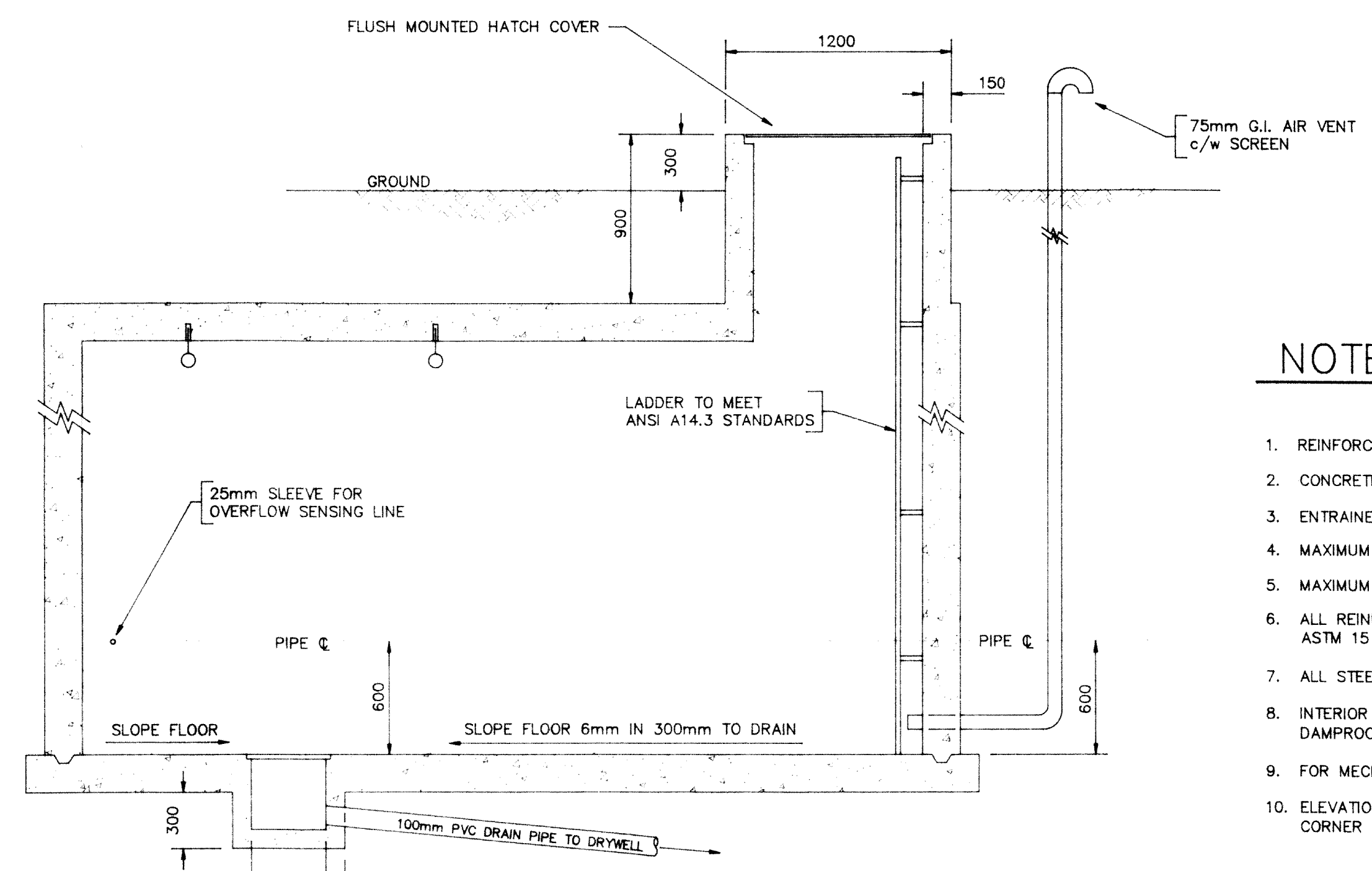
FLOOR PLAN



SECTION 'A-A'



ROOF PLAN

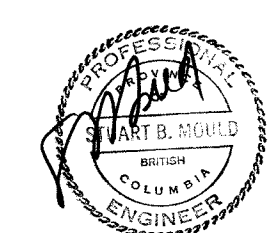
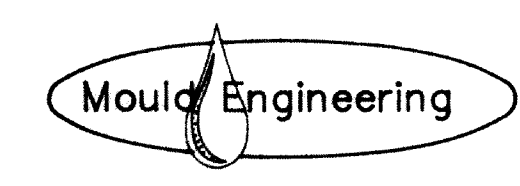


SECTION 'B-B'

NOTES

1. REINFORCING STEEL TO HAVE MINIMUM 40mm COVER.
2. CONCRETE TO BE 25Mpa @ 30 DAY STRENGTH.
3. ENTRAINED AIR SHALL BE MAINTAINED AT 5%.
4. MAXIMUM CONCRETE SLUMP SHALL BE 75mm.
5. MAXIMUM AGGREGATE SIZE SHALL BE 19mm.
6. ALL REINFORCING STEEL TO BE INTERMEDIATE GRADE, ASTM 15 DEFORMED.
7. ALL STEEL TO BE BENT AROUND OPENING.
8. INTERIOR WALLS TO BE PAINTED AND EXTERIOR WALLS TO BE DAMPROOFED WITH ASPHALT PRIMER AND FOUNDATION COATING.
9. FOR MECHANICAL DETAILS SEE DRAWING NO. OI-126.
10. ELEVATIONS DERIVED FROM MONUMENT ON SOUTH-EAST CORNER OF D.L. 572, ELEV. 392.560m.

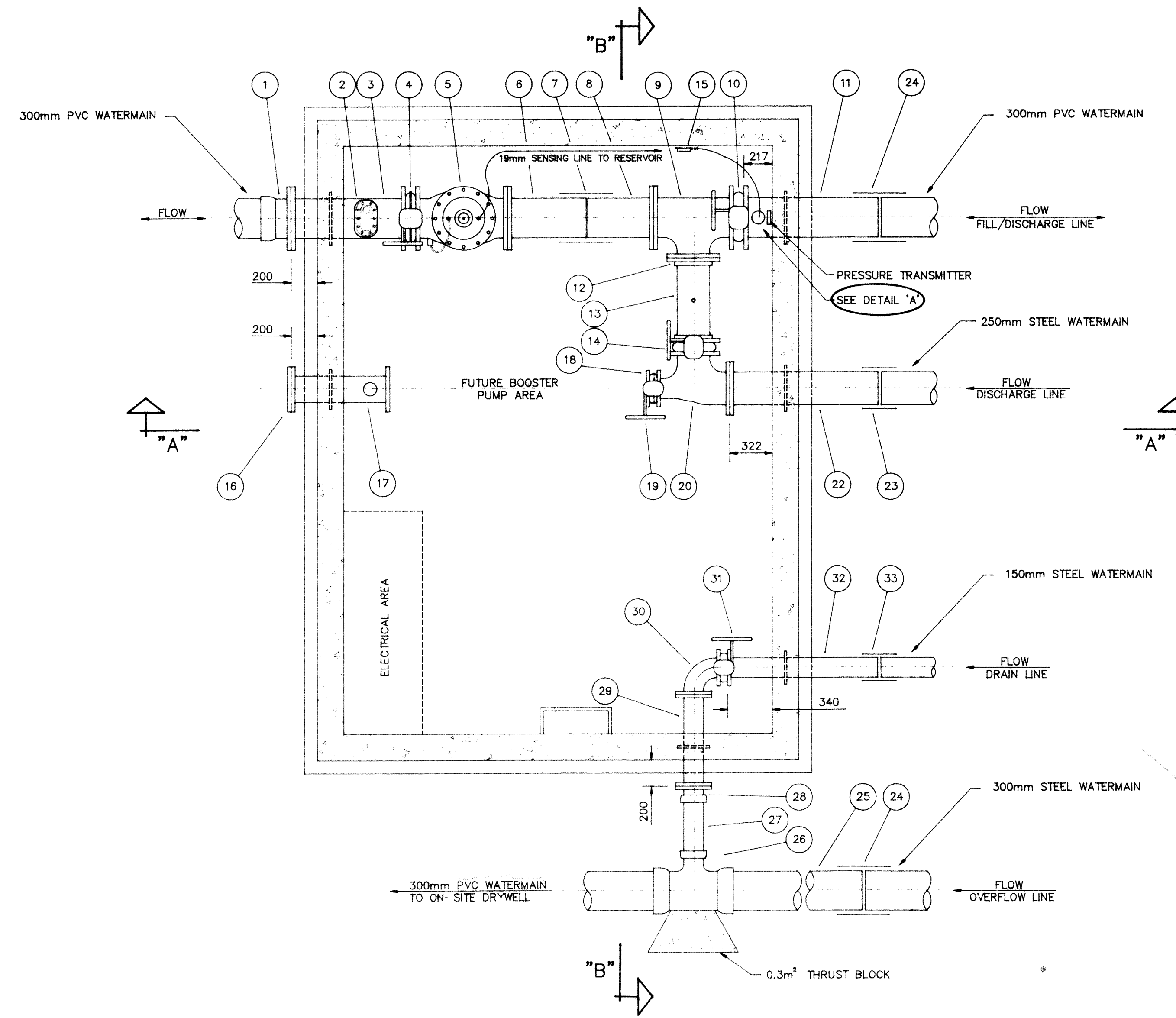
NO.	DATE	BY	REVISION



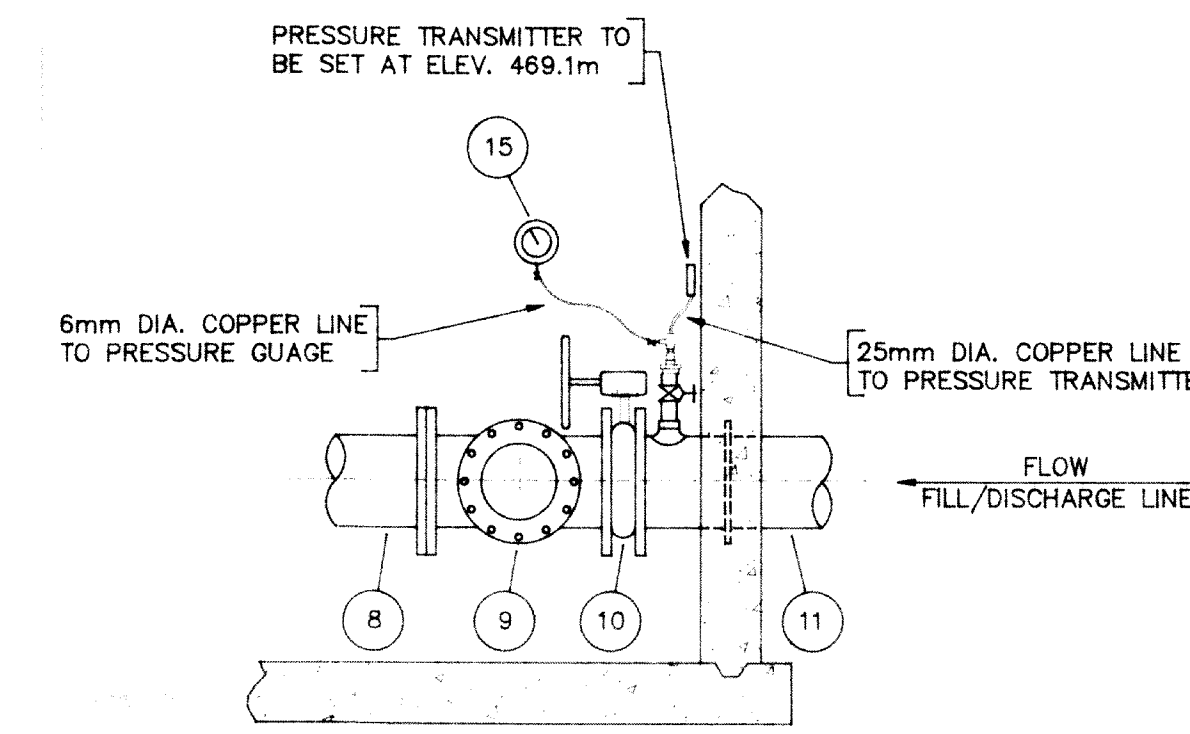
DRAWN	C.C.
DESIGN	J.G.
APPROVED	S.M.
DATE	FEB/1996
SCALE	1:25

OYAMA SERVICE AREA
**OYAMA RESERVOIR
 VALVE CHAMBER
 STRUCTURAL DETAILS**

DRAWING NO.	REV. NO.
OID-125	

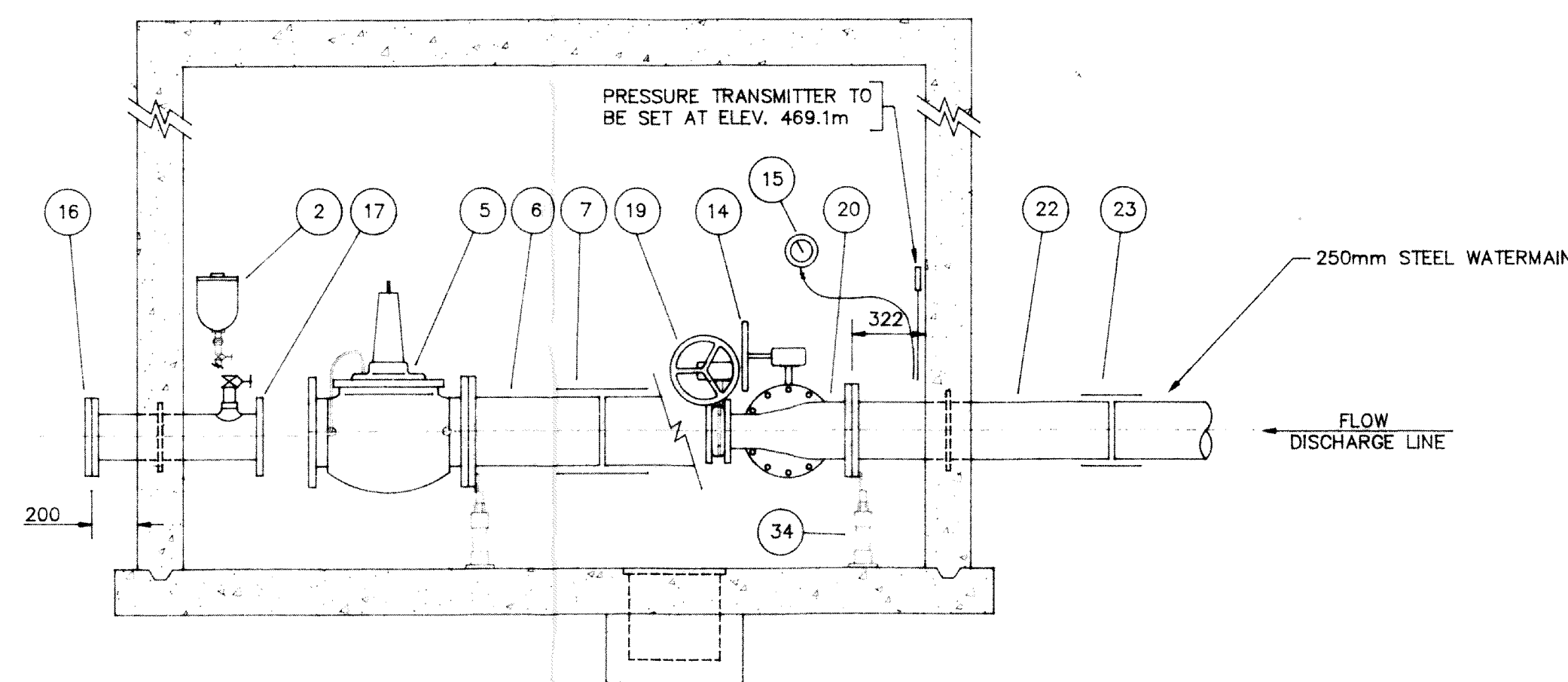


FOR RESERVOIR MECHANICAL
DETAILS SEE DWG. NO. OI-120A

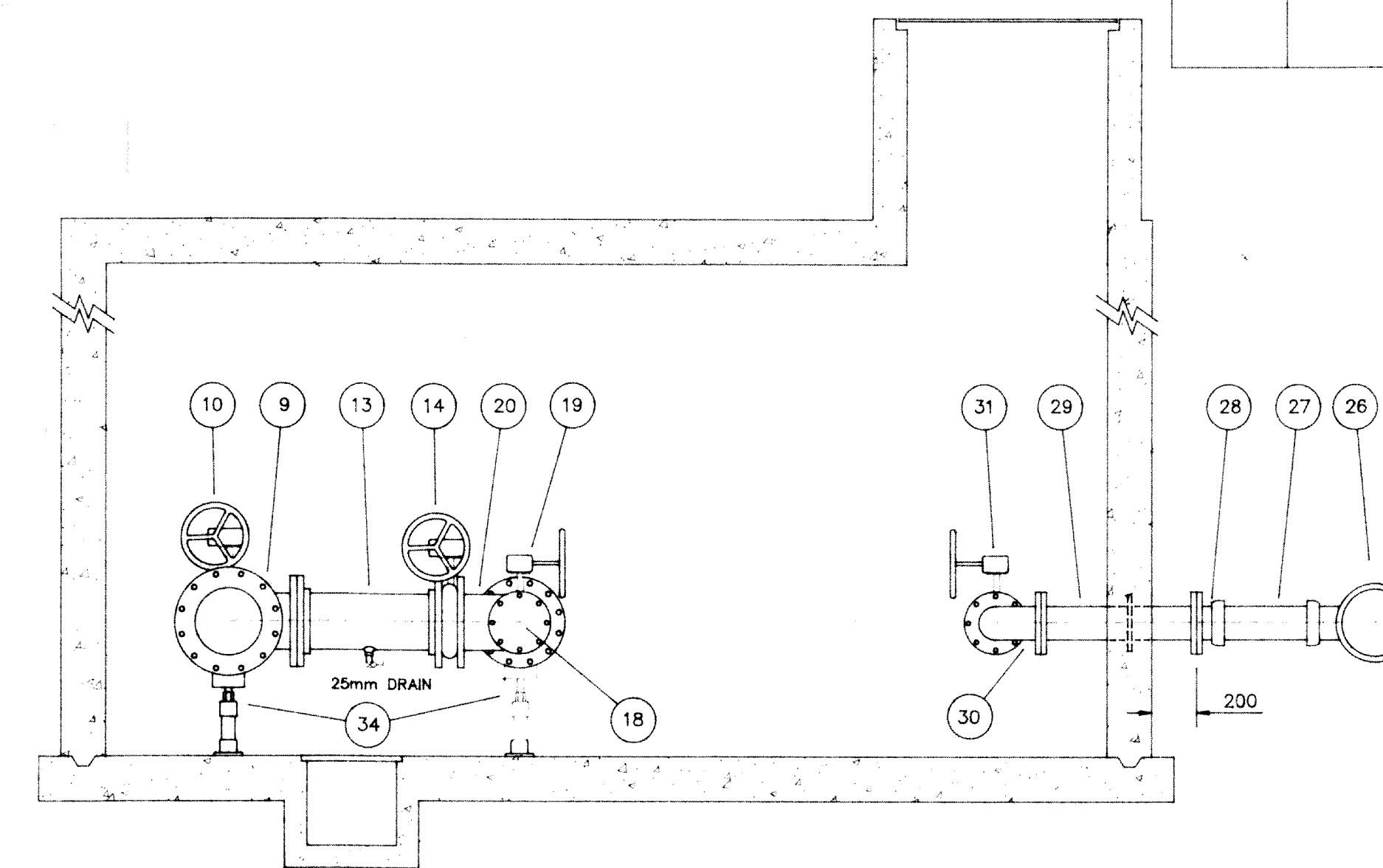


DETAIL 'A'

PLAN



SECTION 'A-A'



SECTION 'B-B'

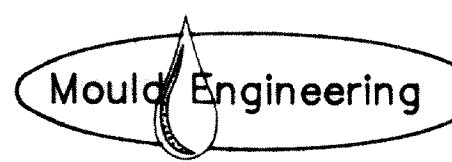
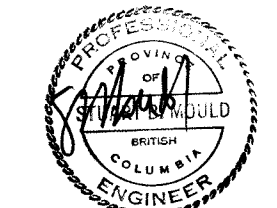
SCHEDULE OF FITTINGS

ITEM NO.	NO. REQ'D	DESCRIPTION
1	1	300H x F ADAPTER
2	1	25mm COMBINATION AIR AND VACUUM VALVE (APCO 143C OR EQUIVALENT)
3	1	300F x F SPOOL PIECE, 870mm LONG, c/w 350mm DIA. x 13mm THICK WALL RING LOCATED 570mm FROM INSIDE FLANGE, AND A 25mm IPT OUTLET LOCATED 300mm FROM INSIDE FLANGE.
4	1	300mm LUG STYLE BUTTERFLY VALVE, c/w HAND WHEEL ACTUATOR
5	1	300F x F MODULATING FLOAT VALVE (SINGER MODEL 106F, TYPE IV OR EQUIVALENT)
6	1	300F x PE SPOOL PIECE, 600mm LONG, CUT TO SUIT IN FIELD
7	1	300mm STEEL PIPE COUPLING
8	1	300F x PE SPOOL PIECE, 500mm LONG
9	1	300F x 300F x 250F TEE
10	1	300mm WAFER STYLE BUTTERFLY VALVE, c/w HAND WHEEL ACTUATOR
11	1	300F x PE SPOOL PIECE, 1020mm LONG, c/w 350mm DIA. x 13mm THICK WALL RING LOCATED 320mm FROM FLANGE, AND A 50mm IPT OUTLET LOCATED 100mm FROM FLANGE
12	2	250mm UNI-FLANGE ADAPTER, c/w SET SCREWS
13	1	250PE x PE SPOOL PIECE, 670mm LONG, c/w 25mm IPT OUTLET, CUT TO SUIT IN FIELD.
14	1	250mm WAFER STYLE BUTTERFLY VALVE, c/w HAND WHEEL ACTUATOR
15	1	PRESSURE GAUGE, 115mm FACE, 0-30 PSI GRADUATION, 1% ACCURACY
16	1	200mm BLIND FLANGE
17	1	200F x F SPOOL PIECE, 750mm LONG c/w 250mm DIA. x 13mm THICK WALL RING LOCATED 450mm FROM INSIDE FLANGE, AND A 50mm IPT OUTLET LOCATED 150mm FROM INSIDE FLANGE.
18	1	150mm BLIND FLANGE
19	1	150mm LUG STYLE BUTTERFLY VALVE, c/w HAND WHEEL ACTUATOR
20	1	250F x 150F x 250F TEE
22	1	250F x PE SPOOL PIECE, 1120mm LONG c/w 300mm DIA. x 13mm THICK WALL RING LOCATED 420mm FROM FLANGE, CUT TO SUIT IN FIELD.
23	1	250mm STEEL PIPE COUPLING
24	2	300mm ADAPTER COUPLING (END CODE: 300mm D4 TO 300mm B7)
25	1	300mm PVC SPOOL PIECE, 3900mm LONG, CUT TO SUIT IN FIELD.
26	1	300H x 300H x 150H TEE
27	1	150mm PVC SPOOL PIECE, 600mm LONG, CUT TO SUIT IN FIELD.
28	1	150H x F ADAPTER
29	1	150F x F SPOOL PIECE, 695mm LONG, c/w 200mm DIA. x 13mm THICK WALL RING LOCATED 400mm FROM INSIDE FLANGE
30	1	150F x F 90° BEND
31	1	150mm WAFER STYLE BUTTERFLY VALVE, c/w HAND WHEEL ACTUATOR
32	1	150F x PE SPOOL PIECE, 1140mm LONG, c/w 200mm DIA. x 13mm THICK WALL RING LOCATED 440mm FROM FLANGE, CUT TO SUIT IN FIELD.
33	1	150mm STEEL PIPE COUPLING
34	2	ADJUSTABLE FLANGE PIPE SUPPORT

NOTES:

- THE FOLLOWING NOTATIONS HAVE BEEN USED IN THE SCHEDULE OF FITTINGS:
 - F - CLASS D FLANGE, AWWA STD C-207.
 - PE - PLAIN END.
 - DI - DUCTILE IRON.
 - H - PUSH ON RUBBER GASKET BELL JOINT AS DESCRIBED IN AWWA STD. C-111.
- ALL STEEL TO BE COATED INSIDE AND OUT WITH EPOXY OR MARINE ENAMEL ACCORDING TO AWWA STD. C-210.

NO.	DATE	BY	REVISION

DRAWN C.C.
 DESIGN J.G.
 APPROVED S.M.
 DATE FEB/1996
 SCALE 1:25

CH'KD 208 - 437 Glenmore Road, Kelowna, B.C. V1Y 1Y5 Telephone 868-2072

OYAMA SERVICE AREA		DRAWING NO.	REV. NO.
OYAMA RESERVOIR VALVE CHAMBER MECHANICAL DETAILS		OID-126	

REPORT

Kalamalka Lake
Water Distribution
System – Upper
Pressure Zone
Review

1577.0046.01
January 2013



Appendix 6-1 Cost Estimates

**Irvine Booster Station
Cost Estimate**

Job No. 1577.0046.01
Date 13/01/2013

Prepared by: J.Clowes
Checked by: D.Nash

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
1	General				
1.1	Mobilization and Demobilization	1	ls	20,000	\$20,000
2	Removals				
2.1	150 m3 reservoir	1	ls	30,000	\$30,000
2.2	Exisiting B1 pump station	1	ls	20,000	\$20,000
2.3	Decomission B2 pump station and discharge line	1	ls	20,000	\$20,000
3	Site Works				
3.1	Access road	1	ls	20,000	\$20,000
3.2	Retaining Wall	85	v.sq m	300	\$25,500
3.2	Tie to existing watermain	2	ls	6,000	\$12,000
3.3	300mm Pipe - Suction	20	lm	215	\$4,300
3.4	250mm Pipe - Discharge	15	lm	200	\$3,000
3.5	250mm buried gate valve	1	ea.	4,000	\$4,000
3.6	300mm buried gate valve	1	ea.	6,000	\$6,000
3.9	Chain Link fence	100	lm	50	\$5,000
3.10	Misc site works and landscaping	1	ls	10,000	\$10,000
4	Pump Station				
4.1	Building	106	sq.m	1,800	\$190,800
4.2	40HP Booster Pump	2	ls	30,000	\$60,000
4.3	3HP Jockey Pump	2	ls	4,500	\$9,000
4.4	150mm pressure relief valve	1	ea.	6,750	\$6,750
4.5	75mm butterfly valve - 150#	2	ea.	500	\$1,000
4.6	150mm butterfly valve - 150#	2	ea.	1,300	\$2,600
4.7	200mm butterfly valve - 150#	2	ea.	1,700	\$3,400
4.8	300mm butterfly valve - 150#	2	ea.	2,500	\$5,000
4.9	200mm check valve - 150#	2	ea.	2,500	\$5,000
4.10	150mm check valve - 150#	1	ea.	2,000	\$2,000
4.11	300mm strainer	2	ea.	4,500	\$9,000
4.12	75mm strainer	1	ea.	500	\$500
4.13	50mm check valve	2	ea.	500	\$1,000
4.14	50mm ball valve	2	ea.	500	\$1,000
4.15	25mm combination air release and air/vac	5	ea.	800	\$4,000
4.16	50mm schedule 10 SS	5	lm	90	\$450
4.17	75mm schedule 10 SS	10	lm	100	\$1,000
4.18	150mm Schedule 10 SS	10	lm	900	\$9,000
4.19	200mm Schedule 10 SS	5	lm	1,200	\$6,000
4.20	250mm Schedule 10 SS	10	lm	1,300	\$13,000
4.21	300mm Schedule 10 SS	20	lm	1,500	\$30,000
4.22	Pressure gauge	6	ea.	500	\$3,000
4.23	Electrical and Instrumentation	1	ls	135,000	\$135,000
4.24	Electrical service	1	ls	30,000	\$30,000
5	Distribution System Upgrades				
5.1	200mm PVC C900 watermain	943	lm	180	\$169,740
5.2	Asphalt road restoration	900	sq.m	50	\$45,000
5.3	Residential service connections	23	ea.	1,500	\$34,500
5.4	Irrigation service connections	8	ea.	2,000	\$16,000
5.5	Hydrant connections	3	ea.	1,500	\$4,500
5.6	Tie to existing watermain	1	ea.	6,000	\$6,000
5.7	Replace air/vacuum valve	6	ea.	1,000	\$6,000
5.8	Add air/vacuum valve	4	ea.	6,000	\$24,000
5.9	Misc (restoration, driveway crossings)	1	ls	50,000	\$50,000
				Subtotal	\$1,064,040
				Contingency and Engineering (35%)	\$372,414
				Total (rounded)	\$1,435,000

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
6	Optional				
6.1	Private Hydrant	2	ea.	4,000	\$8,000
6.2	200 mm buried gate valve	4	ea.	3,500	\$14,000
6.3	Blow-off	2	ea.	3,000	\$6,000
				Subtotal	\$28,000
				Contingency (20%)	\$5,600
				Total (rounded)	\$35,000

Appendix D

Seaton Road Watermain Extension (Urban Systems)

MEMORANDUM



Date: September 12, 2014
To: Janice Pederson, Michael Mercer, District of Lake Country
cc: Mike Mitchell, District of Lake Country
From: Jason Barta
File: 1577.0022.01-C
Subject: Seaton Road – Watermain Extension Analysis for #10011 Seaton Road

Background

The District of Lake Country (District) requested that Urban Systems Ltd. (Urban) investigate water servicing alternatives to provide irrigation water to #10011 Seaton Road. There are no water mains along either roadway frontage for the property, although a domestic service for the property exists along the southern frontage.

The site could be supplied with irrigation water via Seaton Road from either the south or the north. A loop connecting the existing north and south watermains would require a brand new pressure reducing station (PRV), since the watermain to the north is supplied by the Camp & Bond PRV and the watermain to the south is supplied by the Seaton PRV. The existing watermains to the north and south of the site are both currently supplied by the Beaver Lake water source.

Design Criteria

The District has confirmed that the subject parcel is classified (or will be classified) as Class “A” land, is approximately 10 acres in size and will be restricted to a peak irrigation flow of 6 gallons per minute per acre, for a total water demand of 60 gpm, or 228 Litres per minute.

The following design criteria were taken from the District’s Subdivision and Development Servicing Bylaw, No. 97-139 (bylaw):

- Minimum watermain diameter shall be 200mm;
- Maximum static pressure of 145 psi (1,000 kPa);
- Minimum static pressure of 40 psi (275 kPa);
- Minimum residual pressure at peak hour of 36 psi (250 kPa); and
- All fire flow analyses will be performed while adhering to the following constraints:
 - System demands will reflect MDD plus fire flow rate;
 - Minimum residual pressure during fire flow shall be 20 psi (140kPa); and
 - Watermain velocities shall not exceed 4 meters per second under fire flow conditions

Assumptions

The subject parcels and surrounding lands are all zoned A-1, agricultural. For the purposes of this analysis, it is assumed that the minimum required fire flow for the area will be 3,600 L/min.

Cost estimates do not include any land purchases, nor legal fees associated with such.

MEMORANDUM

Date: September 12, 2014
File: 1577.0022.01-C
Subject: Seaton Road – Watermain Extension Analysis for #10011 Seaton Road
Page: 2 of 3



Analysis

North System Analysis

The existing water system north of the site is fed via the Camp and Bond PRV station. The largest PRV in the station is 200mm in diameter and can pass a maximum flow of 3,100 USgpm (11,700 L/min) based on literature from the manufacturer.

The watermains along Seaton Road are predominantly 200mm in diameter, however, there are some sections of 150mm and 100mm mains. A review of the existing conditions water model shows that these smaller mains restrict the available fire flow to approximately 1,800 L/min. This is not sufficient to meet the bylaw minimum of 3,600 L/min, so any mains along Seaton Road to the north of the subject property must be upgraded to a minimum diameter of 200mm.

The hydraulic water model was modified to upsize the Seaton Road watermains to 200mm in diameter, and a new demand of 228 L/min was added to the proposed watermain extension to the subject property. With these new upgrades in place, the model estimates that the pressure under PHD conditions will be 130 psi at an elevation of 487m and the available fire flow will increase to 7,000 L/min. **Figure 1** illustrates the required upgrades along Seaton Road associated with the north watermain extension.

South system analysis

The existing water system south of the site is fed via the Seaton PRV station. The largest PRV in the station is 100mm in diameter and can pass a maximum flow of 800 USgpm (3,000 L/min) based on literature from the manufacturer.

There is a long section of 100mm diameter watermain to the south of the site, along Seaton Road. This watermain, as well as portions of the 200mm further south, are located on private property. The model estimates that there is 4,200 L/min of available fire flow at the intersection of Seaton and Wilson Roads due to the loop along Wilson Road. However,

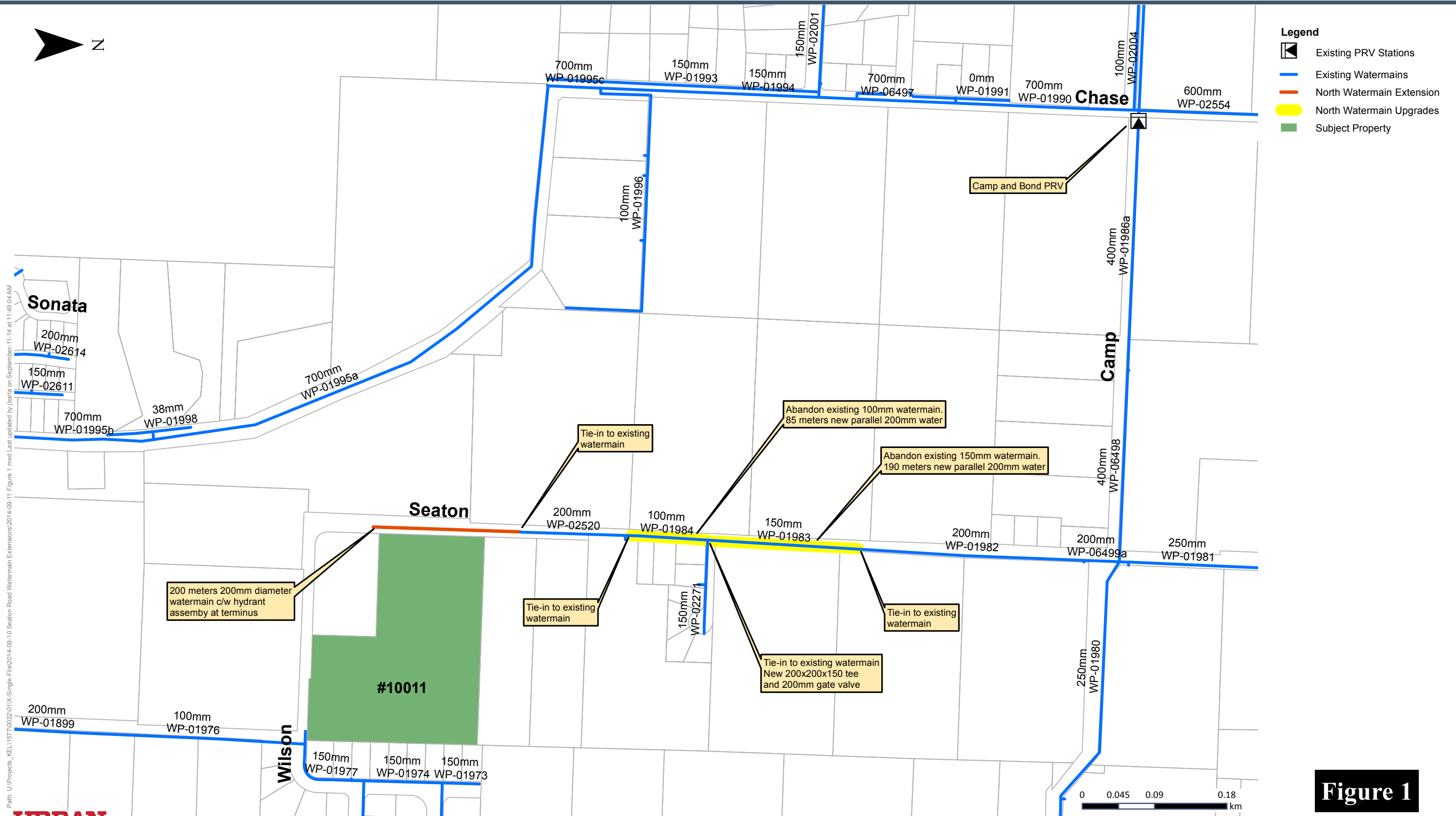
There is also a 150mm watermain along Wilson Road that provides a loop to the intersection near the subject property. The available fire flow in the existing system at the intersection of Seaton and Wilson Road is estimated at 4,200 L/min by the model, due to the loop with Wilson Road.

However, if a 3,600 L/min fire event were to happen near the subject site, the majority of the flow would be pulled via Wilson Road watermain, which would reduce pressures in the area from nearly 70 psi to below 30 psi. Thus, an upgrade to the Seaton PRV and the undersized watermains will be required, and is illustrated on **Figure 2**.

With these new upgrades in place, the model estimates that the pressure under PHD conditions will be 67 psi at an elevation of 487m and the available fire flow will increase to 7,300 L/min.

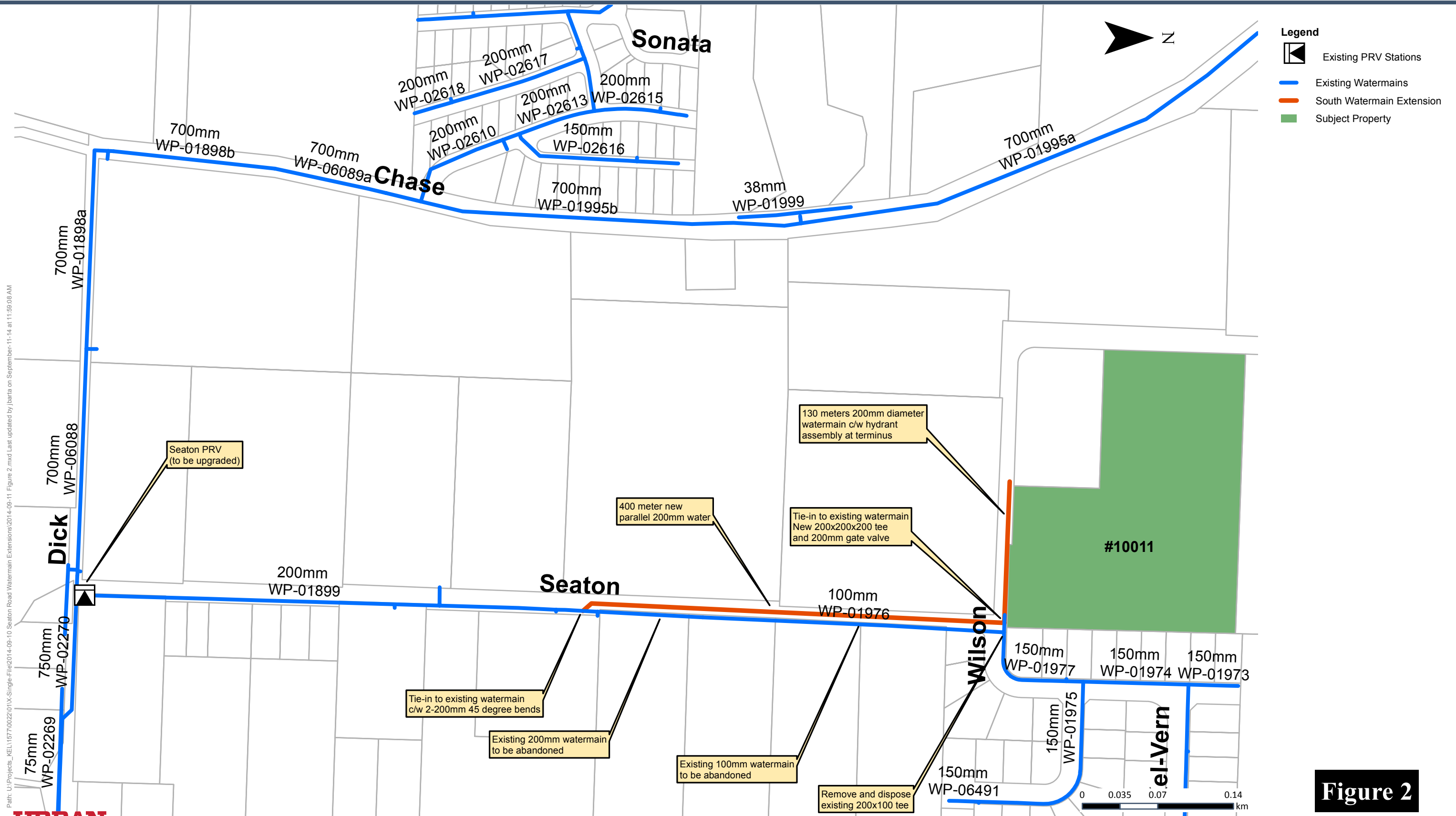
Cost Estimate

The estimates provided in **Table 1** this section are classified as a Class 'C' cost estimates. This estimate, which is prepared with limited site information, is based on probable conditions affecting the project. It



Path: U:\Projects_KELLY157\0022\01X-Single-File\2014-09-10 Seaton Road Watermain Extensions\2014-09-11 Figure 1.mxd Last updated by jbarra on September-11-14 at 11:49:04 AM

Figure 1



Path: U:\Projects_KELLY\70022201X-Single-File\2014-09-10 Seaton Road Watermain Extensions\2014-09-11 Figure 2.mxd Last updated by jbarba on September-11-14 at 11:59:08 AM

Figure 2

MEMORANDUM

Date: September 12, 2014
File: 1577.0022.01-C
Subject: Seaton Road – Watermain Extension Analysis for #10011 Seaton Road
Page: 3 of 3



represents the summation of all identifiable project component costs. A contingency allowance of 25% plus engineering and other allowances is appropriate for this class of estimate. A detailed breakdown is attached to this memorandum.

Table 1 – Cost Estimate

	North Watermain Extension	South Watermain Extension
Removals	\$19,000	\$22,000
Watermain upgrades	\$153,000	\$351,000
Roadway Restoration (*)	\$69,000	\$77,000
Subtotal	\$241,000	\$427,000
Contingency Allowance (40%)	\$97,000	\$171,000
Total	\$338,000	\$598,000

(*) assumes half road width restoration

Summary

Servicing the property at 10011 Seaton Road by extending and upgrading the existing watermain north of the site is the most cost-effective strategy. Both cost estimates are close in value with the exception of the PRV facility required in the south extension option. If the District has additional discretionary funds available, choosing the south watermain extension project will improve operations by providing an above-ground PRV building as well as addressing the existing watermain trespass issue on private property.

Thank you for the opportunity to assist the District with this water system analysis. Should you have any questions, please contact the undersigned.

URBAN SYSTEMS LTD.

Reviewed by:

Jason Barta, B.Sc.
Municipal Infrastructure Analyst
/jb

Randy Runzer, ASCT
Principal, Senior Technologist

10011 Seaton Road - Watermain Extensions

ITEM	DESCRIPTION	UNIT	QUANTITY	UNIT PRICE	AMOUNT
North Watermain Extension					
1.0	Remove & dispose existing asphalt	sq.m	1900	\$6.00	\$ 11,400.00
2.0	Sawcut existing asphalt	m	950	\$8.00	\$ 7,600.00
3.0	200mm DR18 PVC watermain	m	475	\$260.00	\$ 123,500.00
4.0	200x200x150mm tee	each	1	\$1,300.00	\$ 1,300.00
5.0	200mm gate valve	each	1	\$1,700.00	\$ 1,700.00
6.0	Hydrant assembly	each	1	\$5,200.00	\$ 5,200.00
7.0	Tie-in to existing watermain	each	4	\$1,800.00	\$ 7,200.00
8.0	Controlled density fill	m	275	\$50.00	\$ 13,750.00
9.0	Roadway restoration - 300mm depth sub base gravel course	sq. m	1900	\$15.00	\$ 28,500.00
10.0	Roadway restoration - 100mm depth base gravel course	sq. m	1900	\$6.00	\$ 11,400.00
11.0	Roadway restoration - 50mm depth asphaltic concrete pavement	sq. m	1900	\$15.00	\$ 28,500.00
Subtotal (North watermain extension)					\$ 241,000.00
Engineering and contingency (15%+25%)					\$ 97,000.00
Total (North watermain extension)					\$ 338,000.00
South Watermain Extension					
1.0	Remove & dispose existing asphalt	sq.m	2120	\$6.00	\$ 12,720.00
2.0	Sawcut existing asphalt	m	1060	\$8.00	\$ 8,480.00
3.0	200mm DR18 PVC watermain	m	530	\$260.00	\$ 137,800.00
4.0	200x200x200mm tee	each	1	\$1,300.00	\$ 1,300.00
5.0	200mm gate valve	each	1	\$1,700.00	\$ 1,700.00
6.0	200mm 45 degree bend	each	2	\$400.00	\$ 800.00
7.0	Hydrant assembly	each	1	\$5,200.00	\$ 5,200.00
8.0	Above ground PRV building (building, power, heat, lights, 150mm and 50mm pressure valves, SCADA with radio, parking, fencing)	LS	1	\$180,000.00	\$ 180,000.00
9.0	Tie-in to existing watermain	each	2	\$1,800.00	\$ 3,600.00
10.0	Controlled density fill	m	400	\$50.00	\$ 20,000.00
11.0	Roadway restoration - 300mm depth sub base gravel course	sq. m	2120	\$15.00	\$ 31,800.00
12.0	Roadway restoration - 100mm depth base gravel course	sq. m	2120	\$6.00	\$ 12,720.00
13.0	Roadway restoration - 50mm depth asphaltic concrete pavement	sq. m	2120	\$15.00	\$ 31,800.00
Subtotal (South watermain extension)					\$ 427,000.00
Engineering and contingency (15%+25%)					\$ 171,000.00
Total (South watermain extension)					\$ 598,000.00

Appendix E

Moberly Road Watermain Extension (Urban Systems)

MEMORANDUM



Date: October 23, 2015
To: Sid Smith, Michael Mercer; District of Lake Country
cc:
From: Jason Barta
File: 1577.0063.01
Subject: Moberly Road Watermain Extension

Background

Urban Systems met with the District of Lake Country and Mr. Rob Richardson on October 9th, 2015 at the District offices to discuss extending the District's water system to service up to twenty (20) homes that are currently receiving water from a private water utility. The parcels to be serviced are located between Moberly Road and Carrs Landing Road.

This memo investigates the available pressure and fire flow in the District's water system and the upgrades required to provide adequate pressure and fire flow to the twenty properties.

Urban Systems is currently working on the District project "Small Diameter Watermain Analysis" in which upstream upgrades have already been identified to provide sufficient fire flow to the Carrs Landing and Pixie Beach areas.

Methodology

We will add the anticipated water use for the twenty properties to the existing conditions water model under both peak hour and maximum day demand conditions to determine the effect on system pressures and available fire flow.

The following design criteria were taken from the District's Subdivision and Development Servicing Bylaw, No. 97-139 (bylaw):

- Residential maximum day demand (MDD) is 3,000 L/capita/day;
- Minimum pressure under peak hour demands (PHD) shall not be less than 36 psi (250 kPa);
- All fire flow analyses will be performed while adhering to the following constraints:
 - System demands will reflect MDD plus fire flow rate;
 - Minimum residual pressure during fire flow shall be 20 psi (140kPa); and
 - Watermain velocities shall not exceed 4 meters per second under fire flow conditions

Based on twenty parcels and a density of two persons per household, the anticipated MDD is 1.39 L/s. The demands were added to the north end of a proposed watermain along Moberly Road (currently a private driveway) that parallels Carrs Landing Road. At the request of the District, the system was examined under a scenario where the proposed main was 150mm in diameter and a second scenario where the proposed main was 200mm in diameter.

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Results

The proposed watermain water main was modeled as both a 150mm and 200mm diameter watermain extension in the model, with demands assigned to a node at the north end of the main, at an approximate elevation of 400 meters.

A new 250mm diameter watermain must be installed from the intersection of Moberly and Carrs Landing Roads, along Moberly Road, to the private easement.

The proposed watermain servicing the twenty properties through the private driveway easement, as well as the upstream upgrades identified in the Small Diameter Watermain Analysis project are shown in Figure 1.

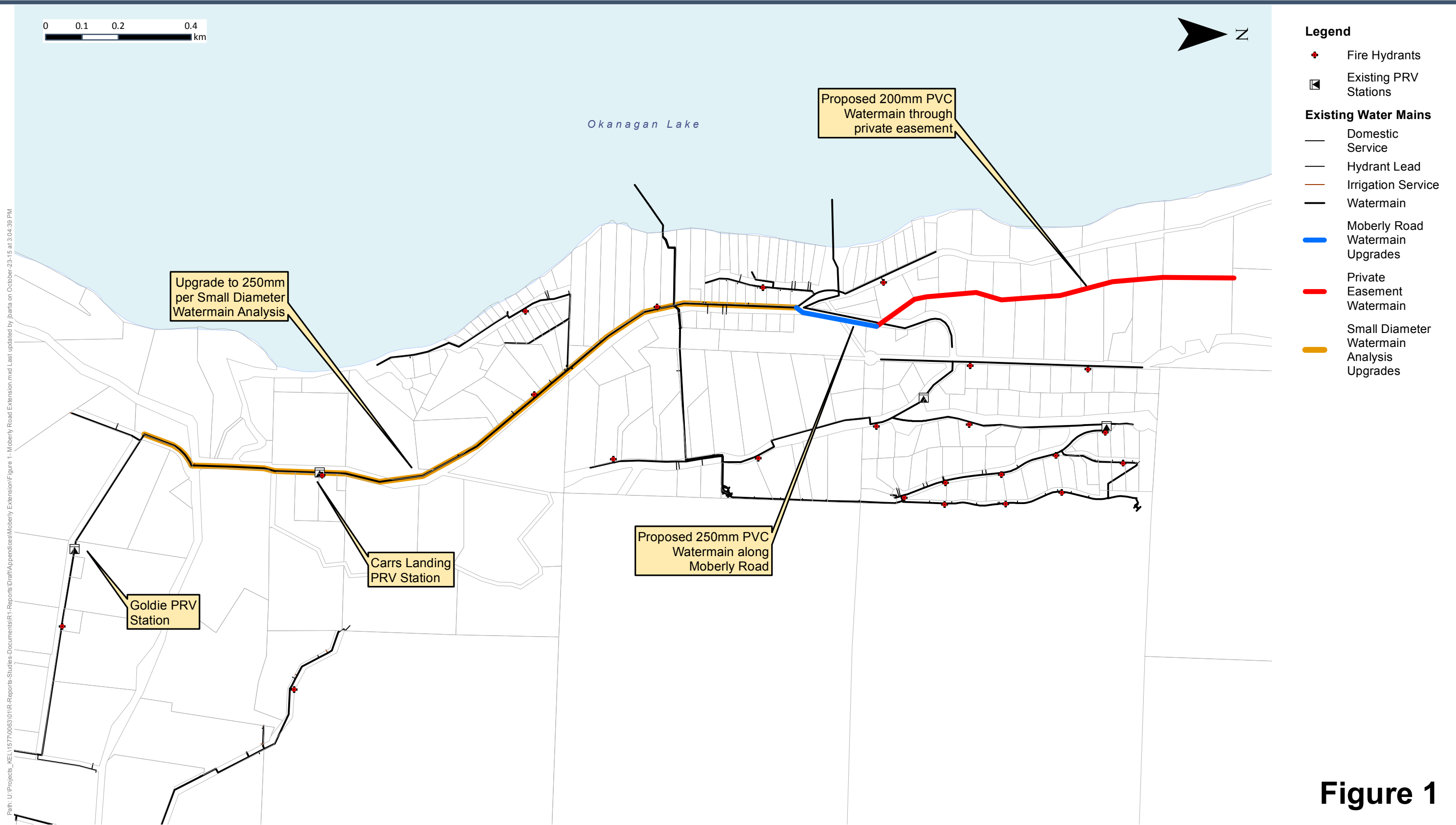
Table 1 below lists the available fire flow based on the anticipated diameter of the new main serving the twenty properties.

Table 1 – Available Fire Flow

Diameter of Proposed Main	150mm	200mm
Available Fire Flow	32 L/s	58 L/s

As shown in Table 1, the available fire flow does not meet the District minimum of 60 L/s. The proposed watermain through the private easement (approximate length 1,040 meters) could be upsized to 250mm in diameter (Option #1) to provide 73 L/s of fire flow, but may cause water quality issues due to the low demands drawing from a large main.

Option #2 would be to oversize some of the upstream upgrades that are identified in the Small Diameter Watermain analysis. Specifically, the proposed 250mm diameter watermain from the Goldie PRV station to the tee servicing the Pixton Road area could be oversized to 300mm in diameter. The estimated length of this oversizing is 1,660 meters. This option is shown in Figure 2. Even with the oversizing to 300mm diameter, the proposed watermain through the private easement still needs to be 200mm in diameter to provide 63 L/s of fire flow.

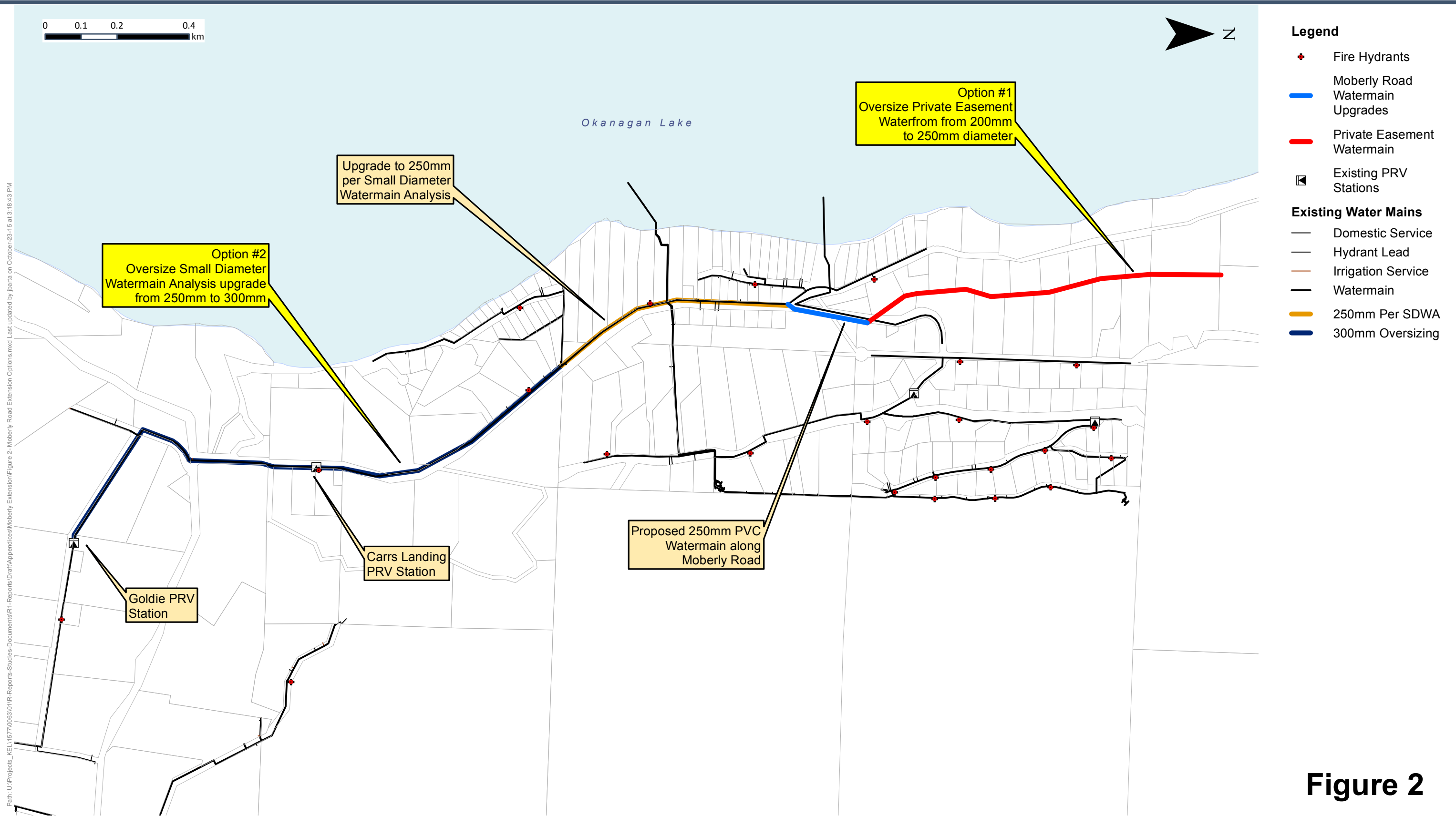


- Legend**
- ✚ Fire Hydrants
 - ☐ Existing PRV Stations
 - Existing Water Mains**
 - Domestic Service
 - Hydrant Lead
 - Irrigation Service
 - Watermain
 - Moberly Road Watermain Upgrades
 - Private Easement Watermain
 - Small Diameter Watermain Analysis Upgrades

Path: U:\Projects_KELY\1577\0063\01R-Reports-S\Utilities-Documents\1577-0063-01R-Reports-S\Utilities-Documents\Moberly Extension\Figure 1-Moberly Road Extension.mxd Last updated by jbaria on October-23-15 at 3:04:39 PM

Figure 1

Moberly Road Watermain Extension Options



Path: U:\Projects_KELY\1577\0063\01R-Reports-S\udis-Documents\1-Reports\Draft\Appendices\Moberly Extension Options\Figure 2-Moberly Road Extension Options.mxd Last updated by jbarra on October-23-15 at 3:18:43 PM

Figure 2

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Recommendation

Given the relatively small price difference in pipe costs for over-sizing, and to avoid water quality issues, it is suggested that Option 2 be selected. The oversizing from the Goldie PRV to the Pixton Road watermain tee fitting provides additional fire flow capacity to the entire Carrs Landing area.

The design for the watermain extension along Moberly Road and through the private easement should be reviewed by the District to confirm compliance with BCBC 2012, with respect to hydrant spacing. The District should also request and review details fire flow calculations for each property to ensure the required fire flow does not exceed 63 L/s.

Closure

Should you have any questions or comments, please contact the undersigned.

URBAN SYSTEMS LTD.

A handwritten signature in blue ink, appearing to read "JBarta".

Jason Barta, B.Sc.
Municipal Infrastructure Analyst
/jb

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Moberly Road Watermain Extension.docx

Appendix F

Pre-Design Cost Estimates

**Pre-Design Cost Estimate
Okanagan Centre - PRV Replacement - 6th Street**

Project Description		Project #	1.1
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 6", 2"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	486	Checked by:	J. Clowes
Existing HGL setting on downstream side of PRV (m)	427	See Figure	1-1
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	130		
Estimated existing maximum day demand (L/min)	841		
Estimated existing peak hour demand (L/min)	1,244		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
	subtotal				\$239,500
	Contingency and Engineering (35%)				\$83,825
	total				\$323,325
	rounded total				\$320,000

Note: Estimates do not include land acquisition costs

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Okanagan Centre - Watermain Upgrades - Option #1**

Project Description	Project # 1.2
Upgrade Camp Road watermain to 300mm	USL Job No. 1577.0063.01
Upgrade Hare Road watermain to 300mm/200mm	Date 06/27/2016
Upgrade Withers Road watermain to 300mm	Prepared by: J.Barta
Upgrade Stokepoges Road watermain to 200mm	Checked by: J. Clowes
Upgrade Maddock Avenue watermain to 200mm	See Figure 1-1
Add new 250mm watermain along Okanagan Centre Road West	
Abandon existing watermain from 5th street to Camp Road	
Reconnect existing water services to new mains	
Replace existing hydrant assembly and lead (9)	
Add new hydrants to ensure maximum spacing of 180m	
New mainline valves per Figure 1-1	
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$95,000	\$95,000
	Insurance and Bonding	1	LS	\$30,000	\$30,000
	75mm asphalt restoration	14940	sq.m	\$20	\$298,800
	100mm base course gravel restoration	14940	sq.m	\$6	\$89,640
	350mm subbase course gravel restoration	14940	sq.m	\$15	\$224,100
	Rock removal allowance	1500	m	\$150	\$225,000
	Utility conflict allowance	1	ls	\$30,000	\$30,000
	Reconnect existing service to new watermain	70	ea	\$1,500	\$105,000
	Replace existing hydrant assembly and lead	9	ea	\$6,500	\$58,500
	New 150mm hydrant assembly	15	ea	\$5,500	\$82,500
	Tie-in to existing 100mm watermain	4	ea	\$5,000	\$20,000
	Tie-in to existing 150mm watermain	4	ea	\$5,000	\$20,000
	Tie-in to existing 200mm watermain	2	ea.	\$5,000	\$10,000
	Tie-in to existing 250mm watermain	2	ea	\$5,000	\$10,000
	200mm PVC watermain	1220	m	\$250	\$305,000
	200mm Gate valve	8	ea	\$2,000	\$16,000
	250mm PVC watermain	1360	m	\$275	\$374,000
	250mm Gate valve	10	ea	\$3,000	\$30,000
	300mm PVC watermain	2400	ea	\$325	\$780,000
	300mm Gate valve	7	ea	\$4,000	\$28,000
	Remove and dispose existing 150mm watermain	230	lm	\$50	\$11,500
	subtotal				\$2,843,040
	Contingency and Engineering (40%)				\$1,137,216
	total				\$3,980,256
	rounded total				\$3,980,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Okanagan Centre - Watermain Upgrades - Option #2**

Project Description	Project # 1.3
New PRV station and watermain between Camp and Hare Roads (greenfield)	USL Job No. 1577.0063.01
Upgrade Hare Road watermain to 250mm	Date 06/27/2016
Upgrade Withers Road watermain to 300mm	Prepared by: J.Barta
Upgrade Stokepoges Road watermain to 200mm	Checked by: J. Clowes
Upgrade Maddock Avenue watermain to 200mm	See Figure 1-2
Add new 250mm watermain along Okanagan Centre Road West	
Abandon existing watermain from 5th street to Camp Road	
Reconnect existing water services to new mains	
Replace existing hydrant assembly and lead (9)	
Add new hydrants to ensure maximum spacing of 180m	
New mainline valves per Figure 1-2	
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$95,000	\$95,000
	Insurance and Bonding	1	LS	\$30,000	\$30,000
	75mm asphalt restoration	12990	sq.m	\$20	\$259,800
	100mm base course gravel restoration	12990	sq.m	\$6	\$77,940
	350mm subbase course gravel restoration	12990	sq.m	\$15	\$194,850
	Rock removal allowance	1300	m	\$150	\$195,000
	Utility conflict allowance	1	ls	\$30,000	\$30,000
	Reconnect existing service to new watermain	65	ea	\$1,500	\$97,500
	Replace existing hydrant assembly and lead	9	ea	\$6,500	\$58,500
	New 150mm hydrant assembly	7	ea	\$5,500	\$38,500
	Tie-in to existing 100mm watermain	4	ea	\$5,000	\$20,000
	Tie-in to existing 150mm watermain	4	ea	\$5,000	\$20,000
	Tie-in to existing 200mm watermain	2	ea.	\$5,000	\$10,000
	Tie-in to existing 250mm watermain	1	ea	\$5,000	\$5,000
	200mm PVC watermain	860	m	\$250	\$215,000
	200mm Gate valve	6	ea	\$2,000	\$12,000
	250mm PVC watermain	3130	m	\$275	\$860,750
	250mm Gate valve	16	ea	\$3,000	\$48,000
	300mm PVC watermain	340	m	\$325	\$110,500
	300mm Gate valve	1	ea	\$4,000	\$4,000
	New PRV station	1	LS	\$260,000	\$260,000
	subtotal				\$2,642,340
	Contingency and Engineering (40%)				\$1,056,936
	total				\$3,699,276
	rounded total				\$3,700,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Okanagan Centre - Optional AC Watermain Replacement - Camp Road I

Project Description	Project # 1.4
Abandon existing 250mm AC watermain along Camp Road	USL Job No. 1577.0063.01
from Cheeseman Road to Davidson PRV and replace with PVC	Date 06/27/2016
Reconnect existing water services to new mains	Prepared by: J.Barta
New mainline valves at 250m spacing	Checked by: J. Clowes
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	See Figure 1-2

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$25,000	\$25,000
	Insurance and Bonding	1	LS	\$7,000	\$7,000
	75mm asphalt restoration	1800	sq.m	\$20	\$36,000
	100mm base course gravel restoration	1800	sq.m	\$6	\$10,800
	350mm subbase course gravel restoration	1800	sq.m	\$15	\$27,000
	Rock removal allowance	400	m	\$150	\$60,000
	Utility conflict allowance	1	ls	\$10,000	\$10,000
	Reconnect existing service to new watermain	23	ea	\$1,500	\$34,500
	Tie-in to existing 250mm watermain	2	ea	\$5,000	\$10,000
	250mm PVC watermain	900	m	\$275	\$247,500
	250mm Gate valve	4	ea	\$3,000	\$12,000
	subtotal				\$479,800
	Contingency and Engineering (40%)				\$191,920
	total				\$671,720
	rounded total				\$670,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate					
Okanagan Centre - Optional AC Watermain Replacement - Camp Road II					
Project Description			Project #	1.5	
Abandon existing 200/250mm AC watermain along Camp Road from Davidson PRV to Tyndall Road and replace with PVC			USL Job No.	1577.0063.01	
Reconnect existing water services to new mains			Date	06/27/2016	
New 250mm mainline valve at existing hydrant location			Prepared by:	J.Barta	
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration			Checked by:	J. Clowes	
			See Figure	1-2	
ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$18,000	\$18,000
	Insurance and Bonding	1	LS	\$6,000	\$6,000
	75mm asphalt restoration	1530	sq.m	\$20	\$30,600
	100mm base course gravel restoration	1530	sq.m	\$6	\$9,180
	350mm subbase course gravel restoration	1530	sq.m	\$15	\$22,950
	Rock removal allowance	300	m	\$150	\$45,000
	Utility conflict allowance	1	ls	\$10,000	\$10,000
	Reconnect existing service to new watermain	13	ea	\$1,500	\$19,500
	Tie-in to existing 250mm watermain	2	ea	\$5,000	\$10,000
	250mm PVC watermain	765	m	\$275	\$210,375
	250mm Gate valve	3	ea	\$3,000	\$9,000
				subtotal	\$390,605
				Contingency and Engineering (40%)	\$156,242
				total	\$546,847
				rounded total	\$550,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Okanagan Centre - Optional PRV Replacement - Davidson PRV

Project Description		Project #	1.6
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 6", 3"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	605	Checked by:	J. Clowes
Existing HGL setting on downstream side of PRV (m)	533	See Figure	1-2
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	262		
Estimated existing maximum day demand (L/min)	2,181		
Estimated existing peak hour demand (L/min)	2,990		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
				subtotal	\$239,500
				Contingency and Engineering (35%)	\$83,825
				total	\$323,325
				rounded total	\$320,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Okanagan Centre - Optional PRV Replacement - Camp PRV

Project Description		Project #	1.7
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 6", 3"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	529	Checked by:	J. Clowes
Existing HGL setting on downstream side of PRV (m)	488	See Figure	1-2
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	224		
Estimated existing maximum day demand (L/min)	1,721		
Estimated existing peak hour demand (L/min)	2,416		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
				subtotal	\$239,500
				Contingency and Engineering (35%)	\$83,825
				total	\$323,325
				rounded total	\$320,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Okanagan Centre - Optional PRV Replacement - Hare PRV

Project Description		Project #	1.8
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 4", 2"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	487	Checked by:	J. Clowes
Existing HGL setting on downstream side of PRV (m)	437	See Figure	1-2
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	9		
Estimated existing maximum day demand (L/min)	89		
Estimated existing peak hour demand (L/min)	118		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
				subtotal	\$239,500
				Contingency and Engineering (35%)	\$83,825
				total	\$323,325
				rounded total	\$320,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Oyama Isthmus Watermain Upgrades**

Project Description	Project # 2.1
Upgrade Greenhow Road watermain to 200mm (1/2 road restoration)	USL Job No. 1577.0063.01
Upgrade Trask Road watermain to 200mm (1/2 road restoration)	Date 06/27/2016
Upgrade Roberts Road watermain to 200mm and loop to Trask Road	Prepared by: J.Barta
Upgrade Ackerman Road watermain to 200mm and loop to Trask Road	Checked by: J. Clowes
Connect hydrant lead at Young/Greenhow Intersection to Greenhow 200mm main	See Figure 2
Replace 50mm GI main on Harris Lane with PVC main	
Reconnect existing water services to new mains	
Replace existing hydrant assembly and lead (3)	
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$30,000	\$30,000
	Insurance and Bonding	1	LS	\$9,000	\$9,000
	75mm asphalt restoration	2475	sq.m	\$20	\$49,500
	100mm base course gravel restoration	2475	sq.m	\$6	\$14,850
	350mm subbase course gravel restoration	2475	sq.m	\$15	\$37,125
	Rock removal allowance	400	m	\$150	\$60,000
	Utility conflict allowance	1	ls	\$10,000	\$10,000
	Reconnect existing service to new watermain	43	ea	\$1,500	\$64,500
	Replace existing hydrant assembly and lead	4	ea	\$6,500	\$26,000
	New 150mm hydrant assembly	0	ea	\$5,500	\$0
	Tie-in to existing 100mm watermain	2	ea	\$5,000	\$10,000
	Tie-in to existing 150mm watermain	3	ea	\$5,000	\$15,000
	Tie-in to existing 200mm watermain	4	ea.	\$5,000	\$20,000
	Tie-in to existing 250mm watermain	0	ea	\$5,000	\$0
	Tie-in to existing 300mm watermain	0	ea	\$5,000	\$0
	Tie-in to existing 350mm watermain	0	ea	\$5,500	\$0
	Tie-in to existing 400mm watermain	1	ea	\$6,000	\$6,000
	50mm PVC watermain	50	m	\$150	\$7,500
	150mm Gate valve	0	ea	\$1,500	\$0
	200mm PVC watermain	985	m	\$250	\$246,250
	200mm Gate valve	7	ea	\$2,000	\$14,000
	250mm PVC watermain	0	m	\$275	\$0
	250mm Gate valve	0	ea	\$3,000	\$0
	300mm PVC watermain	0	ea.	\$325	\$0
	300mm Gate valve	0	ea.	\$4,000	\$0
	subtotal				\$619,725
	Contingency and Engineering (40%)				\$247,890
	total				\$867,615
	rounded total				\$870,000

*Note: Estimates do not include land acquisition costs.
Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.*

Pre-Design Cost Estimate
Oyama Road Watermain Upgrades - Trask Road to Greenhow Road

Project Description	Project #	2.2
Upgrade Oyama Road watermain to 300mm	USL Job No.	1577.0063.01
Reconnect existing water services to new mains	Date	06/27/2016
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	Prepared by:	J.Barta
	Checked by:	J. Clowes
	See Figure	2

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$35,000	\$35,000
	Insurance and Bonding	1	LS	\$10,000	\$10,000
	75mm asphalt restoration	1020	sq.m	\$20	\$20,400
	100mm base course gravel restoration	1020	sq.m	\$6	\$6,120
	350mm subbase course gravel restoration	1020	sq.m	\$15	\$15,300
	Rock removal allowance	150	m	\$150	\$22,500
	Utility conflict allowance	1	ls	\$10,000	\$10,000
	Reconnect existing service to new watermain	8	ea	\$1,500	\$12,000
	Replace existing hydrant assembly and lead	0	ea	\$6,500	\$0
	New 150mm hydrant assembly	0	ea	\$5,500	\$0
	Tie-in to existing 100mm watermain	0	ea	\$5,000	\$0
	Tie-in to existing 150mm watermain	0	ea	\$5,000	\$0
	Tie-in to existing 200mm watermain	0	ea.	\$5,000	\$0
	Tie-in to existing 250mm watermain	0	ea	\$5,000	\$0
	Tie-in to existing 300mm watermain	2	ea	\$5,000	\$10,000
	Tie-in to existing 350mm watermain	0	ea	\$5,500	\$0
	Tie-in to existing 400mm watermain	0	ea	\$6,000	\$0
	150mm PVC watermain	0	m	\$200	\$0
	150mm Gate valve	0	ea	\$1,500	\$0
	200mm PVC watermain	0	m	\$250	\$0
	200mm Gate valve	0	ea	\$2,000	\$0
	250mm PVC watermain	0	m	\$275	\$0
	250mm Gate valve	0	ea	\$3,000	\$0
	300mm PVC watermain	340	ea.	\$325	\$110,500
	300mm Gate valve	2	ea.	\$4,000	\$8,000
	subtotal				\$259,820
	Contingency and Engineering (40%)				\$103,928
	total				\$363,748
	rounded total				\$360,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Irvine Boosted Zone - Removals and New Booster Station**

Project Description	Project # 3.1
Per Upper Pressure Zone Review Report (Appendix C)	USL Job No. 1577.0063.01
Remove reservoir from existing site 1	Date 06/27/2016
Remove pump station from existing site 1	Prepared by: J.Barta
Decommission pump station at site 2.	Checked by: J. Clowes
Proposed site works and pump station at site 1	See Figure 3

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization and Demobilization	1	LS	\$20,000	\$20,000
	Insurance and Bonding	1	LS	\$12,000	\$12,000
	Remove existing 150m3 reservoir from site 1	1	LS	\$30,000	\$30,000
	Remove existing site pump station	1	LS	\$20,000	\$20,000
	Decommission site 2 pump station and discharge line	1	LS	\$20,000	\$20,000
	Site works - Refer to Upper Pressure Zone Review in Appendix C	1	LS	\$89,800	\$89,800
	Pump Station - Refer to Upper Pressure Zone Review in Appendix C	1	LS	\$600,000	\$600,000
	subtotal				\$791,800
	Contingency and Engineering (35%)				\$277,130
	total				\$1,068,930
	rounded total				\$1,070,000

Note: Estimates do not include land acquisition costs.

**Pre-Design Cost Estimate
Irvine Boosted Zone - Watermain Upgrades**

Project Description	Project # 3.2
Upgrade existing watermain to 200mm from Irvine Booster Station to south end of Old Mission Road.	USL Job No. 1577.0063.01
Remove and replace existing AC watermain between Old Mission Road and Highland Road. Create loop back up to Old Mission Road	Date 06/27/2016
Install two private hydrants between Old Mission Road and Irvine PS	Prepared by: J.Barta
Reconnect existing water services to new mains	Checked by: J. Clowes
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	See Figure 3
Replace/add air vacuum valves per Upper Supply Zone Report	
Upgrade piping from Irvine Reservoir to Old Mission Road for lower pressure zone.	

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$35,000	\$35,000
	Insurance and Bonding	1	LS	\$10,000	\$10,000
	75mm asphalt restoration	1260	sq.m	\$20	\$25,200
	100mm base course gravel restoration	1260	sq.m	\$6	\$7,560
	350mm subbase course gravel restoration	1260	sq.m	\$15	\$18,900
	Rock removal allowance	400	m	\$150	\$60,000
	Utility conflict allowance	1	ls	\$20,000	\$20,000
	Remove and dispose existing 100mm watermain	340	m	\$50	\$17,000
	Reconnect existing service to new watermain	40	ea	\$1,500	\$60,000
	Replace existing hydrant assembly and lead	3	ea	\$6,500	\$19,500
	New 150mm hydrant assembly	2	ea	\$5,500	\$11,000
	Tie-in to existing 200mm watermain	1	ea.	\$5,000	\$5,000
	Tie-in to existing 300mm watermain	2	ea	\$5,000	\$10,000
	Replace air/vacuum valve	6	ea	\$1,000	\$6,000
	Add air/vacuum valve	4	ea	\$6,000	\$24,000
	200mm PVC watermain	1445	m	\$250	\$361,250
	200mm Gate valve	6	ea	\$2,000	\$12,000
	300mm PVC watermain	330	ea.	\$325	\$107,250
	300mm Gate valve	2	ea.	\$4,000	\$8,000
	subtotal				\$817,660
	Contingency and Engineering (40%)				\$327,064
	total				\$1,144,724
	rounded total				\$1,140,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Cornwall Road - Waterman Upgrades - Option 1**

Project Description	Project # 4.1
Upgrade existing 200mm main to 250mm along Pelme wash Parkway	USL Job No. 1577.0063.01
Upgrade existing 150mm main to 200mm from Pelme wash to Cornwall Road	Date 06/27/2016
Upgrade existing 100mm main to 200mm along Old Mission Road	Prepared by: J.Barta
Upgrade Sheldon, Cornwall and Highland Road water mains to 200mm	Checked by: J. Clowes
Abandon northern Cornwall watermain between Sheldon and Highland Roads	See Figure 4-2
Create new 200mm loop linking Sheldon and Highland Roads at south end	
Add new hydrant along Pelme wash Parkway and north end of Sheldon Road	
Replace existing hydrant assembly and lead (7)	
Reconnect existing water services to new mains	
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$45,000	\$45,000
	Insurance and Bonding	1	LS	\$15,000	\$15,000
	75mm asphalt restoration	2280	sq.m	\$20	\$45,600
	100mm base course gravel restoration	2280	sq.m	\$6	\$13,680
	350mm subbase course gravel restoration	2280	sq.m	\$15	\$34,200
	Rock removal allowance	800	m	\$150	\$120,000
	Utility conflict allowance	1	ls	\$30,000	\$30,000
	Cap/abandon main to old pump station at Pelme wash Parkway	1	LS	\$50,000	\$50,000
	Reconnect existing service to new watermain	29	ea	\$1,500	\$43,500
	Replace existing hydrant assembly and lead	7	ea	\$6,500	\$45,500
	New 150mm hydrant assembly	2	ea	\$5,500	\$11,000
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 300mm watermain	2	ea	\$5,000	\$10,000
	200mm PVC watermain	1620	m	\$250	\$405,000
	200mm Gate valve	6	ea	\$2,000	\$12,000
	250mm PVC watermain	460	m	\$275	\$126,500
	250mm Gate valve	2	ea	\$3,000	\$6,000
	subtotal				\$1,017,980
	Contingency and Engineering (40%)				\$407,192
	total				\$1,425,172
	rounded total				\$1,430,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Cornwall Road - Waterman Upgrades - Option 2**

Project Description	Project # 4.2
Proposed 200mm loop from Old Mission Road to Pelme wash Parkway	USL Job No. 1577.0063.01
Upgrade existing 150mm main to 200mm from Pelme wash to Cornwall Road	Date 06/27/2016
Upgrade existing 100mm main to 200mm along Old Mission Road	Prepared by: J.Barta
Upgrade Sheldon, Cornwall and Highland Road water mains to 200mm	Checked by: J. Clowes
Abandon northern Cornwall watermain between Sheldon and Highland Roads	See Figure 4-3
Create new 200mm loop linking Sheldon and Highland Roads at south end	
Add new hydrant along Pelme wash Parkway and north end of Sheldon Road	
Replace existing hydrant assembly and lead (7)	
Reconnect existing water services to new mains	
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$45,000	\$45,000
	Insurance and Bonding	1	LS	\$15,000	\$15,000
	75mm asphalt restoration	2280	sq.m	\$20	\$45,600
	100mm base course gravel restoration	2280	sq.m	\$6	\$13,680
	350mm subbase course gravel restoration	2280	sq.m	\$15	\$34,200
	Rock removal allowance	750	m	\$150	\$112,500
	Utility conflict allowance	1	ls	\$30,000	\$30,000
	Cap/abandon main to old pump station at Pelme wash Parkway	1	LS	\$3,000	\$3,000
	Reconnect existing service to new watermain	29	ea	\$1,500	\$43,500
	Replace existing hydrant assembly and lead	7	ea	\$6,500	\$45,500
	New 150mm hydrant assembly	2	ea	\$5,500	\$11,000
	Tie-in to existing 150mm watermain	4	ea	\$5,000	\$20,000
	200mm PVC watermain	1945	m	\$250	\$486,250
	200mm Gate valve	8	ea	\$2,000	\$16,000
	subtotal				\$921,230
	Contingency and Engineering (40%)				\$368,492
	total				\$1,289,722
	rounded total				\$1,290,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Carrs Landing - Carrs Landing PRV Replacement**

Project Description		Project #	5.1
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 4", 2"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	487	Checked by:	J. Clowes
Existing HGL setting on downstream side of PRV (m)	437	See Figure	5-1 and 5-2
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	177		
Estimated existing maximum day demand (L/min)	918		
Ultimate MDD including Ravenridge and Moberly developments (L/min)	3,753		
Estimated existing peak hour demand (L/min)	1,297		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea.	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea.	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea.	\$4,000	\$4,000
	200mm Mag meter	1	ea.	\$6,000	\$6,000
	200 mm expansion joints	2	ea.	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea.	\$500	\$1,000
	sampling tap	1	ea.	\$500	\$500
	air release	1	ea.	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
	subtotal				\$239,500
	Contingency and Engineering (35%)				\$83,825
	total				\$323,325
	rounded total				\$320,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Carrs Landing - Watermain Upgrades Option #1**

Project Description	Project #	5.2
Upgrade Carrs Landing watermain to 250mm	USL Job No.	1577.0063.01
Upgrade Carrs Landing watermain to 200mm north of Moberly Road	Date	06/27/2016
Upgrade McCreight Road watermain to 200mm	Prepared by:	J.Barta
Upgrade Pixton Road watermain to 200mm	Checked by:	J. Clowes
Add new hydrants (7) along Carrs Landing Road	See Figure	5-1
Add new hydrant along Pixton Road		
Replace existing Pixton Road hydrant assembly and lead		
Reconnect existing water services to new mains		
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$75,000	\$75,000
	Insurance and Bonding	1	LS	\$20,000	\$20,000
	75mm asphalt restoration	9225	sq.m	\$20	\$184,500
	100mm base course gravel restoration	9225	sq.m	\$6	\$55,350
	350mm subbase course gravel restoration	9225	sq.m	\$15	\$138,375
	Rock removal allowance	750	m	\$150	\$112,500
	Utility conflict allowance	1	ls	\$30,000	\$30,000
	Reconnect existing service to new watermain	23	ea	\$1,500	\$34,500
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	New 150mm hydrant assembly	8	ea	\$5,500	\$44,000
	Tie-in to existing 100mm watermain	2	ea	\$5,000	\$10,000
	Tie-in to existing 250mm watermain	1	ea	\$5,000	\$5,000
	200mm PVC watermain	1290	m	\$250	\$322,500
	200mm Gate valve	6	ea	\$2,000	\$12,000
	250mm PVC watermain	1995	m	\$275	\$548,625
	250mm Gate valve	4	ea	\$3,000	\$12,000
	subtotal				\$1,610,850
	Contingency and Engineering (40%)				\$644,340
	total				\$2,255,190
	rounded total				\$2,260,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Carrs Landing - Watermain Upgrades Option #2 (Pixton Road loop)**

Project Description	Project #	5.3
Upgrade Carrs Landing watermain to 250mm	USL Job No.	1577.0063.01
Upgrade Carrs Landing watermain to 200mm north of Moberly Road	Date	06/27/2016
Upgrade McCreight Road watermain to 200mm	Prepared by:	J.Barta
Upgrade Pixton Road watermain to 200mm	Checked by:	J. Clowes
Add new hydrants (7) along Carrs Landing Road	See Figure	5-2
Add new hydrant along Pixton Road		
Replace existing Pixton Road hydrant assembly and lead		
New 200mm loop from Carrs Landing PRV to south end Pixton Road		
Reconnect existing water services to new mains		
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$75,000	\$75,000
	Insurance and Bonding	1	LS	\$20,000	\$20,000
	75mm asphalt restoration	8745	sq.m	\$20	\$174,900
	100mm base course gravel restoration	8745	sq.m	\$6	\$52,470
	350mm subbase course gravel restoration	8745	sq.m	\$15	\$131,175
	Rock removal allowance	750	m	\$150	\$112,500
	Utility conflict allowance	1	ls	\$30,000	\$30,000
	Reconnect existing service to new watermain	23	ea	\$1,500	\$34,500
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	New 150mm hydrant assembly	8	ea	\$5,500	\$44,000
	Tie-in to existing 100mm watermain	2	ea	\$5,000	\$10,000
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 250mm watermain	1	ea	\$5,000	\$5,000
	200mm PVC watermain	1200	m	\$250	\$300,000
	200mm Gate valve	5	ea	\$2,000	\$10,000
	250mm PVC watermain	1995	m	\$275	\$548,625
	250mm Gate valve	4	ea	\$3,000	\$12,000
	subtotal				\$1,571,670
	Contingency and Engineering (40%)				\$628,668
	total				\$2,200,338
	rounded total				\$2,200,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Carrs Landing - Carrs Landing Road Oversizing for Moberly/RavenRidge

Project Description	Project #	5.4
Oversize Carrs Landing watermain from 250mm to 300mm to accommodate future Moberly Road and/or RavenRidge customers	USL Job No.	1577.0063.01
	Date	06/27/2016
Oversizing cost calculated by including negative value of 250mm diameter watermain and positive value of 300mm diameter watermain	Prepared by:	J.Barta
	Checked by:	J. Clowes
	See Figure	5-3

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	250mm PVC watermain	-1270	m	\$275	-\$349,250
	250mm Gate valve	-5	ea	\$3,000	-\$15,000
	300mm PVC watermain	1640	ea.	\$325	\$533,000
	300mm Gate valve	6	ea.	\$4,000	\$24,000
				subtotal	\$192,750
				Contingency and Engineering (40%)	\$77,100
				total	\$269,850
				rounded total	\$270,000

*Note: Estimates do not include land acquisition costs.
Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.*

**Pre-Design Cost Estimate
Carrs Landing - Optional PRV Replacement - Goldie PRV**

Project Description		Project #	5.5
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 6", 2"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	575	Checked by:	J. Clowes
Existing HGL setting on downstream side of PRV (m)	491	See Figure	5-1 and 5-2
Required fire flow through station (L/min)	5000		
Estimate existing winter day demand (L/min)	190		
Estimated existing maximum day demand (L/min)	1,292		
Ultimate MDD including Ravenridge and Moberly developments (L/min)	4,127		
Estimated existing peak hour demand (L/min)	1,711		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea.	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea.	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea.	\$4,000	\$4,000
	200mm Mag meter	1	ea.	\$6,000	\$6,000
	200 mm expansion joints	2	ea.	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea.	\$500	\$1,000
	sampling tap	1	ea.	\$500	\$500
	air release	1	ea.	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
	subtotal				\$239,500
	Contingency and Engineering (35%)				\$83,825
	total				\$323,325
	rounded total				\$320,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Ponderosa Drive - Watermain Upgrades**

Project Description	Project #	6.1
Upgrade Old Mission Road watermain to 200mm	USL Job No.	1577.0063.01
Upgrade Ponderosa Drive watermain to 200mm	Date	06/27/2016
Replace exist standpipes with hydrants. Maintained existing 260m spacing	Prepared by:	J.Barta
Reconnect existing water services to new mains	Checked by:	J. Clowes
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	See Figure	6-1

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$25,000	\$25,000
	Insurance and Bonding	1	LS	\$6,000	\$6,000
	75mm asphalt restoration	1560	sq.m	\$20	\$31,200
	100mm base course gravel restoration	2670	sq.m	\$6	\$16,020
	350mm subbase course gravel restoration	2670	sq.m	\$15	\$40,050
	Rock removal allowance	400	m	\$150	\$60,000
	Utility conflict allowance	1	ls	\$10,000	\$10,000
	Reconnect existing service to new watermain	21	ea	\$1,500	\$31,500
	New 150mm hydrant assembly	3	ea	\$5,500	\$16,500
	Tie-in to existing 200mm watermain	3	ea.	\$5,000	\$15,000
	200mm PVC watermain	890	m	\$250	\$222,500
	200mm Gate valve	4	ea	\$2,000	\$8,000
				subtotal	\$481,770
				Contingency and Engineering (40%)	\$192,708
				total	\$674,478
				rounded total	\$670,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Coral Beach - Watermain Upgrades**

Project Description	Project #	7.1
Upgrade distribution mains to 200mm up to hydrant extents	USL Job No.	1577.0063.01
Construct 700 cubic meter reservoir addition	Date	06/27/2016
Replace existing hydrant assembly and lead (3)	Prepared by:	J. Barta
Reconnect existing water services to new mains	Checked by:	J. Clowes
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	See Figure	7

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$40,000	\$40,000
	Insurance and Bonding	1	LS	\$12,000	\$12,000
	75mm asphalt restoration	2940	sq.m	\$20	\$58,800
	100mm base course gravel restoration	2940	sq.m	\$6	\$17,640
	350mm subbase course gravel restoration	2940	sq.m	\$15	\$44,100
	Rock removal allowance	400	m	\$150	\$60,000
	Utility conflict allowance	1	ls	\$20,000	\$20,000
	Reconnect existing service to new watermain	41	ea	\$1,500	\$61,500
	Replace existing hydrant assembly and lead	3	ea	\$6,500	\$19,500
	New 150mm hydrant assembly	3	ea	\$5,500	\$16,500
	Cast-in-place concrete (reservoir)	700	cu.m	\$300	\$210,000
	Reservoir inlet/outlet piping	1	LS	\$100,000	\$100,000
	Tie-in to existing 100mm watermain	2	ea	\$5,000	\$10,000
	Tie-in to existing 250mm watermain	1	ea	\$5,000	\$5,000
	200mm PVC watermain	1180	m	\$250	\$295,000
	200mm Gate valve	6	ea	\$2,000	\$12,000
	subtotal				\$982,040
	Contingency and Engineering (40%)				\$392,816
	total				\$1,374,856
	rounded total				\$1,370,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate					
Owls Nest - Watermain Upgrades - North of Evans Road (Common for Option 1 & 2)					
Project Description				Project #	8.1.1
Upgrade all mains to 200mm diameter				USL Job No.	1577.0063.01
Replace hydrant assembly and lead (2)				Date	06/27/2016
Reconnect existing water services to new mains				Prepared by:	J.Barta
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration				Checked by:	J.Clowes
				See Figure	8-1
ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$10,000	\$10,000
	Insurance and Bonding	1	LS	\$3,000	\$3,000
	75mm asphalt restoration	1080	sq.m	\$20	\$21,600
	100mm base course gravel restoration	1080	sq.m	\$6	\$6,480
	350mm subbase course gravel restoration	1080	sq.m	\$15	\$16,200
	Rock removal allowance	150	m	\$150	\$22,500
	Utility conflict allowance	1	ls	\$10,000	\$10,000
	Reconnect existing service to new watermain	8	ea	\$1,500	\$12,000
	Replace existing hydrant assembly and lead	2	ea	\$6,500	\$13,000
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 250mm watermain	1	ea	\$5,000	\$5,000
	200mm PVC watermain	360	m	\$250	\$90,000
	200mm Gate valve	2	ea	\$2,000	\$4,000
				subtotal	\$218,780
				Contingency and Engineering (40%)	\$87,512
				total	\$306,292
				rounded total	\$310,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Owls Nest - Watermain Upgrades - South of Evans Road - Option 1

Project Description	Project #	8.1.2
Upgrade existing supply main from Irvine Road to Evans Road to 250mm	USL Job No.	1577.0063.01
Replace series 160 piping at north end of Old Mission Road with PVC	Date	06/27/2016
Reconnect existing water services & hydrants to new mains	Prepared by:	J.Barta
Cost estimate does not include any restoration costs within orchard	Checked by:	J.Clowes
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	See Figure	8-1

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$45,000	\$45,000
	Insurance and Bonding	1	LS	\$12,000	\$12,000
	75mm asphalt restoration	3900	sq.m	\$20	\$78,000
	100mm base course gravel restoration	3900	sq.m	\$6	\$23,400
	350mm subbase course gravel restoration	3900	sq.m	\$15	\$58,500
	Rock removal allowance	750	m	\$150	\$112,500
	Utility conflict allowance	1	ls	\$30,000	\$30,000
	Reconnect existing service to new watermain	28	ea	\$1,500	\$42,000
	Reconnect existing hydrant to new watermain	5	ea	\$2,000	\$10,000
	Tie-in to existing 300mm watermain	1	ea	\$5,000	\$5,000
	200mm PVC watermain	240	m	\$250	\$60,000
	250mm PVC watermain	1550	m	\$275	\$426,250
	250mm Gate valve	5	ea	\$3,000	\$15,000
	subtotal				\$917,650
	Contingency and Engineering (40%)				\$367,060
	total				\$1,284,710
	rounded total				\$1,280,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Owls Nest - Watermain Upgrades - South of Evans Road - Option 2

Project Description	Project #	8.1.3
Upgrade Old Mission Road watermain to 250mm diameter	USL Job No.	1577.0063.01
Proposed Pelme wash Parkway 250mm loop to Owls Nest Road	Date	06/27/2016
Existing watermain from Buttersworth to Owls Nest Road to remain in service	Prepared by:	J.Barta
Reconnect existing water services & hydrants to new mains	Checked by:	J.Clowes
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	See Figure	8-1

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$50,000	\$50,000
	Insurance and Bonding	1	LS	\$15,000	\$15,000
	75mm asphalt restoration	5580	sq.m	\$20	\$111,600
	100mm base course gravel restoration	5580	sq.m	\$6	\$33,480
	350mm subbase course gravel restoration	5580	sq.m	\$15	\$83,700
	Rock removal allowance	750	m	\$150	\$112,500
	Utility conflict allowance	1	ls	\$30,000	\$30,000
	Reconnect existing service to new watermain	28	ea	\$1,500	\$42,000
	Reconnect existing hydrant to new watermain	5	ea	\$2,000	\$10,000
	Tie-in to existing 300mm watermain	1	ea	\$5,000	\$5,000
	250mm PVC watermain	1860	m	\$275	\$511,500
	250mm Gate valve	4	ea	\$3,000	\$12,000
				subtotal	\$1,016,780
				Contingency and Engineering (40%)	\$406,712
				total	\$1,423,492
				rounded total	\$1,420,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Seaton Road - Watermain Upgrades**

Project Description	Project #	8.2
Upgrade existing undersized watermain to 200mm diameter	USL Job No.	1577.0063.01
Abandon undersized watermain	Date	06/27/2016
Tie in to Seaton Road culdesac with 200mm tee and new gate valves	Prepared by:	J.Barta
Reconnect existing water services to new mains	Checked by:	J.Clowes
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	See Figure	8-3

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$13,000	\$13,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	75mm asphalt restoration	1410	sq.m	\$20	\$28,200
	100mm base course gravel restoration	1410	sq.m	\$6	\$8,460
	350mm subbase course gravel restoration	1410	sq.m	\$15	\$21,150
	Rock removal allowance	200	m	\$150	\$30,000
	Utility conflict allowance	1	ls	\$10,000	\$10,000
	Reconnect existing service to new watermain	9	ea	\$1,500	\$13,500
	New 150mm hydrant assembly	2	ea	\$5,500	\$11,000
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 200mm watermain	3	ea.	\$5,000	\$15,000
	200mm PVC watermain	470	m	\$250	\$117,500
	200mm Gate valve	3	ea	\$2,000	\$6,000
	subtotal				\$282,810
	Contingency and Engineering (40%)				\$113,124
	total				\$395,934
	rounded total				\$400,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Woodsdale-Oceola Road - Watermain Looping

Project Description	Project #	8.3
280m of 250mm watermain along Oceola per DCC project W-10	USL Job No.	1577.0063.01
Highway crossing (casing pipe) per DCC project W-10	Date	06/27/2016
700m of 250mm along Woodsdale to form new loop	Prepared by:	J.Barta
New hydrant at Woodsdale Ct	Checked by:	J.Clowes
Replace hydrant assembly and lead	See Figure	8-4
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$40,000	\$40,000
	Insurance and Bonding	1	LS	\$10,000	\$10,000
	75mm asphalt restoration	2850	sq.m	\$20	\$57,000
	100mm base course gravel restoration	2850	sq.m	\$6	\$17,100
	350mm subbase course gravel restoration	2850	sq.m	\$15	\$42,750
	Rock removal allowance	500	m	\$150	\$75,000
	Utility conflict allowance	1	ls	\$30,000	\$30,000
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	New 150mm hydrant assembly	1	ea	\$5,500	\$5,500
	Tie-in to existing 250mm watermain	2	ea	\$5,000	\$10,000
	600mm casing pipe under highway 97 by trenchless methods	80	m	\$3,000	\$240,000
	250mm PVC watermain	1030	m	\$275	\$283,250
	250mm Gate valve	6	ea	\$3,000	\$18,000
				subtotal	\$835,100
				Contingency and Engineering (40%)	\$334,040
				total	\$1,169,140
				rounded total	\$1,170,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

**Pre-Design Cost Estimate
Broadwater & Oyama Roads - Watermain Looping**

Project Description	Project # 8.4
Construct 200mm loop on Broadwater Road	USL Job No. 1577.0063.01
Construct 200mm loop on Oyama Road	Date 06/27/2016
Upgrade existing mains to 200mm along Broadwater, west of Oyama Road	Prepared by: J.Barta
Replace hydrant assembly and lead (4)	Checked by: J.Clowes
Reconnect existing water services to new mains	See Figure 8-5
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$30,000	\$30,000
	Insurance and Bonding	1	LS	\$10,000	\$10,000
	75mm asphalt restoration	3780	sq.m	\$20	\$75,600
	100mm base course gravel restoration	3780	sq.m	\$6	\$22,680
	350mm subbase course gravel restoration	3780	sq.m	\$15	\$56,700
	Rock removal allowance	500	m	\$150	\$75,000
	Utility conflict allowance	1	ls	\$10,000	\$10,000
	Reconnect existing service to new watermain	15	ea	\$1,500	\$22,500
	Replace existing hydrant assembly and lead	4	ea	\$6,500	\$26,000
	Tie-in to existing 100mm watermain	5	ea	\$5,000	\$25,000
	200mm PVC watermain	1260	m	\$250	\$315,000
	200mm Gate valve	4	ea	\$2,000	\$8,000
	subtotal				\$676,480
	Contingency and Engineering (40%)				\$270,592
	total				\$947,072
	rounded total				\$950,000

Note: Estimates do not include land acquisition costs.

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Oyama Water System - Optional PRV Replacement - Todd Road (East Hill)

Project Description		Project #	8.5
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 4", 1.5"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	606	Checked by:	J.Clowes
Existing HGL setting on downstream side of PRV (m)	542	See Figure	8-6
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	20		
Estimated existing maximum day demand (L/min)	1,680		
Estimated existing peak hour demand (L/min)	2,520		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
				subtotal	\$239,500
				Contingency and Engineering (35%)	\$83,825
				total	\$323,325
				rounded total	\$320,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Oyama Water System - Optional PRV Replacement - Oyama Road (Cemetery)

Project Description		Project #	8.6
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 1.5", 3/4"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	532	Checked by:	J.Clowes
Existing HGL setting on downstream side of PRV (m)	480	See Figure	8-6
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	5		
Estimated existing maximum day demand (L/min)	100		
Estimated existing peak hour demand (L/min)	150		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
				subtotal	\$239,500
				Contingency and Engineering (35%)	\$83,825
				total	\$323,325
				rounded total	\$320,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Oyama Water System - Optional PRV Replacement - MiddleBench (Harmen)

Project Description		Project #	8.7
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 2.5", 1"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	596	Checked by:	J.Clowes
Existing HGL setting on downstream side of PRV (m)	543	See Figure	8-6
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	5		
Estimated existing maximum day demand (L/min)	620		
Estimated existing peak hour demand (L/min)	930		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
				subtotal	\$239,500
				Contingency and Engineering (35%)	\$83,825
				total	\$323,325
				rounded total	\$320,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Oyama Water System - Optional PRV Replacement - MiddleBench (Alison)

Project Description		Project #	8.8
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 4", 1"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	596	Checked by:	J.Clowes
Existing HGL setting on downstream side of PRV (m)	535	See Figure	8-6
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	25		
Estimated existing maximum day demand (L/min)	1,900		
Estimated existing peak hour demand (L/min)	2,850		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
				subtotal	\$239,500
				Contingency and Engineering (35%)	\$83,825
				total	\$323,325
				rounded total	\$320,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Oyama Water System - Optional PRV Replacement - Oyama Road (Broadwater)

Project Description		Project #	8.9
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 4", 1.5"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	535	Checked by:	J.Clowes
Existing HGL setting on downstream side of PRV (m)	494	See Figure	8-6
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	15		
Estimated existing maximum day demand (L/min)	1,680		
Estimated existing peak hour demand (L/min)	2,520		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
				subtotal	\$239,500
				Contingency and Engineering (35%)	\$83,825
				total	\$323,325
				rounded total	\$320,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Oyama Water System - Optional PRV Replacement - Towgood Road

Project Description		Project #	8.10
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 4", 2"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	586	Checked by:	J.Clowes
Existing HGL setting on downstream side of PRV (m)	550	See Figure	8-6
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	5		
Estimated existing maximum day demand (L/min)	2,300		
Estimated existing peak hour demand (L/min)	3,450		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
				subtotal	\$239,500
				Contingency and Engineering (35%)	\$83,825
				total	\$323,325
				rounded total	\$320,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Oyama Water System - Optional PRV Replacement - Trehwhitt Road

Project Description		Project #	8.11
Decommission existing PRV station.		USL Job No.	1577.0063.01
Replace with new above-ground facility		Date	06/27/2016
Existing valve sizes: 3", 2"		Prepared by:	J.Barta
Existing HGL measurement on upstream side of PRV (m)	617	Checked by:	J.Clowes
Existing HGL setting on downstream side of PRV (m)	552	See Figure	8-6
Required fire flow through station (L/min)	5000		
Estimated existing winter day demand (L/min)	25		
Estimated existing maximum day demand (L/min)	610		
Estimated existing peak hour demand (L/min)	915		

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$4,000	\$4,000
	Remove existing PRV station	1	ea.	\$15,000	\$15,000
	Pre-cast Concrete Building	1	ea.	\$50,000	\$50,000
	Foundation Prep for Building	1	ea.	\$10,000	\$10,000
	Connect to Existing	2	ea.	\$5,000	\$10,000
	200 mm PRV	1	ea	\$15,000	\$15,000
	38 mm PRV c/w isolation valves and piping	1	ls	\$5,000	\$5,000
	Butterfly valve - 200 mm	2	ea	\$2,500	\$5,000
	200mm Pipe (schedule 10SS)	15	m	\$1,200	\$18,000
	200mm strainer	1	ea	\$4,000	\$4,000
	200mm Mag meter	1	ea	\$6,000	\$6,000
	200 mm expansion joints	2	ea	\$5,000	\$10,000
	Pressure transducer	2	ea.	\$1,500	\$3,000
	pressure gauge	2	ea	\$500	\$1,000
	sampling tap	1	ea	\$500	\$500
	air release	1	ea	\$1,000	\$1,000
	Electrical Equipment	1	ea.	\$30,000	\$30,000
	Electrical Service	1	ea.	\$15,000	\$15,000
	Access Road and Gravel parking pad	1	ea.	\$15,000	\$15,000
	Site landscaping and restoration	1	ea.	\$10,000	\$10,000
				subtotal	\$239,500
				Contingency and Engineering (35%)	\$83,825
				total	\$323,325
				rounded total	\$320,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - Oyama Road, 250m south of Crawford Road

Project Description	Project #	8.12
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Install new valves on mainline and lead	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-6

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	2	ea	\$1,500	\$3,000
	subtotal				\$15,074
	Contingency and Engineering (40%)				\$6,030
	total				\$21,104
	rounded total				\$21,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - Intersection of Oyama and Eyles Roads

Project Description	Project #	8.13
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Install new valves on mainline and lead	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-6

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	2	ea	\$1,500	\$3,000
				subtotal	\$15,074
				Contingency and Engineering (40%)	\$6,030
				total	\$21,104
				rounded total	\$21,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Abandon GI watermain and reconnect services - Oyama Road at Eyles Road

Project Description	Project #	8.14
Abandon existing 50,, GI main	USL Job No.	1577.0063.01
Reconnect existing services(5) to existing 150 main along Oyama Road	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-8

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	70	sq.m	\$20	\$1,400
	100mm base course gravel restoration	70	sq.m	\$6	\$420
	350mm subbase course gravel restoration	70	sq.m	\$15	\$1,050
	Reconnect existing service to new watermain	5	ea	\$1,500	\$7,500
	subtotal				\$10,370
	Contingency and Engineering (40%)				\$4,148
	total				\$14,518
	rounded total				\$15,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - Intersection of Oyama and Trehwhitt Roads

Project Description	Project #	8.15
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Install new valves on mainline and lead	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	2	ea	\$1,500	\$3,000
	subtotal				\$15,074
	Contingency and Engineering (40%)				\$6,030
	total				\$21,104
	rounded total				\$21,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Replace GI watermain - Trehwitt Road, south of PRV station

Project Description	Project #	8.16
Abandon existing 50mm GI watermain	USL Job No.	1577.0063.01
New 50mm PVC water main	Date	06/27/2016
Reconnect existing services to new 50mm PVC main	Prepared by:	J.Barta
Main is gravel driveway, no asphalt or subbase restoration	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	100mm base course gravel restoration	70	sq.m	\$6	\$420
	Reconnect existing service to new watermain	1	ea	\$1,500	\$1,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	50mm PVC watermain	35	m	\$75	\$2,625
				subtotal	\$9,545
				Contingency and Engineering (40%)	\$3,818
				total	\$13,363
				rounded total	\$13,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Replace hydrant assembly and lead - Trew hitt Road at PRV station

Project Description	Project #	8.17
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Install new valves on mainline and lead	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	2	ea	\$1,500	\$3,000
	subtotal				\$15,074
	Contingency and Engineering (40%)				\$6,030
	total				\$21,104
	rounded total				\$21,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate					
Abandon GI watermain and reconnect services - Trehitt Road, north of PRV station					
Project Description			Project #	8.18	
Abandon existing 50mm GI watermain			USL Job No.	1577.0063.01	
Reconnect existing services(4) to existing 150 main along Oyama Road			Date	06/27/2016	
Assumed 2m wide trench across roadway for restoration quantity			Prepared by:	J.Barta	
			Checked by:	J.Clowes	
			See Figure	8-5	
ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	56	sq.m	\$20	\$1,120
	100mm base course gravel restoration	56	sq.m	\$6	\$336
	350mm subbase course gravel restoration	56	sq.m	\$15	\$840
	Reconnect existing service to new watermain	4	ea	\$1,500	\$6,000
				subtotal	\$8,296
				Contingency and Engineering (40%)	\$3,318
				total	\$11,614
				rounded total	\$12,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - Trehwitt Road, 270m east of Oyama Road

Project Description	Project #	8.19
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Install new valves on mainline and lead	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
	200mm Gate valve	1	ea	\$2,000	\$2,000
	subtotal				\$15,574
	Contingency and Engineering (40%)				\$6,230
	total				\$21,804
	rounded total				\$22,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate					
Replace hydrant assembly and lead - Oyama Road, 280m south of Towgood Road					
Project Description			Project #	8.20	
Replace hydrant assembly and lead			USL Job No.	1577.0063.01	
Install new valves on mainline and lead			Date	06/27/2016	
Assumed 2m wide trench across roadway for restoration quantity			Prepared by:	J.Barta	
			Checked by:	J.Clowes	
			See Figure	8-5	
ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	2	ea	\$1,500	\$3,000
				subtotal	\$15,074
				Contingency and Engineering (40%)	\$6,030
				total	\$21,104
				rounded total	\$21,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate					
Replace hydrant assembly and lead - Middle Bench Road, 270m south of Towgood Road					
Project Description			Project #	8.21	
Replace hydrant assembly and lead			USL Job No.	1577.0063.01	
Install new valves on 250mm mainline and lead			Date	06/27/2016	
Assumed 2m wide trench across roadway for restoration quantity			Prepared by:	J.Barta	
			Checked by:	J.Clowes	
			See Figure	8-5	
ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 250mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
	250mm Gate valve	1	ea	\$3,000	\$3,000
				subtotal	\$16,574
				Contingency and Engineering (40%)	\$6,630
				total	\$23,204
				rounded total	\$23,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - Intersection of Middle Bench and Towgood Roads

Project Description	Project #	8.22
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Hot tap on 350mm watermain c/w 150mm valve	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 350mm watermain	1	ea	\$5,500	\$5,500
	150mm Gate valve	1	ea	\$1,500	\$1,500
	subtotal				\$14,074
	Contingency and Engineering (40%)				\$5,630
	total				\$19,704
	rounded total				\$20,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - Intersection of Middle Bench and Allison Roads

Project Description	Project #	8.23
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Hot tap on 400mm watermain c/w 150mm valve	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 400mm watermain	1	ea	\$6,000	\$6,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
	subtotal				\$14,574
	Contingency and Engineering (40%)				\$5,830
	total				\$20,404
	rounded total				\$20,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate					
Replace hydrant assembly and lead - Middle Bench Road, 150m north of Allison Road					
Project Description			Project #	8.24	
Replace hydrant assembly and lead			USL Job No.	1577.0063.01	
Hot tap on 400mm watermain c/w 150mm valve			Date	06/27/2016	
Assumed 2m wide trench across roadway for restoration quantity			Prepared by:	J.Barta	
			Checked by:	J.Clowes	
			See Figure	8-5	
ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 400mm watermain	1	ea	\$6,000	\$6,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
subtotal					\$14,574
Contingency and Engineering (40%)					\$5,830
total					\$20,404
rounded total					\$20,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate					
Abandoned GI watermain and reconnect services - Middle Bench Road near Harmen Road					
Project Description			Project #	8.25	
Abandon existing 50mm and 100mm GI mains			USL Job No.	1577.0063.01	
Reconnect Middle Bench Road services (3) to existing 500mm main in roadway			Date	06/27/2016	
Assumed 2m wide trench across roadway for restoration quantity			Prepared by:	J.Barta	
			Checked by:	J.Clowes	
			See Figure	8-5	
ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	42	sq.m	\$20	\$840
	100mm base course gravel restoration	42	sq.m	\$6	\$252
	350mm subbase course gravel restoration	42	sq.m	\$15	\$630
	Reconnect existing service to new watermain	3	ea	\$1,500	\$4,500
	Tie-in to existing 400mm watermain	3	ea	\$6,000	\$18,000
				subtotal	\$24,222
				Contingency and Engineering (40%)	\$9,689
				total	\$33,911
				rounded total	\$34,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate					
Replace hydrant assembly and lead - Intersection of Middle Bench and East Hill Roads					
Project Description			Project #	8.26	
Replace hydrant assembly and lead			USL Job No.	1577.0063.01	
Hot tap on 500mm watermain c/w 150mm valve			Date	06/27/2016	
Assumed 2m wide trench across roadway for restoration quantity			Prepared by:	J.Barta	
			Checked by:	J.Clowes	
			See Figure	8-5	
ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 400mm watermain	1	ea	\$6,000	\$6,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
subtotal					\$14,574
Contingency and Engineering (40%)					\$5,830
total					\$20,404
rounded total					\$20,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace AC watermains and reconnect services - Middle Bench Road, North of East Hill Road

Project Description	Project #	8.27
Remove existing 100mm AC watermain	USL Job No.	1577.0063.01
Reconnect services to new 100mm PVC watermain	Date	06/27/2016
Hot-tap existing 500mm main c/w 100mm valve	Prepared by:	J.Barta
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$5,000	\$5,000
	Insurance and Bonding	1	LS	\$1,500	\$1,500
	75mm asphalt restoration	570	sq.m	\$20	\$11,400
	100mm base course gravel restoration	570	sq.m	\$6	\$3,420
	350mm subbase course gravel restoration	570	sq.m	\$15	\$8,550
	Rock removal allowance	100	m	\$150	\$15,000
	Remove and dispose existing 100mm watermain	190	m	\$50	\$9,500
	Reconnect existing service to new watermain	7	ea	\$1,500	\$10,500
	Tie-in to existing 400mm watermain	1	ea	\$6,000	\$6,000
	100mm PVC watermain	190	m	\$150	\$28,500
	100mm Gate valve	1	ea	\$1,250	\$1,250
	subtotal				\$100,620
	Contingency and Engineering (40%)				\$40,248
	total				\$140,868
	rounded total				\$141,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - East Hill Road, 250m south of Todd Road

Project Description	Project #	8.28
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Hot tap on 500mm watermain c/w 150mm valve	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 400mm watermain	1	ea	\$6,000	\$6,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
	subtotal				\$14,574
	Contingency and Engineering (40%)				\$5,830
	total				\$20,404
	rounded total				\$20,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - East Hill Road, 530m south of Todd Road

Project Description	Project #	8.29
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Hot tap on 500mm watermain c/w 150mm valve	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 400mm watermain	1	ea	\$6,000	\$6,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
	subtotal				\$14,574
	Contingency and Engineering (40%)				\$5,830
	total				\$20,404
	rounded total				\$20,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Abandon AC watermain and reconnect services - Middle Bench Road, 500m south of Todd Road

Project Description	Project # 8.30
Abandon AC watermain (2) on east and west side of 500mm trunk	USL Job No. 1577.0063.01
Reconnect existing services (9) to 500mm trunk watermain	Date 06/27/2016
Assumed 2m wide trench within roadway for restoration quantity	Prepared by: J.Barta
	Checked by: J.Clowes
	See Figure 8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	108	sq.m	\$20	\$2,160
	100mm base course gravel restoration	108	sq.m	\$6	\$648
	350mm subbase course gravel restoration	108	sq.m	\$15	\$1,620
	Reconnect existing service to new watermain	9	ea	\$1,500	\$13,500
	subtotal				\$17,928
	Contingency and Engineering (40%)				\$7,171
	total				\$25,099
	rounded total				\$25,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Replace watermains - Middle Bench Road, 500m south of Todd Road

Project Description	Project #	8.31
Abandon existing 38 and 50mm service mains	USL Job No.	1577.0063.01
Install new 50mm PVC watermain	Date	06/27/2016
Reconnect existing services (2) to new 50mm watermain	Prepared by:	J.Barta
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$2,500	\$2,500
	Insurance and Bonding	1	LS	\$750	\$750
	75mm asphalt restoration	390	sq.m	\$20	\$7,800
	100mm base course gravel restoration	390	sq.m	\$6	\$2,340
	350mm subbase course gravel restoration	390	sq.m	\$15	\$5,850
	Rock removal allowance	70	m	\$150	\$10,500
	Reconnect existing service to new watermain	2	ea	\$1,500	\$3,000
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 400mm watermain	1	ea	\$6,000	\$6,000
	50mm PVC watermain	130	m	\$70	\$9,100
	subtotal				\$52,840
	Contingency and Engineering (40%)				\$21,136
	total				\$73,976
	rounded total				\$74,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate					
Replace hydrant assembly and lead - Middle Bench Road, 400m south of Todd Road					
Project Description			Project #	8.32	
Replace hydrant assembly and lead			USL Job No.	1577.0063.01	
Install new valves on mainline and lead			Date	06/27/2016	
Assumed 2m wide trench across roadway for restoration quantity			Prepared by:	J.Barta	
			Checked by:	J.Clowes	
			See Figure	8-5	
ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	2	ea	\$1,500	\$3,000
subtotal					\$15,074
Contingency and Engineering (40%)					\$6,030
total					\$21,104
rounded total					\$21,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - Intersection of East Hill and Todd Roads

Project Description	Project #	8.33
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Install new valves on mainline and lead	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-5

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 350mm watermain	1	ea	\$5,500	\$5,500
	150mm Gate valve	1	ea	\$1,500	\$1,500
	300mm Gate valve	1	ea.	\$4,000	\$4,000
	subtotal				\$18,074
	Contingency and Engineering (40%)				\$7,230
	total				\$25,304
	rounded total				\$25,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Replace 100mm AC watermain with 150mm PVC water - Lacresta Road

Project Description	Project #	8.34
Abandon existing 100mm AC watermain.	USL Job No.	1577.0063.01
Tie in to existing 150mm watermain on south side of Lacresta	Date	06/27/2016
Reconnect existing services (5) to new 150mm PVC watermain	Prepared by:	J.Barta
Reconnect existing hydrant to new 150mm watermain	Checked by:	J.Clowes
Assumed half road restoration per watermain length	See Figure	8-6

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$3,000	\$3,000
	Insurance and Bonding	1	LS	\$1,000	\$1,000
	75mm asphalt restoration	315	sq.m	\$20	\$6,300
	100mm base course gravel restoration	315	sq.m	\$6	\$1,890
	350mm subbase course gravel restoration	315	sq.m	\$15	\$4,725
	Rock removal allowance	50	m	\$150	\$7,500
	Reconnect existing service to new watermain	5	ea	\$1,500	\$7,500
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 100mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm PVC watermain	90	m	\$200	\$18,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
				subtotal	\$67,915
				Contingency and Engineering (40%)	\$27,166
				total	\$95,081
				rounded total	\$95,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Replace 50mm GI watermain with 50mm PVC - Floral Road

Project Description	Project #	8.35
Abandon existing 50mm GI watermain	USL Job No.	1577.0063.01
Replace with 50mm PVC watermain	Date	06/27/2016
Reconnect existing services (3) to new 150mm PVC watermain	Prepared by:	J.Barta
Assumed half road restoration per watermain length	Checked by:	J.Clowes
	See Figure	8-6

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	245	sq.m	\$20	\$4,900
	100mm base course gravel restoration	245	sq.m	\$6	\$1,470
	350mm subbase course gravel restoration	245	sq.m	\$15	\$3,675
	Rock removal allowance	30	m	\$150	\$4,500
	Reconnect existing service to new watermain	3	ea	\$1,500	\$4,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	50mm PVC watermain	70	m	\$70	\$4,900
				subtotal	\$28,945
				Contingency and Engineering (40%)	\$11,578
				total	\$40,523
				rounded total	\$41,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - Daniel Drive, north of Edan Place

Project Description	Project #	8.36
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Install new valves on mainline and lead	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-6

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	2	ea	\$1,500	\$3,000
	subtotal				\$15,074
	Contingency and Engineering (40%)				\$6,030
	total				\$21,104
	rounded total				\$21,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Replace hydrant assembly and lead - Artella Road

Project Description	Project #	8.37
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Install new valve on lead	Date	06/27/2016
Assumed 2m wide trench across roadway for restoration quantity	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-6

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
				subtotal	\$13,574
				Contingency and Engineering (40%)	\$5,430
				total	\$19,004
				rounded total	\$19,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Replace hydrant supply main - Bond Road, 160m north of Williams Road

Project Description	Project #	8.38
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Replace existing 100/150 main with new 200mm PVC main	Date	06/27/2016
Majority of main replacement in roadway shoulder	Prepared by:	J.Barta
Assumed 3m wide road restoration needed for new main at road edge	Checked by:	J.Clowes
	See Figure	8-6

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$12,000	\$12,000
	Insurance and Bonding	1	LS	\$3,000	\$3,000
	75mm asphalt restoration	1395	sq.m	\$20	\$27,900
	100mm base course gravel restoration	1395	sq.m	\$6	\$8,370
	350mm subbase course gravel restoration	1395	sq.m	\$15	\$20,925
	Rock removal allowance	200	m	\$150	\$30,000
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	2	ea	\$5,000	\$10,000
	200mm PVC watermain	465	m	\$250	\$116,250
	200mm Gate valve	3	ea	\$2,000	\$6,000
				subtotal	\$240,945
				Contingency and Engineering (40%)	\$96,378
				total	\$337,323
				rounded total	\$337,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace existing main, hydrant assembly and lead - OK Centre Road, 300m south of Robinson Road

Project Description	Project #	8.39
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Replace 100mm watermain between hydrant and upstream section of 150mm main	Date	06/27/2016
Install new valves on mainline and lead	Prepared by:	J.Barta
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	Checked by:	J.Clowes
	See Figure	8-6

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$3,500	\$3,500
	Insurance and Bonding	1	LS	\$1,000	\$1,000
	75mm asphalt restoration	375	sq.m	\$20	\$7,500
	100mm base course gravel restoration	375	sq.m	\$6	\$2,250
	350mm subbase course gravel restoration	375	sq.m	\$15	\$5,625
	Rock removal allowance	60	m	\$150	\$9,000
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Remove and dispose existing 100mm watermain	125	ea	\$50	\$6,250
	Tie-in to existing 100mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm PVC watermain	125	m	\$200	\$25,000
	150mm Gate valve	2	ea	\$1,500	\$3,000
	subtotal				\$79,625
	Contingency and Engineering (40%)				\$31,850
	total				\$111,475
	rounded total				\$111,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate					
Abandon GI watermain and reconnect services - Intersection of Camp and Tyndall Roads					
Project Description			Project #	8.40	
Abandon existing 50mm GI watermain.			USL Job No.	1577.0063.01	
Reconnect existing services (3) to existing main on west side of road			Date	06/27/2016	
Assumed 2m wide trench across roadway for restoration quantity			Prepared by:	J.Barta	
			Checked by:	J.Clowes	
			See Figure	8-7	
ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	60	sq.m	\$20	\$1,200
	100mm base course gravel restoration	60	sq.m	\$6	\$360
	350mm subbase course gravel restoration	60	sq.m	\$15	\$900
	Reconnect existing service to new watermain	3	ea	\$1,500	\$4,500
				subtotal	\$6,960
				Contingency and Engineering (40%)	\$2,784
				total	\$9,744
				rounded total	\$10,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - Intersection of Seaton and Camp Roads

Project Description	Project #	8.41
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Replace 100mm watermain between hydrant and upstream section of 150mm main	Date	06/27/2016
Install new valves on mainline and lead	Prepared by:	J.Barta
Watermain appears to be in shoulder - no road restoration included	Checked by:	J.Clowes
	See Figure	8-8

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
	200mm Gate valve	1	ea	\$2,000	\$2,000
	subtotal				\$15,000
	Contingency and Engineering (40%)				\$6,000
	total				\$21,000
	rounded total				\$21,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Replace existing 100mm watermain - Newene Road

Project Description	Project # 8.42
Abandon existing 100mm watermain. Install new 200mm PVC main	USL Job No. 1577.0063.01
Reconnect existing services (2) to new main	Date 06/27/2016
Install new gate valve at north end of replacement.	Prepared by: J.Barta
Assume half road restoration	Checked by: J.Clowes
	See Figure 8-8

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$6,000	\$6,000
	Insurance and Bonding	1	LS	\$2,000	\$2,000
	75mm asphalt restoration	765	sq.m	\$20	\$15,300
	100mm base course gravel restoration	765	sq.m	\$6	\$4,590
	350mm subbase course gravel restoration	765	sq.m	\$15	\$11,475
	Rock removal allowance	100	m	\$150	\$15,000
	Reconnect existing service to new watermain	2	ea	\$1,500	\$3,000
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 200mm watermain	1	ea.	\$5,000	\$5,000
	200mm PVC watermain	255	m	\$250	\$63,750
	200mm Gate valve	1	ea	\$2,000	\$2,000
	subtotal				\$133,115
	Contingency and Engineering (40%)				\$53,246
	total				\$186,361
	rounded total				\$186,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace existing main, hydrant assembly and lead - Intersection of OK Centre and Read Roads

Project Description	Project #	8.43
Remove existing 100mm main between intersection and hydrant tee	USL Job No.	1577.0063.01
Replace hydrant assembly and lead	Date	06/27/2016
Install new valves on mainline and lead	Prepared by:	J.Barta
Assumed half road restoration	Checked by:	J.Clowes
	See Figure	8-8

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	75	sq.m	\$20	\$1,500
	100mm base course gravel restoration	75	sq.m	\$6	\$450
	350mm subbase course gravel restoration	75	sq.m	\$15	\$1,125
	Rock removal allowance	10	m	\$150	\$1,500
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Remove and dispose existing 100mm watermain	25	ea	\$50	\$1,250
	Tie-in to existing 100mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm PVC watermain	25	m	\$200	\$5,000
	150mm Gate valve	2	ea	\$1,500	\$3,000
	subtotal				\$30,325
	Contingency and Engineering (40%)				\$12,130
	total				\$42,455
	rounded total				\$42,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Replace existing main, hydrant assembly and lead - Pollard Road

Project Description	Project #	8.44
Remove existing 100mm main between intersection and hydrant tee	USL Job No.	1577.0063.01
Replace hydrant assembly and lead	Date	06/27/2016
Install new valve on lead	Prepared by:	J.Barta
Assumed half road restoration	Checked by:	J.Clowes
	See Figure	8-8

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	30	sq.m	\$20	\$600
	100mm base course gravel restoration	30	sq.m	\$6	\$180
	350mm subbase course gravel restoration	30	sq.m	\$15	\$450
	Rock removal allowance	5	m	\$150	\$750
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Remove and dispose existing 100mm watermain	10	ea	\$50	\$500
	Tie-in to existing 100mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 150mm watermain	1	ea	\$5,000	\$5,000
	150mm PVC watermain	10	m	\$200	\$2,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
				subtotal	\$22,480
				Contingency and Engineering (40%)	\$8,992
				total	\$31,472
				rounded total	\$31,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate
Replace existing 100mm and 150mm watermains - Shanks Road

Project Description	Project #	8.45
Abandon existing 100mm and 150mm mains.	USL Job No.	1577.0063.01
Construct 200mm main extension to southern-most existing hydrant	Date	06/27/2016
Increases fire flow at last hydrant to 4,200 L/min	Prepared by:	J.Barta
Reconnect existing services (7) to new 200mm PVC watermain	Checked by:	J.Clowes
Assumed half road restoration	See Figure	8-9

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$10,000	\$10,000
	Insurance and Bonding	1	LS	\$3,000	\$3,000
	75mm asphalt restoration	1200	sq.m	\$20	\$24,000
	100mm base course gravel restoration	1200	sq.m	\$6	\$7,200
	350mm subbase course gravel restoration	1200	sq.m	\$15	\$18,000
	Rock removal allowance	200	m	\$150	\$30,000
	Reconnect existing service to new watermain	7	ea	\$1,500	\$10,500
	Tie-in to existing 100mm watermain	1	ea	\$5,000	\$5,000
	Tie-in to existing 200mm watermain	1	ea	\$5,000	\$5,000
	200mm PVC watermain	400	m	\$250	\$100,000
	200mm Gate valve	1	ea	\$2,000	\$2,000
				subtotal	\$214,700
				Contingency and Engineering (40%)	\$85,880
				total	\$300,580
				rounded total	\$301,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace existing 75mm GI watermain - Seaton Road, West of Read Road

Project Description	Project #	8.46
remove existing 75 GI watermain. Replace with 75mm PVC	USL Job No.	1577.0063.01
Reconnect existing services (3) to new 200mm PVC watermain	Date	06/27/2016
New mains in shoulders where possible. Still assumed 3m wide asphalt restoration	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-9

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$4,000	\$4,000
	Insurance and Bonding	1	LS	\$1,000	\$1,000
	75mm asphalt restoration	480	sq.m	\$20	\$9,600
	100mm base course gravel restoration	480	sq.m	\$6	\$2,880
	350mm subbase course gravel restoration	480	sq.m	\$15	\$7,200
	Rock removal allowance	80	m	\$150	\$12,000
	Reconnect existing service to new watermain	3	ea	\$1,500	\$4,500
	Remove and dispose existing 750mm watermain	160	ea	\$50	\$8,000
	Tie-in to existing 400mm watermain	1	ea	\$6,000	\$6,000
	75mm PVC watermain	160	m	\$150	\$24,000
	subtotal				\$79,180
	Contingency and Engineering (40%)				\$31,672
	total				\$110,852
	rounded total				\$111,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace existing main, hydrant assembly and lead - Mountainview Road, north of Janet Road

Project Description	Project #	8.47
Remove existing 100mm main, replace with 150mm PVC	USL Job No.	1577.0063.01
Replace hydrant assembly and lead	Date	06/27/2016
Install new valves on main and lead	Prepared by:	J.Barta
Reconnect existing services (3) to new 150mm PVC watermain	Checked by:	J.Clowes
Assumed half road restoration	See Figure	8-9

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Mobilization & Demobilization	1	LS	\$3,000	\$3,000
	Insurance and Bonding	1	LS	\$1,000	\$1,000
	75mm asphalt restoration	315	sq.m	\$20	\$6,300
	100mm base course gravel restoration	315	sq.m	\$6	\$1,890
	350mm subbase course gravel restoration	315	sq.m	\$15	\$4,725
	Rock removal allowance	0	m	\$150	\$0
	Reconnect existing service to new watermain	3	ea	\$1,500	\$4,500
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Remove and dispose existing 100mm watermain	90	ea	\$50	\$4,500
	Tie-in to existing 150mm watermain	2	ea	\$5,000	\$10,000
	150mm PVC watermain	90	m	\$200	\$18,000
	150mm Gate valve	2	ea	\$1,500	\$3,000
	subtotal				\$63,415
	Contingency and Engineering (40%)				\$25,366
	total				\$88,781
	rounded total				\$89,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Abandon 75mm Steel main and reconnect services - Glenmore Road, south of Janet Road

Project Description	Project # 8.48
Abandon existing 75mm watermain	USL Job No. 1577.0063.01
Reconnect existing services (5) to existing 300mm main	Date 06/27/2016
Watermains appear to be in the shoulder and not require road restoration	Prepared by: J.Barta
	Checked by: J.Clowes
	See Figure 8-9

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	Reconnect existing service to new watermain	5	ea	\$1,500	\$7,500
				subtotal	\$7,500
				Contingency and Engineering (40%)	\$3,000
				total	\$10,500
				rounded total	\$11,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Pre-Design Cost Estimate

Replace hydrant assembly and lead - Shanks Road, 140m south of Glenmore Road

Project Description	Project #	8.49
Replace hydrant assembly and lead	USL Job No.	1577.0063.01
Install new valves on main and lead	Date	06/27/2016
Assumed 2m wide road restoration for new lead across Shanks Road	Prepared by:	J.Barta
	Checked by:	J.Clowes
	See Figure	8-9

ITEM	DESCRIPTION	QTY	UNIT	\$/UNIT	EXTENDED
	75mm asphalt restoration	14	sq.m	\$20	\$280
	100mm base course gravel restoration	14	sq.m	\$6	\$84
	350mm subbase course gravel restoration	14	sq.m	\$15	\$210
	Replace existing hydrant assembly and lead	1	ea	\$6,500	\$6,500
	Tie-in to existing 250mm watermain	1	ea	\$5,000	\$5,000
	150mm Gate valve	1	ea	\$1,500	\$1,500
	250mm Gate valve	1	ea	\$3,000	\$3,000
	subtotal				\$16,574
	Contingency and Engineering (40%)				\$6,630
	total				\$23,204
	rounded total				\$23,000

Note: Estimates do not include land acquisition costs, nor legal fees

Cost estimate prepared without geotech or survey. Complete field investigations to refine estimate.

Appendix G

List of Miscellaneous Projects

Water Project	Project Description (Miscellaneous Projects)	Project Cost	Refer to Figure	Misc. Priority
8.4	Broadwater & Oyama Roads - Watermain Looping	\$950,000	8-5	
8.5	Oyama Water System - Optional PRV Replacement - Todd Road (East Hill)	\$320,000	8-6	
8.6	Oyama Water System - Optional PRV Replacement - Oyama Road (Cemetery)	\$320,000	8-6	
8.7	Oyama Water System - Optional PRV Replacement - MiddleBench (Harmen)	\$320,000	8-6	
8.8	Oyama Water System - Optional PRV Replacement - MiddleBench (Alison)	\$320,000	8-6	
8.9	Oyama Water System - Optional PRV Replacement - Oyama Road (Broadwater)	\$320,000	8-6	
8.10	Oyama Water System - Optional PRV Replacement - Towgood Road	\$320,000	8-6	
8.11	Oyama Water System - Optional PRV Replacement - Trew hitt Road	\$320,000	8-6	
8.12	Replace hydrant assembly and lead - Oyama Road, 250m south of Crawford Road	\$21,000	8-6	
8.13	Replace hydrant assembly and lead - Intersection of Oyama and Eyles Roads	\$21,000	8-6	
8.14	Abandon GI watermain and reconnect services - Oyama Road at Eyles Road	\$15,000	8-6	
8.15	Replace hydrant assembly and lead - Intersection of Oyama and Trew hitt Roads	\$21,000	8-6	
8.16	Replace GI watermain - Trew hitt Road, south of PRV station	\$13,000	8-6	
8.17	Replace hydrant assembly and lead - Trew hitt Road at PRV station	\$21,000	8-6	
8.18	Abandon GI watermain and reconnect services - Trew hitt Road, north of PRV station	\$12,000	8-6	
8.19	Replace hydrant assembly and lead - Trew hitt Road, 270m east of Oyama Road	\$22,000	8-6	
8.20	Replace hydrant assembly and lead - Oyama Road, 280m south of Towgood Road	\$21,000	8-6	
8.21	Replace hydrant assembly and lead - Middle Bench Road, 270m south of Towgood Road	\$23,000	8-6	
8.22	Replace hydrant assembly and lead - Intersection of Middle Bench and Towgood Roads	\$20,000	8-6	
8.23	Replace hydrant assembly and lead - Intersection of Middle Bench and Allison Roads	\$20,000	8-6	
8.24	Replace hydrant assembly and lead - Middle Bench Road, 150m north of Allison Road	\$20,000	8-6	
8.25	Abandoned GI watermain and reconnect services - Middle Bench Road near Harmen Road	\$34,000	8-6	
8.26	Replace hydrant assembly and lead - Intersection of Middle Bench and East Hill Roads	\$20,000	8-6	
8.27	Replace AC watermains and reconnect services - Middle Bench Road, North of East Hill Road	\$141,000	8-6	
8.28	Replace hydrant assembly and lead - East Hill Road, 250m south of Todd Road	\$20,000	8-6	
8.29	Replace hydrant assembly and lead - East Hill Road, 530m south of Todd Road	\$20,000	8-6	
8.30	Abandon AC watermains and reconnect services - Middle Bench Road, 500m south of Todd Road	\$25,000	8-6	
8.31	Replace watermains - Middle Bench Road, 500m south of Todd Road	\$74,000	8-6	
8.32	Replace hydrant assembly and lead - Middle Bench Road, 400m south of Todd Road	\$21,000	8-6	
8.33	Replace hydrant assembly and lead - Intersection of East Hill and Todd Roads	\$25,000	8-6	
8.34	Replace 100mm AC watermain with 150mm PVC water - Lacresta Road	\$95,000	8-7	
8.35	Replace 50mm GI watermain with 50mm PVC - Floral Road	\$41,000	8-7	
8.36	Replace hydrant assembly and lead - Daniel Drive, north of Edan Place	\$21,000	8-7	
8.37	Replace hydrant assembly and lead - Artella Road	\$19,000	8-7	
8.38	Replace hydrant supply main - Bond Road, 160m north of Williams Road	\$337,000	8-7	
8.39	Replace existing main, hydrant assembly and lead - OK Centre Road, 300m south of Robinson Road	\$111,000	8-7	
8.40	Abandon GI watermain and reconnect services - Intersection of Camp and Tyndall Roads	\$10,000	8-8	
8.41	Replace hydrant assembly and lead - Intersection of Seaton and Camp Roads	\$21,000	8-9	
8.42	Replace existing 100mm watermain - Newene Road	\$186,000	8-9	
8.43	Replace existing main, hydrant assembly and lead - Intersection of OK Centre and Read Roads	\$42,000	8-9	
8.44	Replace existing main, hydrant assembly and lead - Pollard Road	\$31,000	8-9	
8.45	Replace existing 100mm and 150mm watermains - Shanks Road	\$301,000	8-10	
8.46	Replace existing 75mm GI watermain - Seaton Road, West of Read Road	\$111,000	8-10	
8.47	Replace existing main, hydrant assembly and lead - Mountainview Road, north of Janet Road	\$89,000	8-10	
8.48	Abandon 75mm Steel main and reconnect services - Glenmore Road, south of Janet Road	\$11,000	8-10	
8.49	Replace hydrant assembly and lead - Shanks Road, 140m south of Glenmore Road	\$23,000	8-10	